

# **APPENDIX K**

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## **Scofield Island Back-Barrier Geotechnical Analysis**

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- Annex K1: Boring Logs and Laboratory Test Data
- Annex K2: Slope Stability Analysis
- Annex K3: Settlement Analysis

# **Scofield Island Back-Barrier Geotechnical Analysis**

## **1.0 INTRODUCTION**

The Scofield Island Back-Barrier Geotechnical Analysis was completed in support of the Feasibility Study and Preliminary Design Phases for the Riverine Sand Mining / Scofield Island Restoration Project (Project). The Project is sponsored by the Louisiana Department of Natural Resources (LDNR), State of Louisiana Office of Coastal Protection and Restoration (OCPR) and NOAA Fisheries. The Project design is funded and authorized in accordance with the provisions of the Coastal Wetlands Planning, Protection and Restoration Act (CWPPRA) (16 U.S.C.A., Sections 3951-3956) and has been approved by the Public Law 101-646 Task Force. The Project's CWPPRA designation is BA-40.

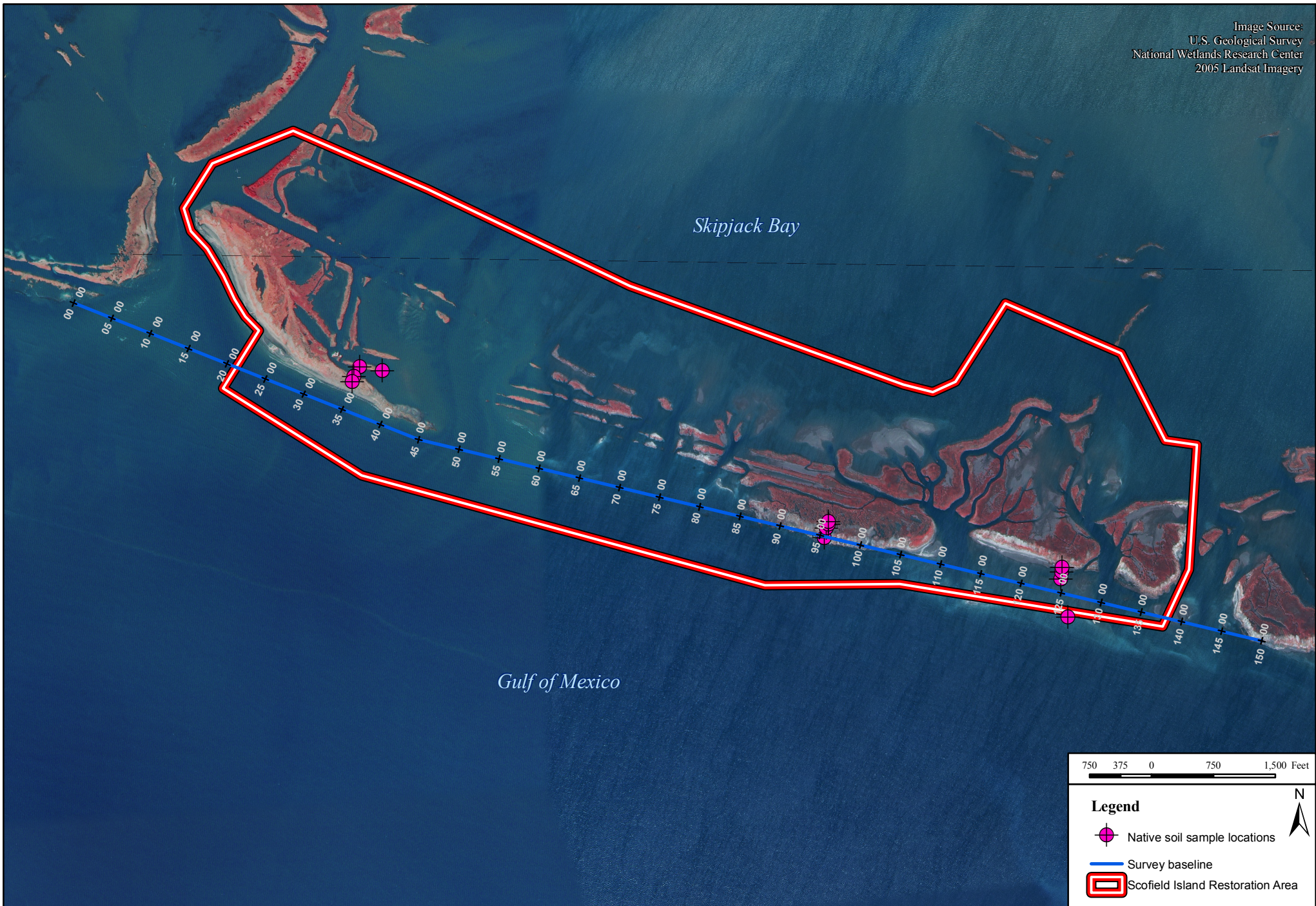
The purpose of the geotechnical investigation was to provide design data for the marsh creation plan for Scofield Island as fully described in the Preliminary Design Main Report and Scofield Island Restoration Area Design Analysis (Appendix M). The scope of services included the drilling of soil test borings to determine subsoil conditions and stratification, and to obtain samples of the various substrata. Soil mechanics laboratory tests, performed on samples obtained from the borings, were used to evaluate the physical properties of the subsoils. Engineering analyses, based on the soil borings and laboratory test results, were made to evaluate the stability of the foundation support for the proposed shoreline protection and marsh creation features. Analyses were performed to evaluate stability of the containment dike and retained marsh fill. Analyses were also made to estimate settlement of the dike and marsh area fill and the time-rate of settlement of these features. The soil test borings and analyses were conducted by Eustis Engineering Company, LLC. (EEC, 2009) and reviewed by Coastal Engineering Consultants, Inc (CEC). SJB Group, LLC. (SJB) provided survey control.

The analyses and results were prepared in accordance with generally accepted geotechnical engineering practice for the exclusive use of OCPR and their designated representatives specific to the Project. In the event of significant changes in the marsh creation design, the conclusions and recommendations contained in this report shall not be considered valid unless the changes are reviewed and conclusions modified and verified in writing by EEC. Should these data be used by anyone other than OCPR and their designated representatives, EEC should be consulted for interpretation of data and to secure any other information pertinent to the Project.

## **2.0 PROJECT AREA AND LOCATION**

Scofield Island is a 2.4 mile long barrier island located east of Pelican Island between Scofield Bayou and the merger of Bay Coquette and the Gulf of Mexico, Plaquemines Parish. The Project area is shown in Figure 1.





**Figure 1:**  
**Scofield Island Native Sediment Sample Locations**



### **3.0 SOIL BORINGS**

Six undisturbed sample type soil test borings, designated as B-1 through B-6, were made on October 7 through 11, 2008 at the approximate locations shown on Figure 1. Each boring was drilled to a depth of 50 feet below the mudline using a skid mounted rotary type drill rig mounted on a shallow draft boat. The borings were located in the field using a hand held GPS unit. Based on site access, several of the borings had to be offset from their intended locations. The proposed and actual locations are identified on Figure 1. Subsequent to the drilling operations, the mudline elevations at the boring locations were surveyed on October 30, 2008. Upon completion of drilling operations, the borings were backfilled in accordance with the laws of the State of Louisiana. Detailed descriptive logs of the borings are shown in both tabular and graphical form in Annex K1. Included on these logs are the coordinates and mudline elevations.

Samples of cohesive or semi-cohesive subsoils were obtained at close intervals or changes in stratum using a 3-inch diameter thinwall Shelby tube sampling barrel. The samples were immediately extruded from the sampling barrel, inspected, and visually classified. Pocket penetrometer tests were performed on the soil samples to give a general indication of their shear strength or consistency. The results of these tests are shown on the boring logs under the column heading "PP." Representative portions were then promptly placed in moisture proof containers and sealed for preservation of their natural moisture content.

Samples of non-cohesive and semi-cohesive materials were obtained during the performance of in situ Standard Penetration Tests. This test consisted of driving a 2-inch diameter splitspoon sampler 1 foot into the ground after first seating it 6 inches. A 140-lb weight dropped 30 inches is used to advance the sampler. The number of blows required to drive the sampler is indicative of the relative density of non-cohesive soils and the approximate consistency of cohesive soils. The samples were retained in moisture proof containers for preservation of their natural moisture content. The results of the Standard Penetration Tests are shown on the boring logs under the column heading ASPT.@

### **4.0 LABORATORY TESTS**

Soil mechanics laboratory tests, consisting of natural water content, unit weight, and either unconfined compression shear (UC) or one-point unconsolidated un-drained triaxial compression shear (OB), were performed on undisturbed samples obtained from the borings. In addition, Atterberg liquid and plastic limits tests were performed on selected samples to aid in classification of the subsoils and to give an indication of their relative compressibility. The results of the laboratory tests are summarized on the boring logs in Annex K1.

Organic content tests were performed on selected samples obtained from the borings to determine the amount of organic matter in the samples. The results of these tests are shown on the boring logs.

Consolidation tests (CONS) were performed on selected samples to determine their compressibility and stress history. Grain size analyses (SV) were performed on selected samples obtained from the borings to determine their particle distribution curves. The results of these tests are shown on separate sheets following the boring logs in Annex K1.

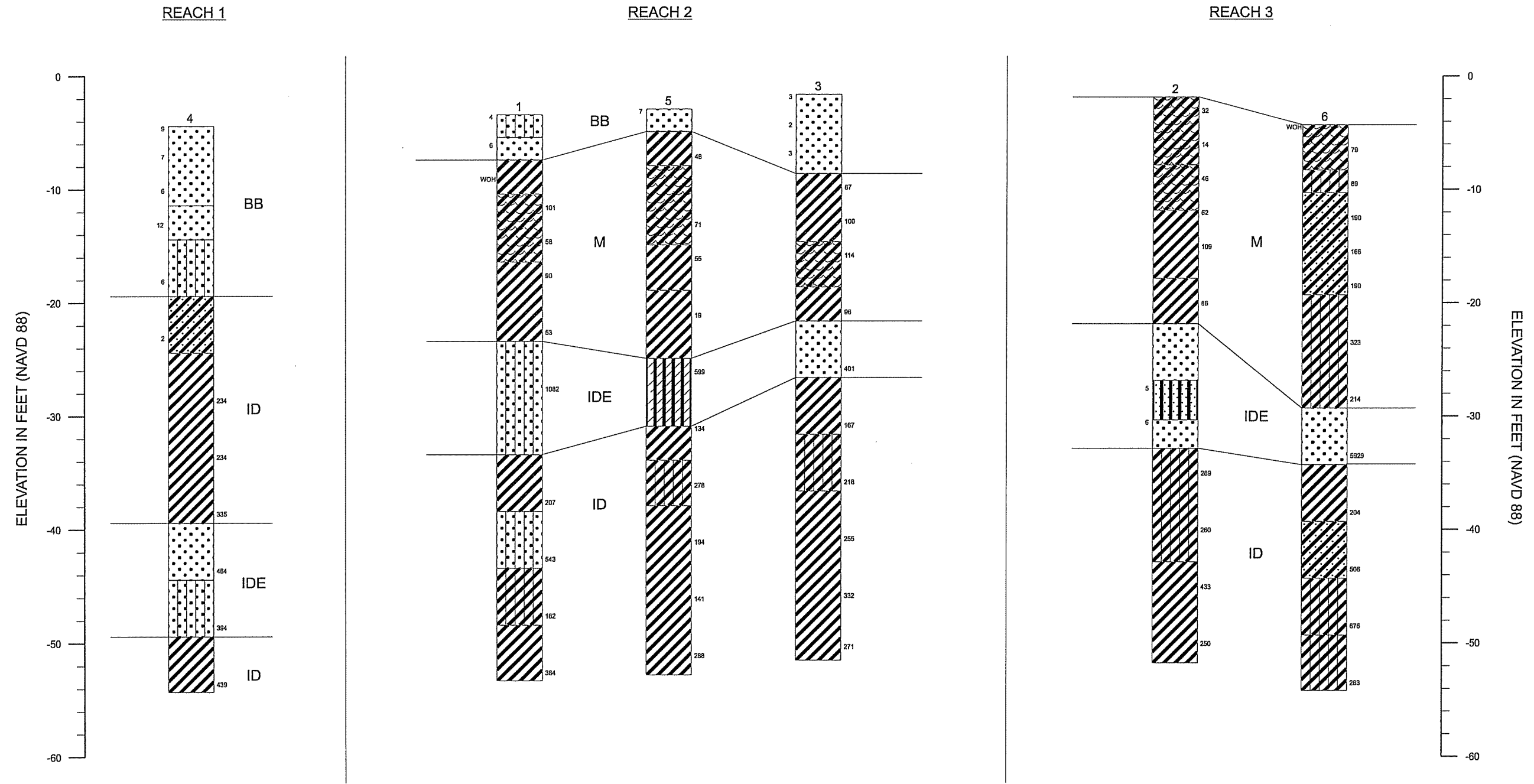
## **5.0 DESCRIPTION OF SUBSOIL CONDITIONS**

### **5.1 Geologic Setting**

The project is located at the distal end of the abandoned Lafourche Delta in southeastern Louisiana. Specifically, the project site is located due south of Buras, Louisiana, along the retreating Louisiana coastline. Severe land loss and shoreline retreat has resulted in isolating this remnant barrier beach complex located at the mouth of Scofield Island Bayou, an abandoned distributary of Big Cypress Bayou, which in turn, is an abandoned distributary of the Mississippi River. Continuous wave attack and landward retreat has winnowed out the fine clays and silts, leaving a frontal small barrier beach complex overlying the marsh and interdistributary deposits across which the shoreline is retreating.

### **5.2 Stratigraphy**

A generalized geologic subsoil profile is presented on Figure 2. At most of the boring locations, the surface consists of a layer of beach sand with abundant shells. As a result of the retreat of the shoreline through storm overwash and breaching, this surface layer is absent at Borings 2 and 6 (eastern end of the island) and varies in thickness from 2 feet at Boring 5 to 15 feet in Boring 4. Underlying the sand deposits are very soft organic clay marsh deposits varying in thickness from approximately 25 feet in Boring 6 to about 9 feet in Boring 3. The marsh deposits are absent in Boring 4 because of the proximity to the active Scofield Bayou migrating channel. The marsh deposits are underlain by intradelta deposits of silty sand and sand which are in turn underlain by very soft to soft interdistributary clays to the termination of the borings at depths of 50 feet below the mudline. The interpretation of subsoil conditions are also shown on the individual boring logs.



**STRATA SYMBOLS**

	HIGH PLASTICITY ORGANIC CLAYS		CLAYEY SILT		SILTY SAND		SANDY LOW PLASTICITY CLAY
	HIGH PLASTICITY CLAY		SILTY LOW PLASTICITY CLAY		POORLY GRADED SAND		SANDY SILT

- NOTES:**
- VALUES SHOWN TO THE RIGHT OF LOG ARE SHEAR STRENGTH IN PSF.
  - VALUES SHOWN TO THE LEFT OF LOG ARE SPT RESULTS.

**GEOLOGIC UNIT**

BB:	BARRIER BEACH DEPOSITS
M:	MARSH DEPOSITS
IDE:	INTRADELTA DEPOSITS
ID:	INTERDISTURBUTARY DEPOSITS

<p><b>EUSTIS ENGINEERING SERVICES, L.L.C.</b>          GEOTECHNICAL ENGINEERS          3011 28TH STREET METAIRIE, LOUISIANA</p>		
<p><b>GEOLOGIC PROFILE</b></p> <p>STATE OF LOUISIANA          RIVERINE SAND MINING/SCOFIELD          ISLAND RESTORATION (BA-40)          PLAQUEMINES PARISH, LOUISIANA</p>		
DRAWN BY: J.L.S.	PLOT DATE: 19 MAR 09	CADD FILE: PROFILE.DGN
CHECKED BY: G.P.S.	JOB NO.: 19292	<b>FIGURE 2</b>

### 5.3 Water Levels

The borings were drilled in either standing water or in the marsh with ground water at the soil surface. Water depths measured at the time of the field investigation generally varied from 2 to 3 feet. Water depths will vary with climatic conditions, tidal fluctuations, and other factors. Site conditions should be investigated by the contractor for construction immediately prior to beginning work. Furnished information indicates the mean low water (MLW) is at +0.55 North American Vertical Datum of 1988 (NAVD88) and mean high water (MHW) is +1.60 NAVD88.

### 6.0 PROJECT DESCRIPTION

The Project goal is to repair breaches and tidal inlets in the shoreline, reinforce the existing shoreline with sand, and increase the island width with back barrier marsh creation. Preliminary project parameters and assumptions were provided by SJB and CEC in order to conduct foundation and settlement analysis as follows:

- Water Levels
  - MLW = +0.55 feet NAVD88,
  - MHW = +1.60 feet NAVD88
- Project design life is 20 years
- Topographic survey data including cross-sections
- Geologic Subsidence rate of 0.025 feet/year
- Sea-level Rise = 0.03 feet/year
- Containment Dike
  - Construction from borrow material excavated on site
  - Construction depth 6 to 8 feet below MLW
  - Crest elevations will range from +4 to +6 feet NAVD88, approximately 2 feet above the marsh platform elevation
  - Crest width = 20 feet
- Marsh Platform
  - Construction from material dredged and transported from the Scofield Offshore Borrow Area (SOBA)
  - Target platform elevation will range from +2 to +3.5 feet NAVD88
- Beach and Dunes
  - Construction from sand dredged from the Mississippi River and transported to the location by sediment pipeline
  - Target beach berm elevation will range from +4 to +6 feet NAVD88
  - Target dune crest elevation will range from +6 to +7 feet NAVD88

## **7.0 FOUNDATION ANALYSES**

### **7.1 Design Parameters**

A geologic subsoil profile of the three reaches is shown on Figure 2, as previously noted. Plots of the soil design parameters used in the analyses for each of these reaches are provided on Figures 3 through 5. For the settlement analyses, additional parameters, not shown on Figures 3 through 5, were also assumed below the available boring data as discussed with the settlement analysis results.

Design parameters for the in situ marsh containment fill materials were assumed to have an average wet unit weight of 88 pcf and remolded shear strength of 100 psf. The analyses assumed the marsh fill will be placed by uncompacted methods in standing water. A wet unit weight of 100 pcf was selected for the SOBA sediment comprised of a mixture of sand, silt, and clay (Appendix J).

### **7.2 Stability Analyses**

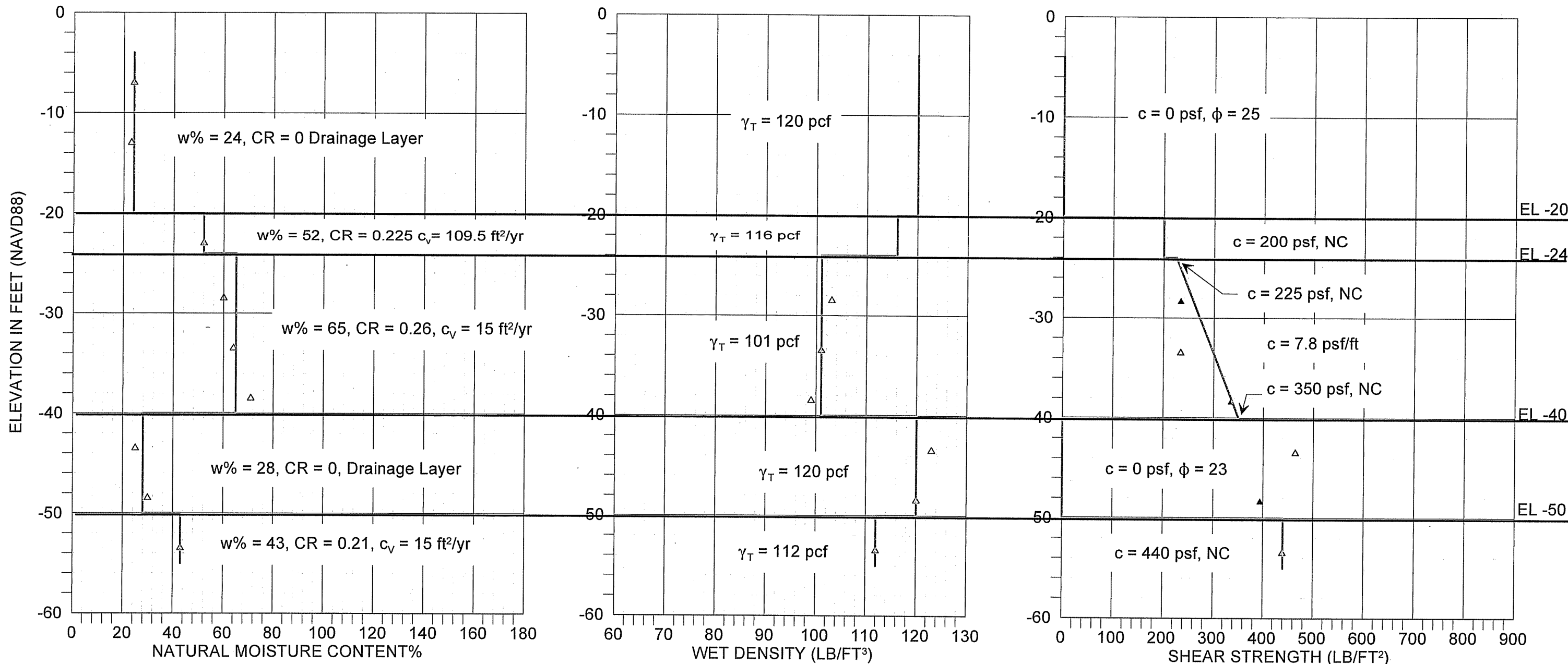
#### **7.2.1 Methodology**

Slope stability analyses were conducted by a two-dimensional limit equilibrium stability analysis of selected trial failure surfaces. These analyses were performed using GEO-SLOPE International, Ltd.'s program Slope/W 2007, Version 7.11. This program generally utilizes circular arcs to define the soil failure planes. These arcs are then divided into slices and the factor of safety computed by summing forces, summing moments, or both. For these analyses, the inter-slice forces are typically considered. The factors of safety presented are based on Spencer's Method of Slices. Using this method, the recommended minimum factor of safety was computed to be 1.3. Slope stability analyses were performed for the design of proposed containment dikes that will enclose the marsh creation features.

#### **7.2.2 Design Assumptions**

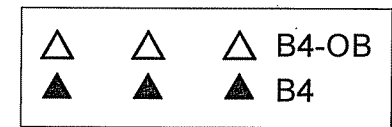
The design assumptions for the containment dikes included construction at a maximum bottom level (mudline elevation) of -2 feet NAVD88 and a crest height between +4 and +6 feet NAVD88. Further, the crest width will be approximately 20 feet and the dikes will be constructed from dredged material excavated from adjacent borrow areas / floatation channels.



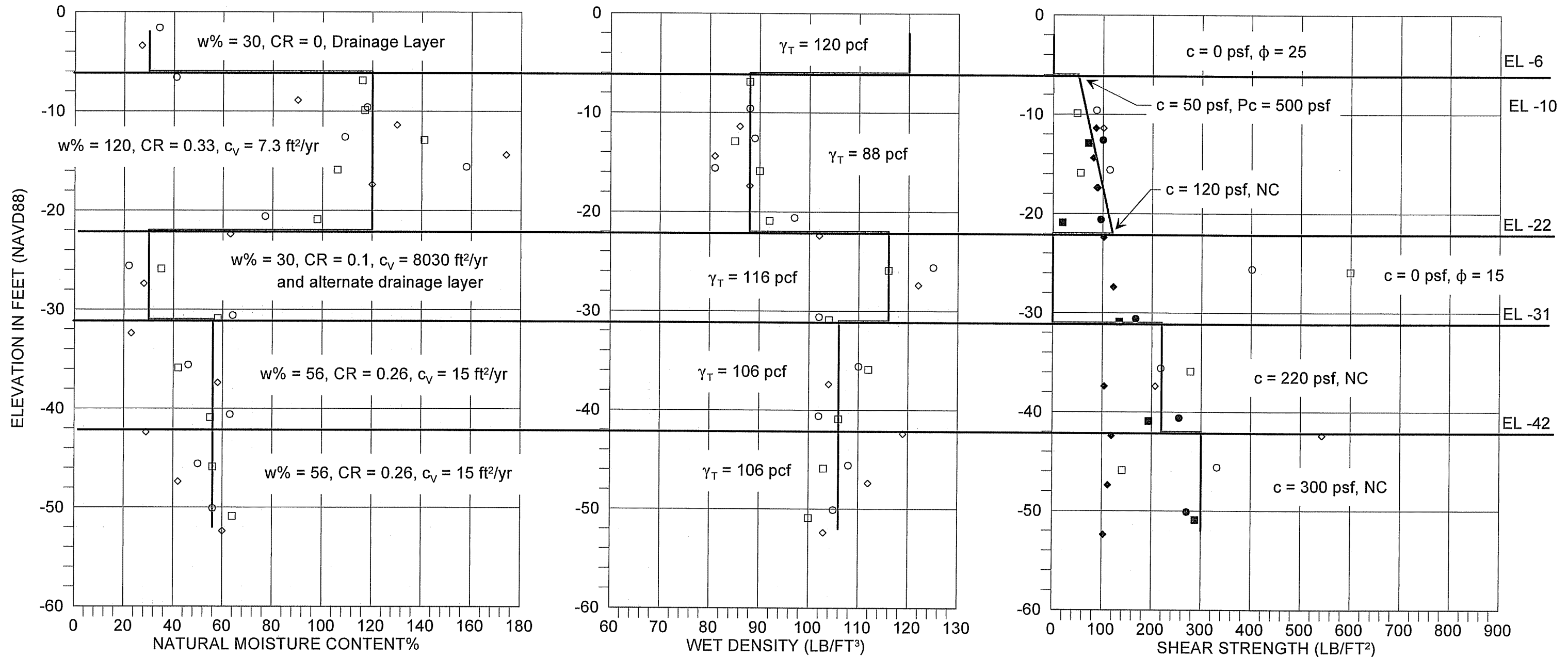


NOTES:

- LOGS OF SOIL BORINGS PROVIDED IN APPENDIX I.
- UNIT WEIGHTS SHOWN ARE TOTAL UNIT WEIGHTS AND MUST BE APPROPRIATELY REDUCED TO ESTIMATE EFFECTIVE STRESS STATES.
- DESIGN PROFILES SHOWN CAN NOT FULLY ANTICIPATE ALL PARAMETERS WHICH MAY INFLUENCE SELECTION OF DESIGN VALUES FOR A SPECIFIC ANALYSIS. FOR THIS REASON, THE USER SHOULD CONTACT EUSTIS ENGINEERING PRIOR TO USE OF DESIGN PROFILES IN ANY ANALYSES.
- w% = NATURAL MOISTURE CONTENT, CR = COMPRESSION RATIO, c<sub>v</sub> = COEFFICIENT OF CONSOLIDATION  
 γ<sub>T</sub> = TOTAL (WET) UNIT WEIGHT, C = COHESION, φ = FRICTION ANGLE  
 NC = NORMALLY CONSOLIDATED, OCR = OVER CONSOLIDATION RATIO

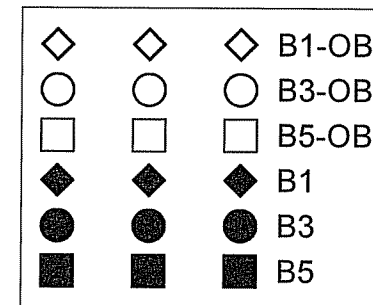


 <b>EUSTIS ENGINEERING SERVICES, LLC</b> GEOTECHNICAL ENGINEERS 3011 28TH STREET    METAIRIE, LOUISIANA		
<b>SOIL DESIGN PARAMETERS</b> <b>SOIL REACH 1 (B-4)</b>		
STATE OF LOUISIANA RIVERINE SAND MINING SCOFIELD ISLAND RESTORATION (BA-40) PLAQUEMINES PARISH, LOUISIANA		
DRAWN BY: J.T.H.	5 JAN 2009	FILE: FIGURE3_SHEET1 DESIGN REACH 1.GRF
CHECKED BY: G.P.S.	JOB NO.: 19292	FIGURE 3, SHEET 1

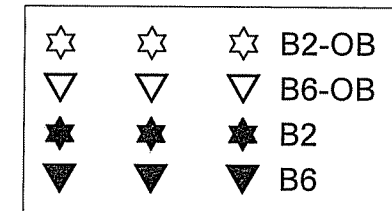
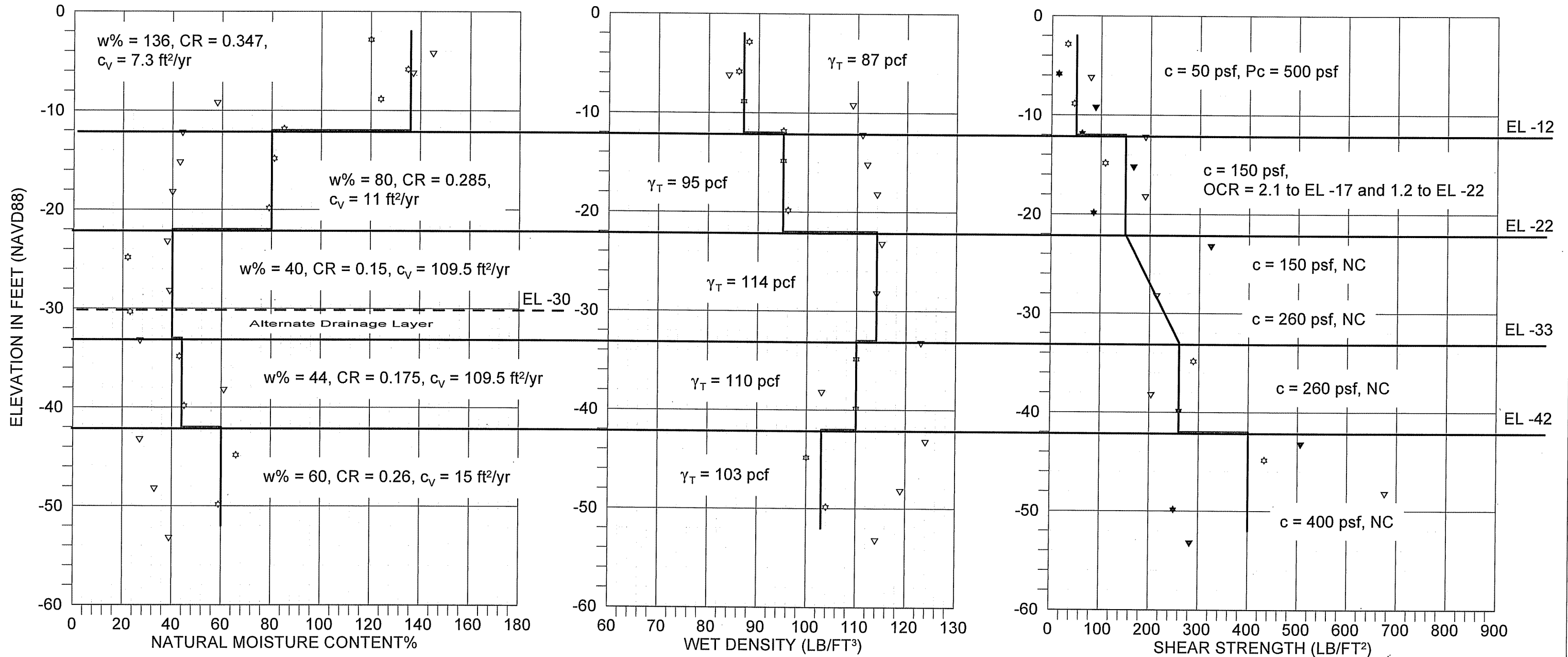


NOTES:

- LOGS OF SOIL BORINGS PROVIDED IN APPENDIX I.
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- w% = NATURAL MOISTURE CONTENT, CR = COMPRESSION RATIO, c<sub>v</sub> = COEFFICIENT OF CONSOLIDATION  
 $\gamma_T$  = TOTAL (WET) UNIT WEIGHT, C = COHESION,  $\phi$  = FRICTION ANGLE  
 NC = NORMALLY CONSOLIDATED, OCR = OVER CONSOLIDATION RATIO



 <b>EUSTIS ENGINEERING SERVICES, LLC</b> GEOTECHNICAL ENGINEERS 3011 28TH STREET    METAIRIE, LOUISIANA		
<b>SOIL DESIGN PARAMETERS</b> <b>SOIL REACH2 (B-1, B-3 AND B-5)</b>		
STATE OF LOUISIANA RIVERINE SAND MINING SCOFIELD ISLAND RESTORATION (BA-40) PLAQUEMINES PARISH, LOUISIANA		
DRAWN BY: J.T.H.	5 JAN 2009	FILE: FIGURE3_SHEET2 DESIGN REACH 2.GRF
CHECKED BY: G.P.S.	JOB NO.: 19292	FIGURE 3, SHEET 2



**NOTES:**

- LOGS OF SOIL BORINGS PROVIDED IN APPENDIX I.
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 γ<sub>T</sub> = TOTAL (WET) UNIT WEIGHT, C = COHESION, φ = FRICTION ANGLE  
 NC = NORMALLY CONSOLIDATED, OCR = OVER CONSOLIDATION RATIO

<b>EUSTIS ENGINEERING SERVICES, LLC</b> GEOTECHNICAL ENGINEERS 3011 28TH STREET    METAIRIE, LOUISIANA		
<b>SOIL DESIGN PARAMETERS</b> <b>SOIL REACH 3 (B-2 AND B-6)</b>		
STATE OF LOUISIANA RIVERINE SAND MINING SCOFIELD ISLAND RESTORATION (BA-40) PLAQUEMINES PARISH, LOUISIANA		
DRAWN BY: J.T.H.	5 JAN 2009	FILE: FIGURE3_SHEETS3 DESIGN REACH 3.GRF
CHECKED BY: G.P.S.	JOB NO.: 19292	FIGURE 3, SHEET 3

### **7.2.3 Data Evaluation**

Based on the furnished topographic data, the average mudline in the vicinity of the marsh containment dike is at -2 to 0 feet NAVD88. This excludes the deeper channel elevations, which should be avoided.

### **7.2.4 Water Levels**

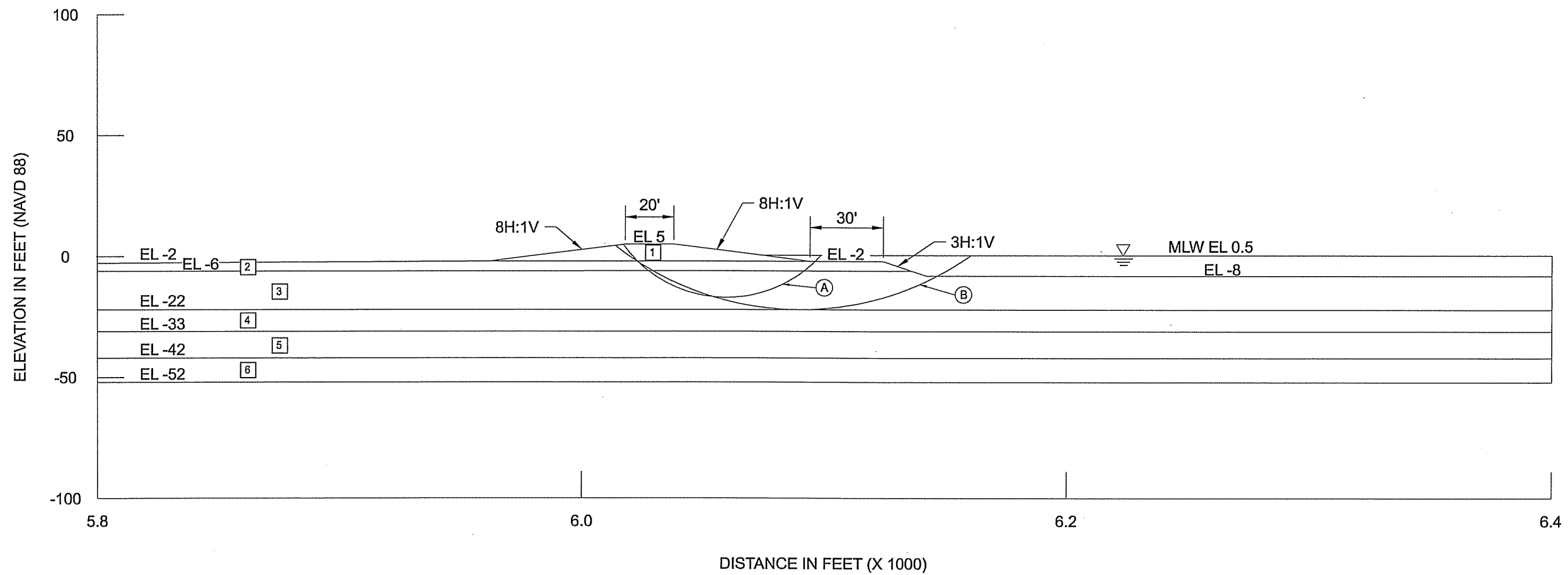
The stability analyses presented are based on the furnished low water level. Extreme low or high water levels due to a storm event were not evaluated. Water levels above or below that analyzed may result in localized sloughing or failure of the recommended section. Long term maintenance should consider this potential.

### **7.2.5 Results of Analyses**

The results of the stability analyses for the containment dike to be constructed in Reach 2 are shown on Figures 6 and 7 and in Annex K2.

### **7.2.6 Containment Dike Slopes**

Considering containment dikes constructed from +4 to +6 feet NAVD88, the recommended side slope is 1 vertical on 8 horizontal. This slope will allow construction to the +6 feet NAVD88 crown when the mudline is at 0 feet NAVD88. For a mudline at -2 feet NAVD88, the maximum crown height that can be achieved is at +5 feet NAVD88. Evaluations for Reach 3 indicated construction within or directly adjacent to the existing channels should be avoided. Other limited reaches where marsh deposits have been exposed should also be anticipated to require additional measures during construction to attain a stable dike. Considering a crown width of 20 feet, placement of marsh area fill will have minimal impact on the stability of the dike. Area fill will, however, induce additional settlement at the dike as addressed below.



SOIL NO.	DESCRIPTION	FRICTION ANGLE IN DEGREES	UNIT WEIGHT IN PCF	COHESION IN PSF	
				AVG.	BASE
1	CONTAINMENT DIKE FILL	0	88	100	100
2	BARRIER BEACH SAND	25	120	0	0
3	MARSH CLAY	0	88	85	120
4	SILT/SAND	15	116	0	0
5	CLAY	0	106	220	220
6	CLAY	0	106	300	300

SLIP SURFACE DESIGNATION	TANGENT ELEVATION OF SLIP SURFACE	TYPE OF FAILURE SURFACE	POINT	DEFINED FAILURE SURFACE			COMPUTED FACTOR OF SAFETY	COMPUTER FILE
				X-COORDINATE	Y-COORDINATE	RADIUS		
(A)	EL -17	CIRCULAR	CENTER	6059.56	35	52	1.314	REACH2 INSITU DIKE CROWN5C.GSZ
(B)	EL -22	CIRCULAR	CENTER	6089.7	100	122	1.412	

NOTES:

- 1) SOIL DESIGN PARAMETERS AND STRATA BREAK ELEVATIONS BASED ON SOIL REACH 2.
- 2) STABILITY ANALYSES PERFORMED USING SLOPE/W AND SPENCER'S METHOD OF SLICES.



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GEOTECHNICAL ENGINEERS

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METAIRIE, LOUISIANA

STABILITY OF CONTAINMENT DIKE AND BORROW  
REACH 2 - MUDLINE AT EL -2

STATE OF LOUISIANA  
RIVERINE SAND MINING/SCOFIELD  
ISLAND RESTORATION (BA-40)  
PLAQUEMINES PARISH, LOUISIANA

DRAWN BY: J.L.S.

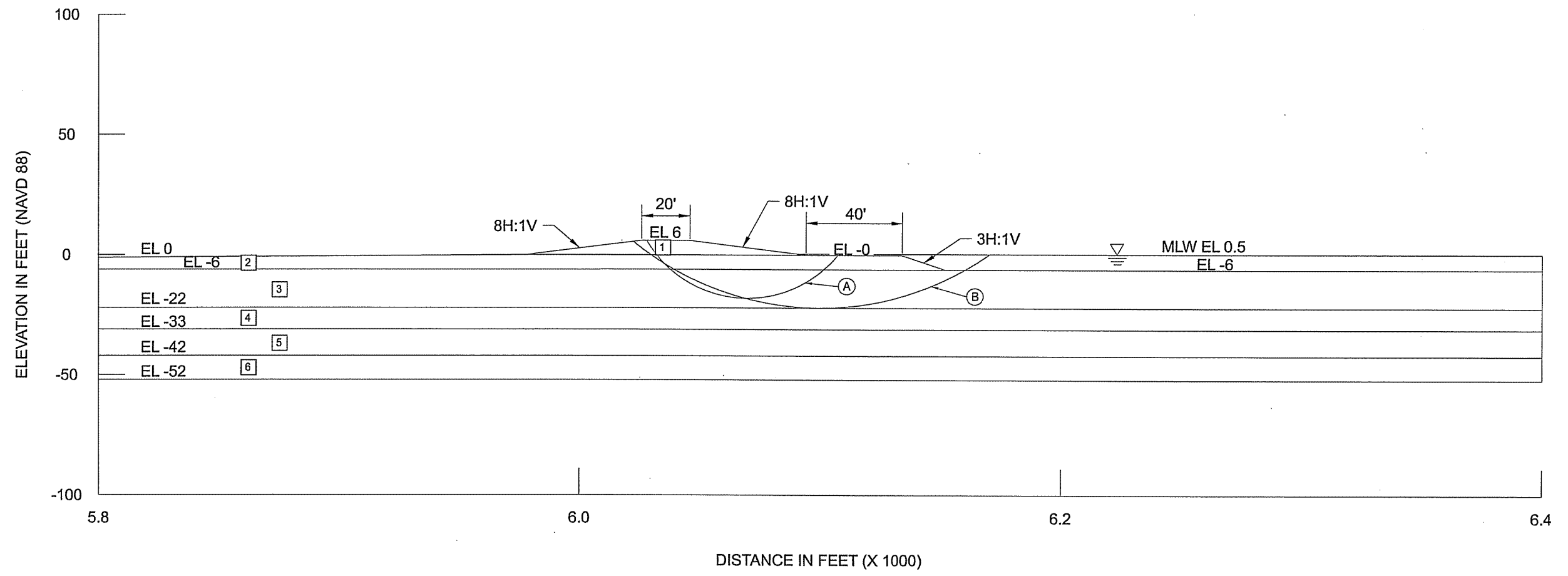
PLOT DATE: 23 MAR 09

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MUDLINE EL-2.DGN

CHECKED BY: G.P.S.

JOB NO.: 19292

FIGURE 4 SHEET 1 OF 2



SOIL NO.	DESCRIPTION	FRICTION ANGLE IN DEGREES	UNIT WEIGHT IN PCF	COHESION IN PSF	
				AVG.	BASE
1	CONTAINMENT DIKE FILL	0	88	100	100
2	BARRIER BEACH SAND	25	120	0	0
3	MARSH CLAY	0	88	85	120
4	SILT/SAND	15	116	0	0
5	CLAY	0	106	220	220
6	CLAY	0	106	300	300

SLIP SURFACE DESIGNATION	TANGENT ELEVATION OF SLIP SURFACE	TYPE OF FAILURE SURFACE	POINT	DEFINED FAILURE SURFACE			COMPUTED FACTOR OF SAFETY	COMPUTER FILE
				X-COORDINATE	Y-COORDINATE	RADIUS		
(A)	EL -18	CIRCULAR	CENTER	6069.61	30	48	1.328	REACH2 INSITUDIKE CROWN6D.GSZ
(B)	EL -22	CIRCULAR	CENTER	6099.75	100	122	1.313	REACH2 INSITUDIKE CROWN6F.GSZ

NOTES:

- 1) SOIL DESIGN PARAMETERS AND STRATA BREAK ELEVATIONS BASED ON SOIL REACH 2.
- 2) STABILITY ANALYSES PERFORMED USING SLOPE/W AND SPENCER'S METHOD OF SLICES.



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GEOTECHNICAL ENGINEERS

3011 28TH STREET

METAIRIE, LOUISIANA

STABILITY OF CONTAINMENT DIKE AND BORROW  
REACH 2 - MUDLINE AT EL 0

STATE OF LOUISIANA  
RIVERINE SAND MINING/SCOFIELD  
ISLAND RESTORATION (BA-40)  
PLAQUEMINES PARISH, LOUISIANA

DRAWN BY: J.L.S.

PLOT DATE: 23 MAR 09

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MUDLINE EL0.DGN

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FIGURE 4  
SHEET 2 OF 2

### **7.2.7 Containment Dike Buffer to Borrow/Floatation Channel**

To provide a factor of safety of 1.3 with respect to the borrow area or floatation channel, the recommended buffer, defined by the distance between the toe of the containment dike and location of borrow area / floatation channel, should be at least 30 feet plus three times the dredge depth within the borrow area / floatation channel. For the -2 feet NAVD88 mudline, the buffer should be increased to at least 40 feet plus three times the dredge depth for the 0 feet NAVD88 mudline. These dimensions are all based on Reach 2 conditions and are suitable for use in Reach 1, although higher dikes or flatter slopes would also work in Reach 1. These analyses are based on a maximum cut of -8 feet NAVD88. Borrow area / floatation channel side slopes were assumed as 1 vertical on 3 horizontal, although actual slopes will depend on construction methods and borrow material properties. This buffer should also be provided between the proposed dike and any existing channels that are equal to or deeper than -8 feet NAVD88.

## **8.0 SETTLEMENT ANALYSES**

### **8.1 Stress History**

Consolidation tests were performed on samples of the subsoils. The results of these tests are summarized in Annex K3. Surficial barrier beach deposits were considered as an incompressible drainage layer. The underlying or surficial marsh deposits were considered slightly pre-compressed to normally consolidated. Intradelta deposits were modeled as both a second drainage layer or as a silt layer having a very high rate of consolidation. Interdistributary deposits were generally considered to be normally consolidated.

Soils below the boring depths were also considered to be normally consolidated to a depth of 80 feet. Settlement beyond the 80-foot depth was not considered in the analyses. The settlement design parameters through the boring depths are shown on Figure 3 with the other design values. Parameters assumed between the 50 and 80-foot depths were generally normally consolidated clays having a compression ratio of 0.2 and a coefficient of consolidation of 7 ft<sup>2</sup>/yr. Isolated lenses and layers of sand may be present beneath the boring depths; however, available geologic data were not conclusive. Therefore, no additional drainage layers were assumed when estimating the rates of consolidation in these deposits.

### **8.2 Design Assumptions**

The fill areas assumed in the settlement analyses considered a 1,000 foot x 10,000 foot marsh fill area, an adjacent 1,000 foot x 10,000 foot beach fill area, and a 20 foot x 10,000 foot containment dike area. Assuming the beach fill and containment dikes are constructed prior to

placement of the marsh area fill, the settlement of the dike was considered with no influence from the area fill. Similarly, settlement estimates have been made considering the influence of the area fill on long term dike consolidation.

For the beach fill, the analyses considered fill having an average wet unit weight of 120 pcf and placed to an average height at +6 feet NAVD88. For the marsh area consolidation, it was assumed the area fill has an average wet unit weight of 100 pcf and is placed from +2 to +3.5 feet NAVD88. It was considered that the dikes would be constructed to +4 and +6 feet NAVD88 and a variation in the mudline along the dike between 0 and -2 feet NAVD88. A variation in the water surface level from +0.5 to +1.5 feet NAVD88 was assumed to compute the submerged fill weights.

Time-rates of consolidation settlement of the dredged and pumped marsh fill material and the underlying subsoils were estimated within the proposed marsh creation and the containment dike areas. It is noted there may be differences in the time-settlements of excavated and pipeline placed materials.

### **8.3 Results**

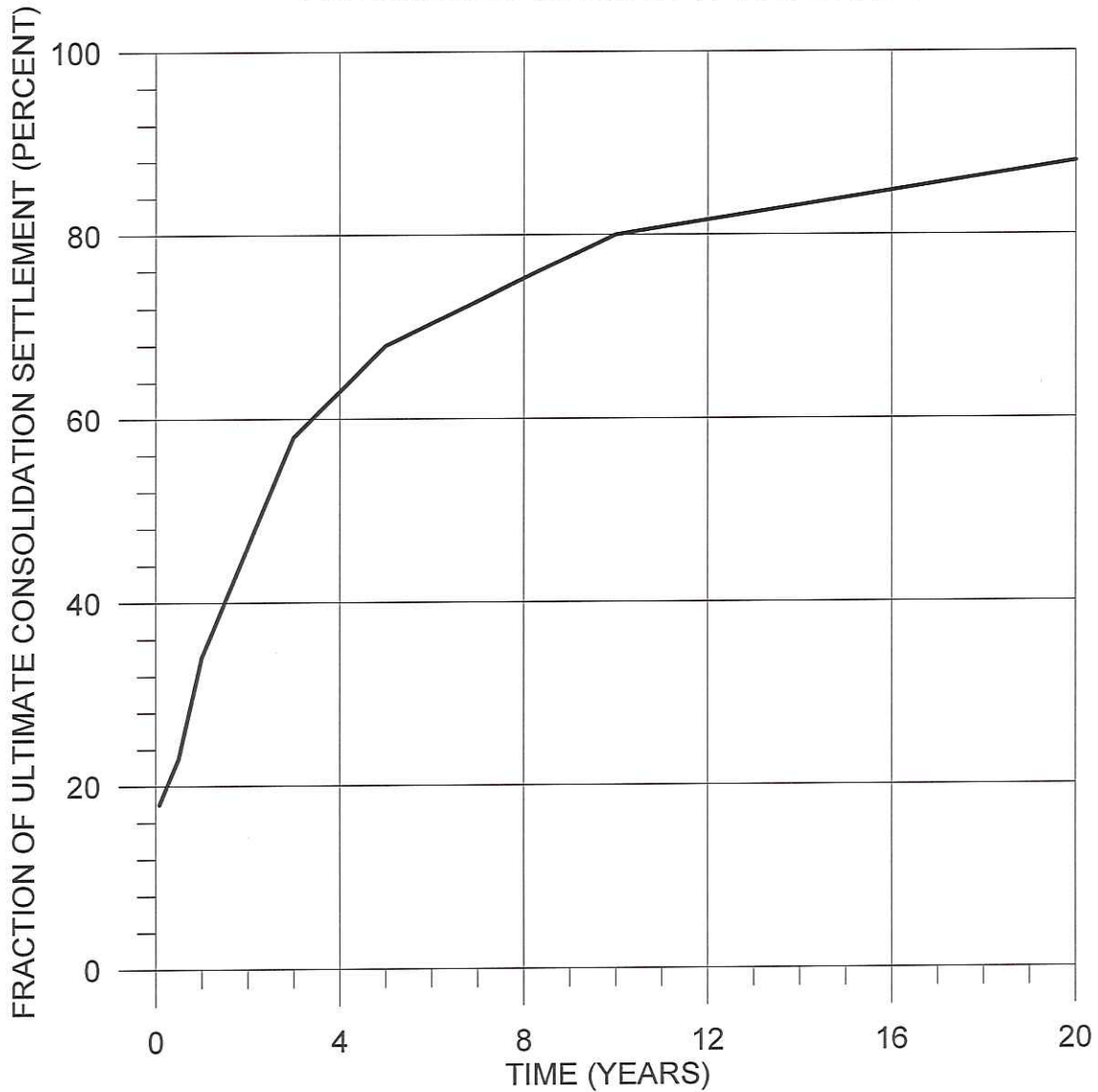
The results of the settlement analyses are presented in Figures 8 through 15. The analyses account for settlement of the subsoils due only to the weight of the fill, this consolidation plus the self-weight consolidation within the imported fill materials themselves, and finally, the effects of geologic subsidence. Summary tables, graphs, and sample settlement calculations are also included in Annex K3.

The dike consolidation graphs and tables are based on Reach 2, since Reach 1 would have minimal settlement and Reach 3 requires additional previously noted measures. There are two sets of values presented, one case where the dike is in place and no marsh fill has yet been placed, "no influence," and the other case that considers the additional influence of the area fill and beach fill.

In addition to settlement of the underlying subsoils, settlement or "shrinkage" of the un-compacted fill will occur. Shrinkage is due to drying, consolidation of the fill under its own weight, and deterioration due to biodegradation of organic fill materials inadvertently placed within the containment area. The desiccation of soft clays proceeds from the exposed surface inward and leads to formation of a crust that becomes thicker with age. Based on similar restoration projects in coastal Louisiana, the estimated volume loss, due to shrinkage of the fill, will be approximately 10% to 15% of the surficial crust formed by drying out of the soils.



**ESTIMATE OF SETTLEMENT TIME RATE  
FOR SUBSOILS IN SOIL REACHES 2 AND 3**



**NOTES:**

- 1) TIME RATE FOR CONSOLIDATION SETTLEMENT OF SUBSURFACE DEPOSITS ONLY.
- 2) COMPOSITE TIME RATE BASED ON SOIL REACHES 2 AND 3, ALTERNATE FILL HEIGHTS, MUDLINES AND WATER SURFACE ELEVATIONS.



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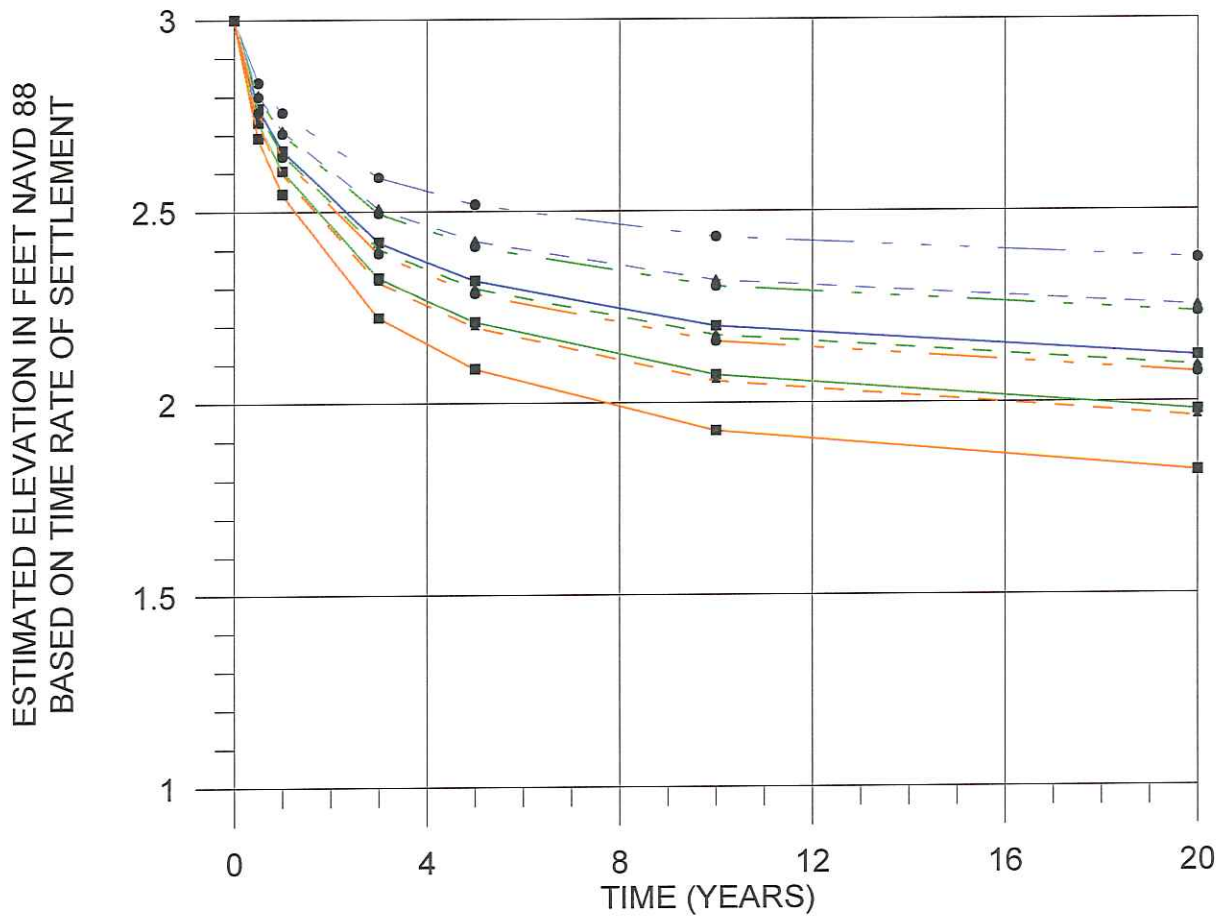
**ESTIMATED TIME-RATE OF SETTLEMENT**

STATE OF LOUISIANA  
 RIVERINE SAND MINING  
 SCOFIELD ISLAND RETORATION (BA-40)  
 PLAQUEMINES PARISH, LOUISIANA

DRAWN BY: G.P.S.	21 MARCH 2009	FILE: SETL_TIME_RATE.GRF
CHECKED BY: W.W.G.	JOB NO.: 19292	FIGURE 5 SHEET 1

ESTIMATES OF SETTLEMENT TIME RATE  
MARSH FILL PLATFORM  
CENTER OF 1,000' X 10,000' AREA

SOIL REACH 2  
INITIAL PLATFORM EL 3 NAVD88  
EXCLUDING SELF-WEIGHT CONSOLIDATION



MUDLINE ELEV  
 — EL 0  
 — EL -1  
 — EL -2

WATER SURFACE  
 ■ — ■ — ■ EL 0.5  
 ▲ — ▲ — ▲ EL 1  
 ● — ● — ● EL 1.5

NOTES:

- 1) PLOT COLOR REPRESENTS MUDLINE ELEVATION.
- 2) PLOT SYMBOL AND LINE TYPE REPRESENT WATER SURFACE ELEVATION.
- 3) SETTLEMENT ESTIMATES SHOULD BE ANTICIPATED TO VARY  $\pm 25\%$ .



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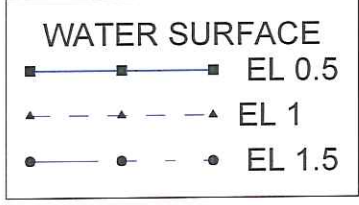
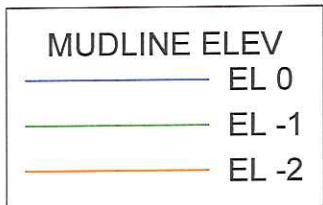
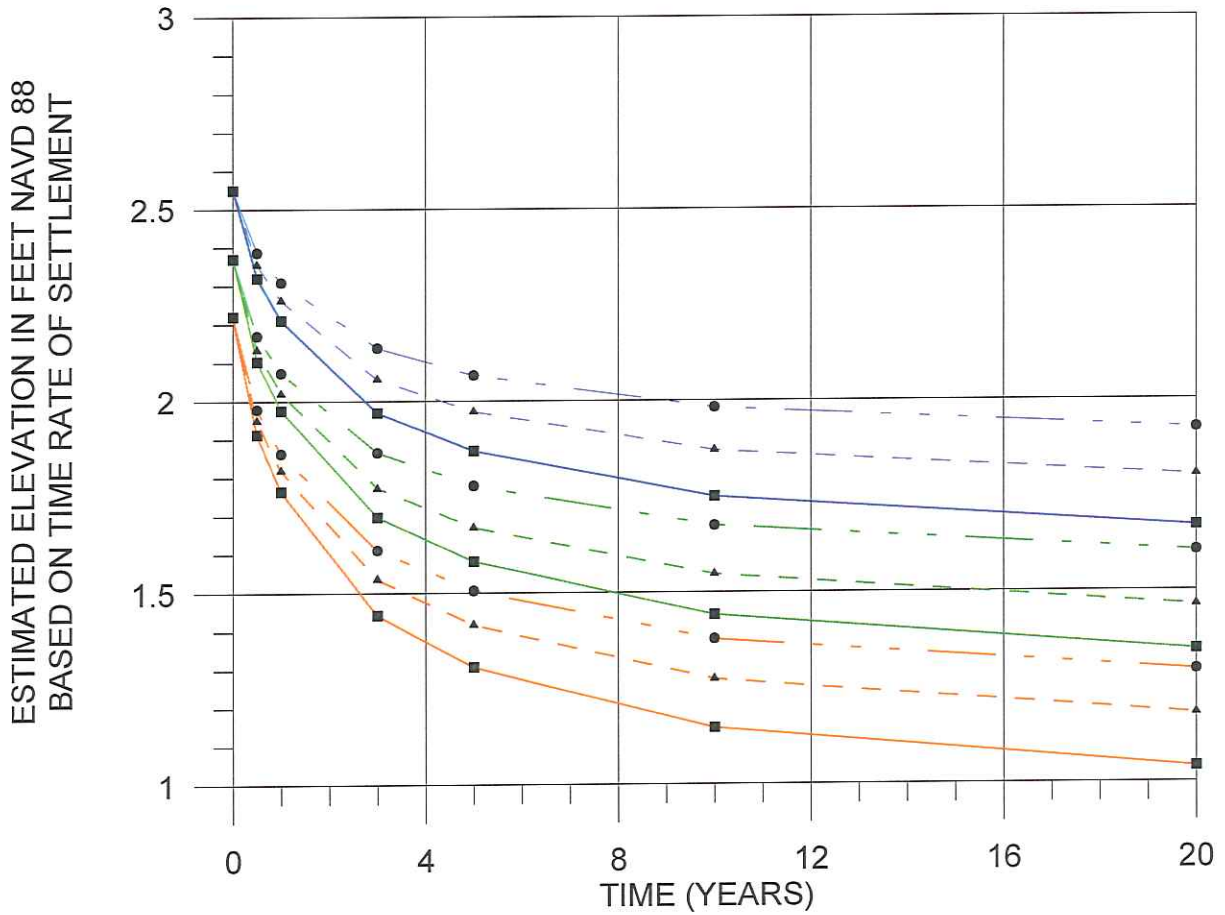
ESTIMATED TIME-RATE OF SETTLEMENT

STATE OF LOUISIANA  
 RIVERINE SAND MINING  
 SCOFIELD ISLAND RETORATION (BA-40)  
 PLAQUEMINES PARISH, LOUISIANA

DRAWN BY: G.P.S.	16 MARCH 2009	FILE: SETL_TIME_RATE.GRF
CHECKED BY: W.W.G.	JOB NO.: 19292	FIGURE 5 SHEET 2

ESTIMATES OF SETTLEMENT TIME RATE  
 MARSH FILL PLATFORM  
 CENTER OF 1,000' X 10,000' AREA

SOIL REACH 2  
 INITIAL PLATFORM EL 3 NAVD88  
 CONSIDERING SELF-WEIGHT CONSOLIDATION



NOTES:

- 1) PLOT COLOR REPRESENTS MUDLINE ELEVATION.
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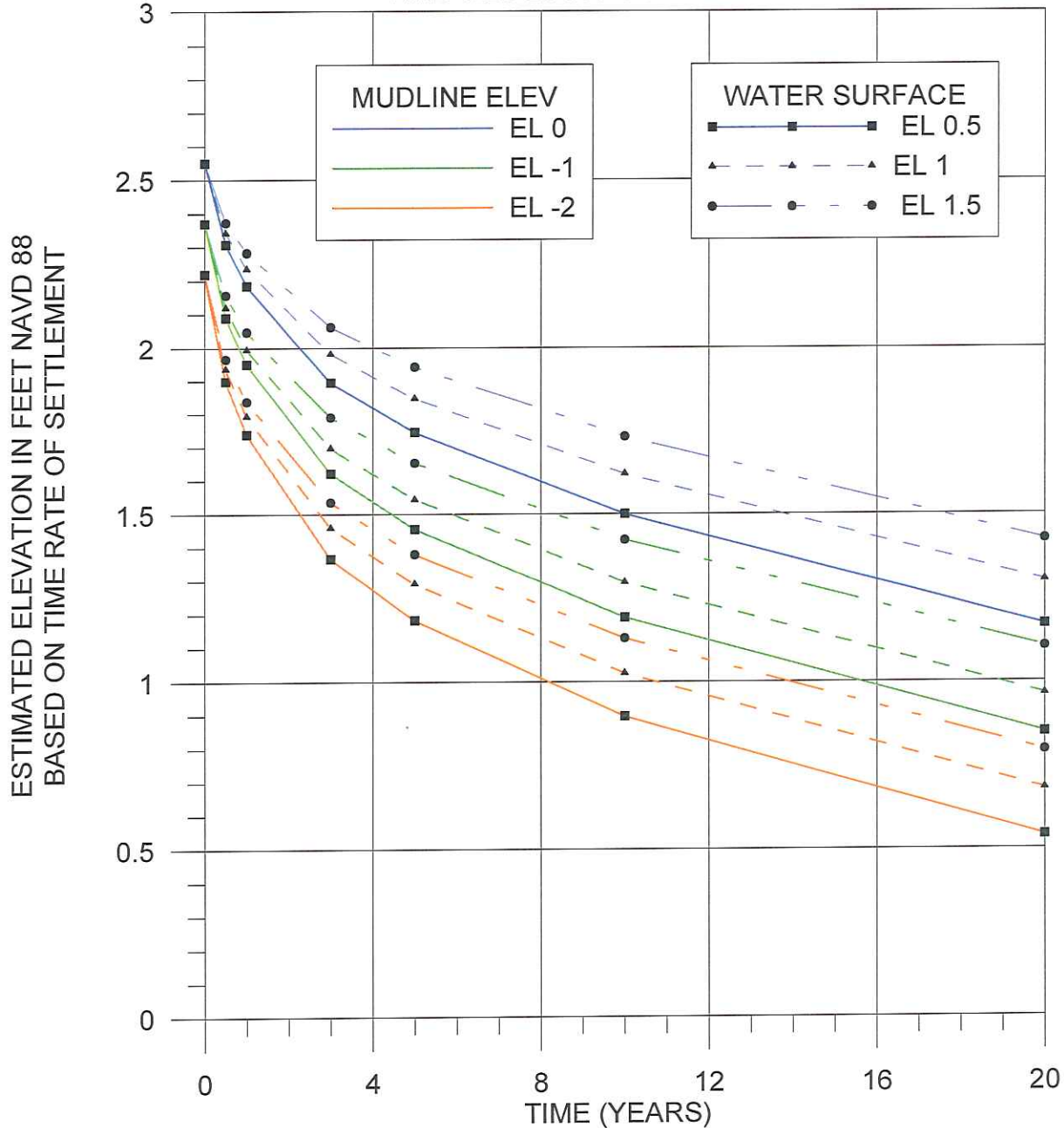
ESTIMATED TIME-RATE OF SETTLEMENT

STATE OF LOUISIANA  
 RIVERINE SAND MINING  
 SCOFIELD ISLAND RETORATION (BA-40)  
 PLAQUEMINES PARISH, LOUISIANA

DRAWN BY: G.P.S.	16 MARCH 2009	FILE: SETL_TIME_RATE_SW.GRF
CHECKED BY: W.W.G.	JOB NO.: 19292	FIGURE 5 SHEET 3

ESTIMATES OF SETTLEMENT TIME RATE  
MARSH FILL PLATFORM  
CENTER OF 1,000' X 10,000' AREA

SOIL REACH 2  
INITIAL PLATFORM EL 3 NAVD88  
CONSIDERING SELF-WEIGHT CONSOLIDATION  
AND GEOLOGIC SUBSIDENCE



NOTES:

- 1) PLOT COLOR REPRESENTS MUDLINE ELEVATION.
- 2) PLOT SYMBOL AND LINE TYPE REPRESENT WATER SURFACE ELEVATION.
- 3) SETTLEMENT ESTIMATES SHOULD BE ANTICIPATED TO VARY  $\pm 25\%$ .



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ESTIMATED TIME-RATE OF SETTLEMENT

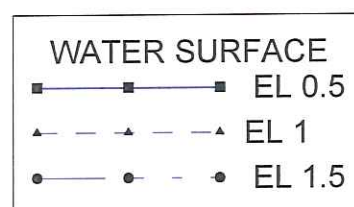
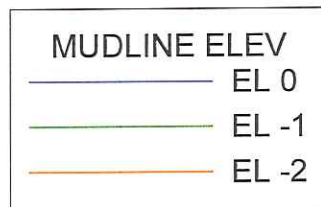
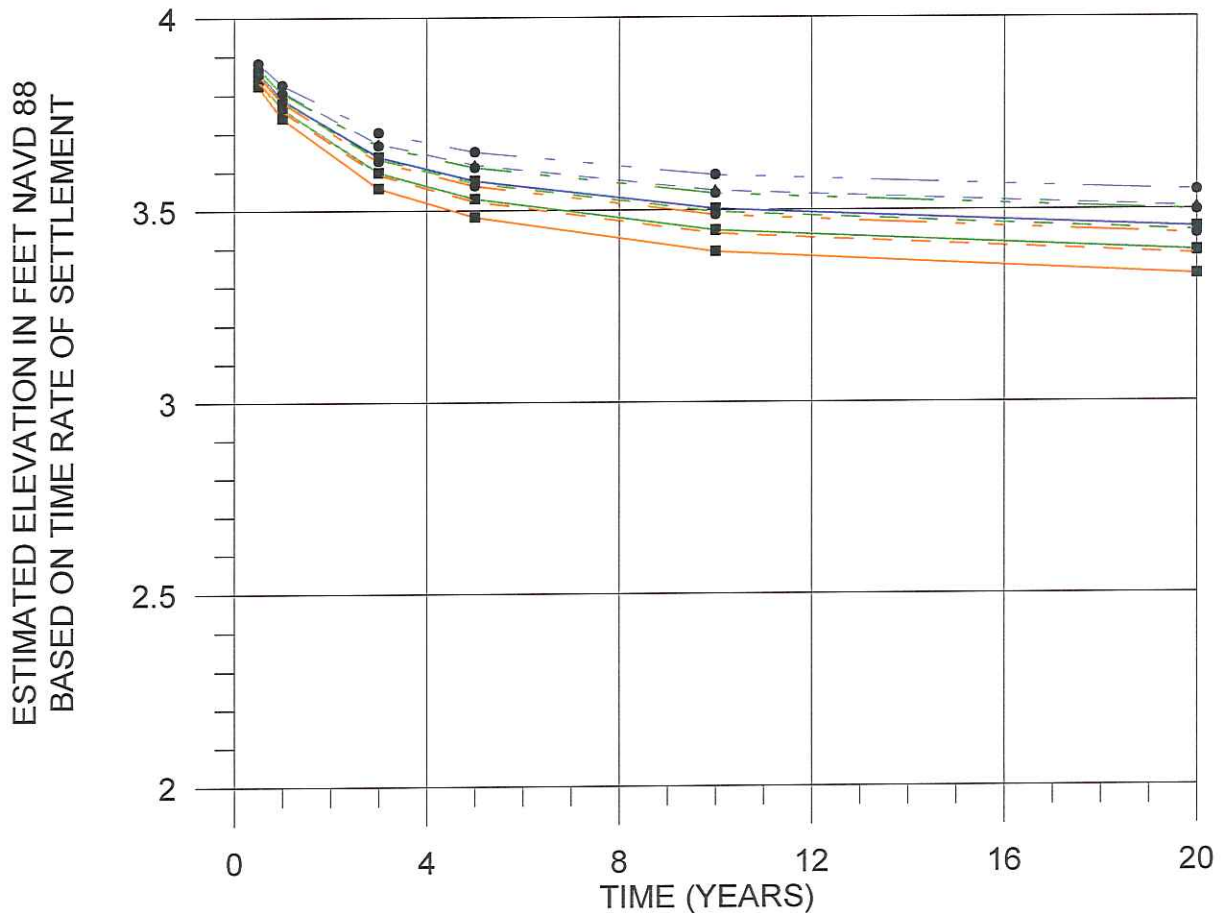
STATE OF LOUISIANA  
RIVERINE SAND MINING  
SCOFIELD ISLAND RETORATION (BA-40)  
PLAQUEMINES PARISH, LOUISIANA

DRAWN BY: G.P.S.	16 MARCH 2009	FILE: SETL_TIME_RATE_ALL.GRF
CHECKED BY: W.W.G.	JOB NO.: 19292	FIGURE 5 SHEET 4



ESTIMATES OF SETTLEMENT TIME RATE  
CONTAINMENT DIKE  
CENTER OF 20' X 10,000' AREA

SOIL REACH 2  
INITIAL DIKE EL 4 NAVD88  
EXCLUDING SELF-WEIGHT CONSOLIDATION  
AND GEOLOGIC SUBSIDENCE  
ALSO EXCLUDES ADJACENT FEATURE INFLUENCE



NOTES:

- 1) PLOT COLOR REPRESENTS MUDLINE ELEVATION.
- 2) PLOT SYMBOL AND LINE TYPE REPRESENT WATER SURFACE ELEVATION.
- 3) SETTLEMENT ESTIMATES SHOULD BE ANTICIPATED TO VARY  $\pm 25\%$ .



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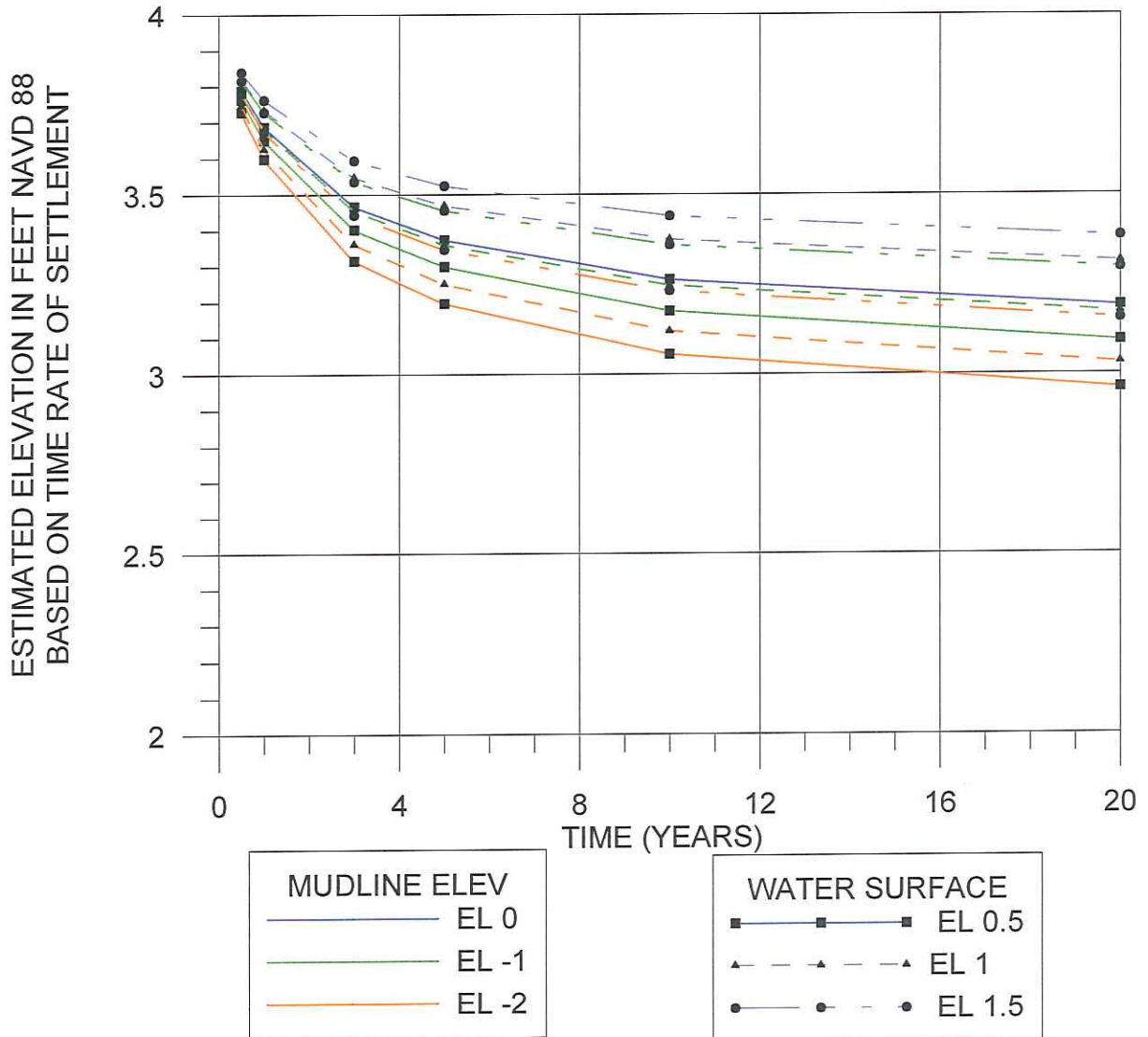
ESTIMATED TIME-RATE OF SETTLEMENT

STATE OF LOUISIANA  
RIVERINE SAND MINING  
SCOFIELD ISLAND RETORATION (BA-40)  
PLAQUEMINES PARISH, LOUISIANA

DRAWN BY: G.P.S.	16 MARCH 2009	FILE: SETL_TIME_RATE.GRF
CHECKED BY: W.W.G.	JOB NO.: 19292	FIGURE 5 SHEET 5

ESTIMATES OF SETTLEMENT TIME RATE  
CONTAINMENT DIKE  
CENTER OF 20' X 10,000' AREA

SOIL REACH 2  
INITIAL DIKE EL 4 NAVD88  
EXCLUDES SELF-WEIGHT AND GEOLOGIC  
ONLY CONSIDERS INFLUENCE OF ADJACENT FEATURES



NOTES:

- 1) PLOT COLOR REPRESENTS MUDLINE ELEVATION.
- 2) PLOT SYMBOL AND LINE TYPE REPRESENT WATER SURFACE ELEVATION.
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ESTIMATED TIME-RATE OF SETTLEMENT

STATE OF LOUISIANA  
RIVERINE SAND MINING  
SCOFIELD ISLAND RETORATION (BA-40)  
PLAQUEMINES PARISH, LOUISIANA

DRAWN BY: G.P.S.

16 MARCH 2009

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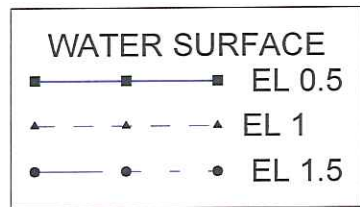
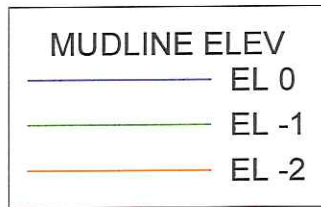
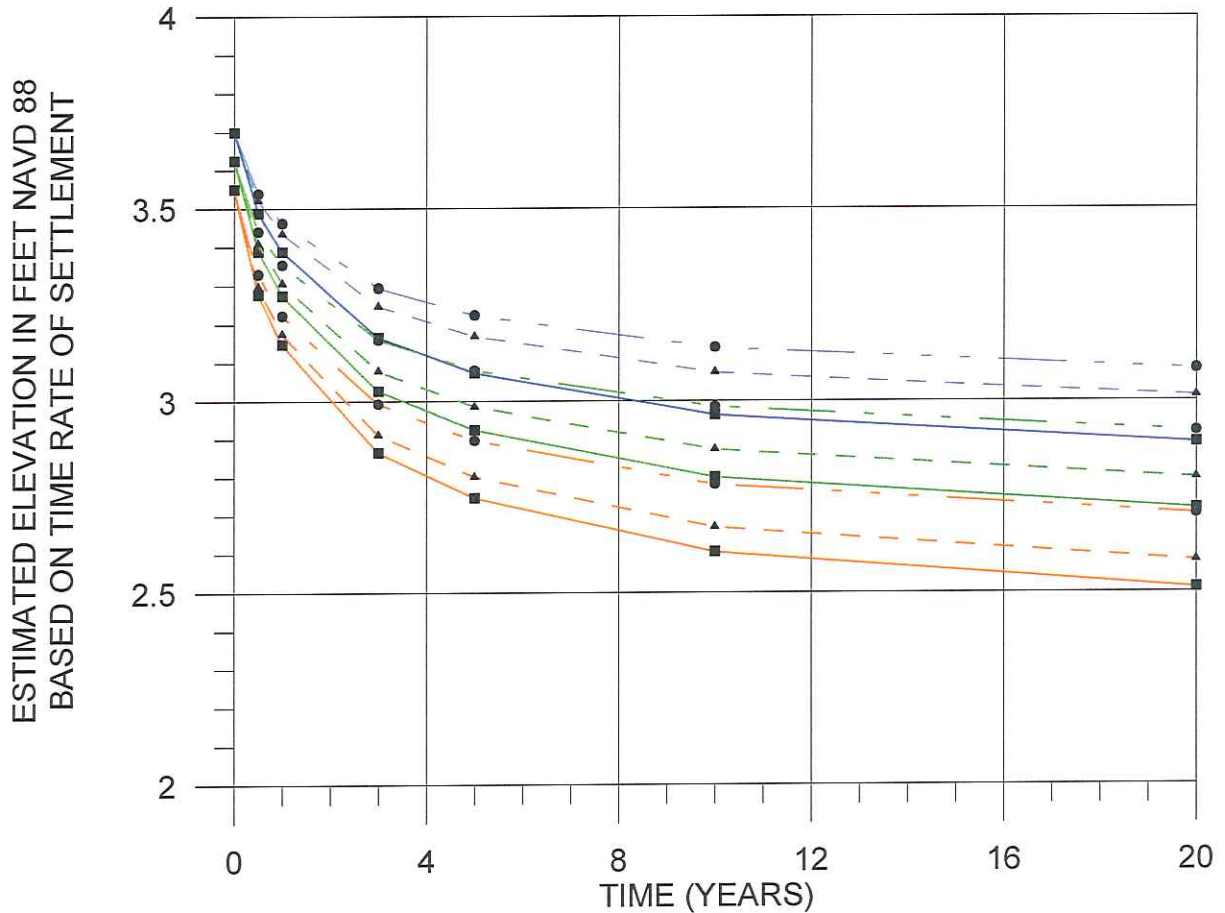
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JOB NO.: 19292

FIGURE 5  
SHEET 6

ESTIMATES OF SETTLEMENT TIME RATE  
CONTAINMENT DIKE  
CENTER OF 20' X 10,000' AREA

SOIL REACH 2  
INITIAL DIKE EL 4 NAVD88  
CONSIDERING SELF-WEIGHT CONSOLIDATION



NOTES:

- 1) PLOT COLOR REPRESENTS MUDLINE ELEVATION.
- 2) PLOT SYMBOL AND LINE TYPE REPRESENT WATER SURFACE ELEVATION.
- 3) SETTLEMENT ESTIMATES SHOULD BE ANTICIPATED TO VARY  $\pm 25\%$ .



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ESTIMATED TIME-RATE OF SETTLEMENT

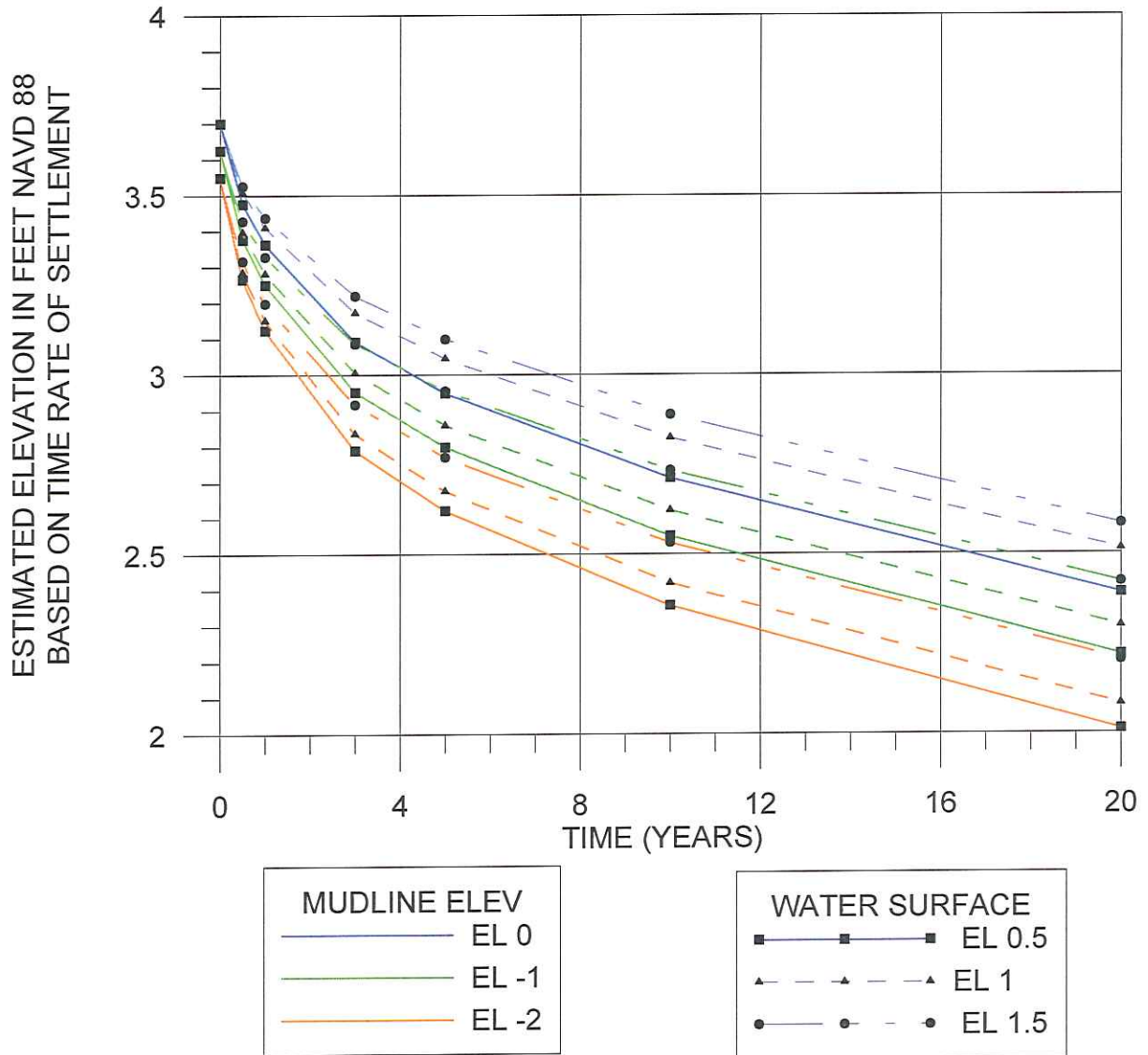
STATE OF LOUISIANA  
RIVERINE SAND MINING  
SCOFIELD ISLAND RETORATION (BA-40)  
PLAQUEMINES PARISH, LOUISIANA

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ESTIMATES OF SETTLEMENT TIME RATE  
CONTAINMENT DIKE  
CENTER OF 20' X 10,000' AREA

SOIL REACH 2  
INITIAL DIKE EL 4 NAVD88  
CONSIDERING SELF-WEIGHT CONSOLIDATION  
AND GEOLOGIC SUBSIDENCE



NOTES:

- 1) PLOT COLOR REPRESENTS MUDLINE ELEVATION.
- 2) PLOT SYMBOL AND LINE TYPE REPRESENT WATER SURFACE ELEVATION.
- 3) SETTLEMENT ESTIMATES SHOULD BE ANTICIPATED TO VARY  $\pm 25\%$ .



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ESTIMATED TIME-RATE OF SETTLEMENT

STATE OF LOUISIANA  
RIVERINE SAND MINING  
SCOFIELD ISLAND RETORATION (BA-40)  
PLAQUEMINES PARISH, LOUISIANA

DRAWN BY: G.P.S. 16 MARCH 2009 FILE: SETL\_TIME\_RATE.GRF

CHECKED BY: W.W.G. JOB NO.: 19292 FIGURE 5 SHEET 8



Assuming a crust approximately 2.5 feet thick for the containment dikes, it was estimated that an additional 3 to 5 inches of settlement will occur. For the marsh areas, minimal shrinkage is anticipated since these features will generally remain saturated. The amount of time for shrinkage to occur will depend on the amount of organic matter present and variations in the moisture content of the fill. Moisture content is dependent on weather conditions, tidal fluctuations, and ground water levels. It was anticipated that shrinkage will occur relatively rapidly due to seasonal variations occurring the first year after fill placement. Due to variations in the organic matter present and moisture ranges, shrinkage will generally result in differential settlement along the dike alignment.

## **8.4 Borrow to Fill Ratios**

### **8.4.1 Proposed Containment Dike Materials**

Estimates of the amount of borrow required to construct the proposed dike section were obtained from the U.S. Army Corps of Engineers based on data compiled on similar projects. Ratios assume the fill material is placed by an excavator and not by pipeline. Based on the available data, a typical borrow-to-dike fill ratio is 2:1 for natural moisture contents ranging from approximately 40% to 60%. For higher moisture contents, a borrow-to-dike fill ratio of 3:1 or more may also occur. The tested water contents may not be indicative of the water contents of the placed material. These borrow-to-dike fill ratios do not include the volume of fill required due to settlement and shrinkage, which should be added to the theoretical volume prior to estimating the borrow required. In addition, any stripping or removal of organic material, which is considered to be unsuitable for the dike section, is not included in the estimated borrow ratio.

### **8.4.2 Proposed Dredged Sediments**

Similar to estimating shrinkage, estimating the borrow-to-fill ratio for dredged sediments is difficult considering the variability of the potential borrow materials encountered. For sandy sediments, a borrow-to-fill ratio between approximately 1.25:1 and 1.5:1 may be used where moisture contents are less than 40%. Refer to the previous paragraph for borrow-to-fill ratios of other soil types.

## **9.0 CONSTRUCTION RECOMMENDATIONS**

### **9.1 Constructability**

Construction techniques are critical to the constructability and ultimate stability of the dike section. The analyses assumed the dike fill is placed as recommended. The stability of the dike

constructed of in situ materials will be dependent on the borrow materials used and the rate at which the dredged fill is placed.

## **9.2 Site Preparation**

Uncompacted fill material may be placed directly over any existing vegetation along the proposed dike alignments. Trees and stumps should be cut to existing grade. However, the root mass should remain in place to minimize disturbance of the subgrade and provide additional stability for the dike.

## **9.3 Water Levels**

Water levels at the Project site are subject to seasonal and tidal fluctuations. Site conditions should be investigated immediately prior to initiating construction. If possible, placement of the initial fill lifts should not be in more than 18 inches of standing water. It is understood this is not always possible. However, when there is more than 18 inches of water, it will be difficult to control the fill because of impaired visibility.

## **9.4 Containment Fill Materials**

The marsh containment dike will be constructed of borrow materials. Large roots and organic matter should not be placed within the dike section. Exposed marsh deposits, such as those identified in Reach 3, are not suitable as fill for the containment dike construction. These materials are generally exposed within existing channels. Thus, as previously noted, construction of the containment dikes should not coincide with these features.

## **9.5 Placement of Uncompacted Fill**

The borrow material will be placed by uncompacted methods for construction of the containment dike. The stability analyses assume these materials will be excavated and placed by mechanical methods using a dragline, clamshell, conventional bucket, or similar mechanical equipment. Uncompacted dike fill should be placed in lifts of no more than 3 feet. Depending on the depth of standing water and moisture content of the borrow materials, consideration should be given to placing an initial fill lift for the entire length of the dike before proceeding to the next lift. This method will initiate consolidation of foundation soils as well as provide a means for the uncompacted fill to provide a sufficient wearing surface. This will also decrease the potential for slope failure within the fill as the dike is constructed.

## **9.6 Staged Dike Construction**

Construction of the marsh containment dike should be performed in stages, particularly in the area of Reach 3. Staged construction will allow consolidation of the subsoils to begin and affect a gain in strength in the rapidly consolidating marsh deposits. This will minimize the potential for lateral plastic deformation of these soils. Staged construction will also minimize localized failures within the un-compacted fill as described above, particularly when these materials remain saturated during initial lift placement. With existing grades generally at -2 feet NAVD88, the dike construction can be performed in three stages.

## **9.7 Marsh Area Fill Materials**

The sediment from SOBA will be excavated by a cutterhead dredge and transported via the sediment pipeline to the marsh fill area. The placement limits of the hydraulic fill should be based on stability considerations (maximum fill heights) as previously presented as well as construction constraints and environmental factors (minimum fill heights). For decanting considerations, fill should be placed no higher than 1 foot below the crown of the containment dikes.

## **9.8 Drainage Controls**

During the placement of the hydraulic fill, the contractor should provide drainage control measures to facilitate construction operations. Drainage control measures could include weirs, pipes, and drop inlets. The number, size, and location of these drainage control measures should be considered during the design of the borrow area and for the permit application. Some deciding factors will include the position of the dredge and floatation canal, natural slope of the land formations, and the type and size of the dredging equipment. Self weight consolidation of the marsh creation fill will create ponding of water at the surface as the settlement occurs over time. Some of this water will evaporate, but consideration should also be given to decanting free surficial water using weirs.

## **9.9 Maintenance**

The stability and settlement analyses do not consider maintenance of the proposed crown elevation for the marsh containment, or beach/dune features. Long term maintenance may be required to accommodate the actual ongoing settlements and other observed movements or degradation. Maintenance may include the addition of materials as a result of accelerated settlement rates or erosion. Similarly, dike degrading to maintain tidal flows in areas where observed settlement is less than anticipated may also be required. A routine evaluation should be made to determine essential maintenance requirements.

## **9.10 Monitoring**

Monitoring the performance of the containment dikes and marsh platform is essential. The use of settlement plates or other surveying methods is recommended to monitor the actual rates of settlement for the project. As indicated previously, the estimates of settlement and settlement time-rate are based on assumed conditions below the boring depths. Natural variations in the materials placed as well as the desiccation and biodegradation of these deposits may also affect the estimates. In addition, construction of the containment area may affect water levels due to tidal fluctuations in the Project area. If long term performance of the fill placement is to be evaluated, the monitoring should be performed at regular intervals to provide sufficient data.

## **10.0 REFERENCES**











Eustis Engineering Services, LLC. 2009. Riverine Sand Mining / Scofield Island Restoration (BA-40), Geotechnical Investigation, Plaquemines Parish, LA. Back-barrier geotechnical investigation of Scofield Island. Submitted to SJB Group, LLC. Baton Rouge, Louisiana.

## **ANNEX K1**

### **BORING LOGS AND LABORATORY TEST DATA**



**LEGEND AND NOTES FOR  
LOG OF BORING AND TEST RESULTS**

PP	Pocket penetrometer: Resistance in tons per square foot					
SPT	Standard Penetration Test: Number of blows of a 140-lb hammer dropped 30 inches required to drive 2-in. O.D., 1.4-in. I.D. sampler a distance of 1 foot into the soil after first seating it 6 inches					
SPLR	Type of Sampling	 Shelby	 SPT	 Auger	 No sample	
SYMBOL	Clay	Silt	Sand	Peat/Humus	Shells	Stone/Gravel
						
	Predominant type shown heavy; Modifying type shown light					
USC	Unified Soil Classification					
DENSITY	Unit weight in pounds per cubic foot					

**SHEAR TESTS**

TYPE

UC	Unconfined compression shear
OB	Unconsolidated undrained triaxial compression shear on one specimen confined at the approximate overburden pressure
UU	Unconsolidated undrained triaxial compression shear
CU	Consolidated undrained triaxial compression shear
DS	Direct shear

$\phi$	Angle of internal friction in degrees
c	Cohesion in pounds per square foot

**ATTERBERG LIMITS**

LL	Liquid Limit
PL	Plastic Limit
PI	Plasticity Index

**OTHER TESTS**

CON	Consolidation
PD	Particle size distribution (sieve and/or hydrometer)
k	Coefficient of permeability in centimeters per second
SP	Swelling pressure in pounds per square foot

Other laboratory test results reported on separate figures

**GENERAL NOTES**

- (1) If a ground water depth is shown on the boring log, these observations were made at the time of drilling and were measured below the existing ground surface. These observations are shown on the boring logs. However, ground water levels may vary due to seasonal fluctuations and other factors. If important to construction, the depth to ground water should be determined by those persons responsible for construction immediately prior to beginning work.
- (2) While the individual logs of borings are considered to be representative of subsurface conditions at their respective locations on the dates shown, it is not warranted that they are representative of subsurface conditions at other locations and times.

# LOG OF BORING AND TEST RESULTS

STATE OF LOUISIANA

RIVERINE SAND MINING/SCOFIELD ISLAND RESTORATION (BA-40)  
PLAQUEMINES PARISH, LOUISIANA



Ground Elev.: -3.4 Datum: NAVD88 Gr. Water Depth: Job No.: 19292 Date Drilled: 10/08/08 Boring: 1 Refer to "Legends & Notes"

Scale In Feet	PP	SPT	S P L R	Symbol	Visual Classification	USC	Sample Number	Depth In Feet	Water Content Percent	Density		Shear Tests			Atterberg Limits			Other Tests
										Dry	Wet	Type	σ	C	LL	PL	PI	
0		4	X	[Symbol]	Very loose gray silty sand w/shell fragments	SM	1	0-1.5	27									
		6	X	[Symbol]	Loose gray fine sand w/silt & shell fragments	SP	2	2.5-4										SV
		WOH	X	[Symbol]	Very soft gray clay w/organic matter & shell fragments	CH	3	5.5-7	90									
				[Symbol]	Extremely soft gray organic clay w/wood	OH	4	8-9	130	38	86	OB	0	101				
10				[Symbol]	Extremely soft gray clay w/organic matter, wood, & shell fragments	CH	5	11-12	174	29	81	UC	--	58				
				[Symbol]	w/silt pockets		6	14-15	120	40	88	OB	0	90	114	27	87	CONS
20				[Symbol]	Loose to medium dense gray silty sand	SM	7	19-20	63	63	102	UC	--	53				
0.25				[Symbol]			8	24-25	28	96	122	OB	0	1082	23	20	3	
30				[Symbol]	Very soft gray clay w/silty sand lenses & layers	CH	9	29-30	23									
0.25				[Symbol]	Loose to medium dense gray silty sand	SM	10	34-35	58	66	104	OB	0	207				
40	0.25			[Symbol]	Very soft to soft gray silty clay	CL	11	39-40	29	92	119	OB	0	543				
0.25				[Symbol]	Soft gray clay w/silt lenses	CH	12	44-45	42	79	112	UC	--	182	42	19	23	
50	0.25			[Symbol]			13	49-50	60	65	103	UC	--	384				

Comments: Latitude: 29° 14.809' N  
Longitude: 89° 33.423' W  
3 feet of water

# LOG OF BORING AND TEST RESULTS

STATE OF LOUISIANA

RIVERINE SAND MINING/SCOFIELD ISLAND RESTORATION (BA-40)  
PLAQUEMINES PARISH, LOUISIANA



Ground Elev.: -1.85 Datum: NAVD88 Gr. Water Depth: Job No.: 19292 Date Drilled: 10/11/08 Boring: 2 Refer to "Legends & Notes"

Scale In Feet	PP	SPT	S P L R Symbol	Visual Classification	USC	Sample Number	Depth In Feet	Water Content Percent	Density		Shear Tests			Atterberg Limits			Other Tests		
									Dry	Wet	Type	φ	C	LL	PL	PI			
0				Extremely soft gray & brown organic clay w/wood	OH	1	1-2	120	40	88	OB	0	32	110	30	80	ORG CONT = 6.51		
				Extremely soft to very soft gray & brown organic clay w/wood	OH	2	4-5	135	36	86	UC	--	14						
				Extremely soft gray clay w/wood	CH	3	7-8	124	39	87	OB	0	46						
10					Extremely soft gray clay w/wood	CH	4	10-11	85	51	95	UC	--	62				ORG CONT = 6.33	
					Extremely soft to very soft gray clay w/silty clay layers	CH	5	13-14	81	52	95	OB	0	109	86	26	60		
0.10					Extremely soft to very soft gray clay w/silty clay layers	CH	6	18-19	79	54	96	UC	--	86					
20					Very loose gray fine sand	SP	7	23-24	22										
		5			Loose gray sandy silt	ML	8	25.5-27										SV	
		6			Loose gray fine sand w/silt	SP-SM	9	28.5-30	23										
30					Loose gray fine sand w/silt	SP-SM	9	28.5-30	23										
0.10					Soft gray silty clay w/clay lenses & fine sand layers	CL	10	33-34	43	77	111	OB	0	289	37	19	18		
0.10					Soft gray silty clay w/clay lenses & fine sand layers	CL	10	33-34	43	77	111	OB	0	289	37	19	18		
40					Soft gray silty clay w/clay lenses & fine sand layers	CL	10	33-34	43	77	111	OB	0	289	37	19	18		
0.10				Soft gray silty clay w/clay lenses & fine sand layers	CL	10	33-34	43	77	111	OB	0	289	37	19	18			
0.25				Soft gray clay w/silt lenses	CH	12	43-44	66	60	100	OB	0	433						
0.25				Soft gray clay w/silt lenses	CH	12	43-44	66	60	100	OB	0	433						
50				Soft gray clay w/silt lenses	CH	12	43-44	66	60	100	OB	0	433						
				Soft gray clay w/silt lenses	CH	13	48-49	59	65	104	UC	--	250						

Comments: Latitude: 29° 14.491' N  
Longitude: 89° 31.915' W  
3 feet of water



# LOG OF BORING AND TEST RESULTS

STATE OF LOUISIANA

RIVERINE SAND MINING/SCOFIELD ISLAND RESTORATION (BA-40)  
PLAQUEMINES PARISH, LOUISIANA



Ground Elev.: -1.6 Datum: NAVD88 Gr. Water Depth: Job No.: 19292 Date Drilled: 10/09/08 Boring: 3 Refer to "Legends & Notes"

Scale In Feet	PP	SPT	S P L R	Symbol	Visual Classification	USC	Sample Number	Depth In Feet	Water Content Percent	Density		Shear Tests			Atterberg Limits			Other Tests
										Dry	Wet	Type	σ	C	LL	PL	PI	
0		3	X	[Dotted]	Very loose gray fine sand w/silt	SP-SM	1	0-1.5	34									
		2	X	[Dotted]			2	2.5-4										SV
		3	X	[Dotted]	w/shells		3	5-6.5	41									
10				[Diagonal]	Extremely soft gray clay w/organic matter & wood	CH	4	8-9	118	40	88	OB	0	87				
				[Diagonal]			5	11-12	109	43	89	UC	--	100	119	29	90	ORG CONT = 7.08
				[Diagonal]	Extremely soft to very soft gray & dark gray organic clay w/wood	OH	6	14-15	158	31	81	OB	0	114				
20				[Diagonal]	Extremely soft gray clay w/silty clay layers	CH	7	19-20	77	55	97	UC	--	96				
				[Dotted]	Loose gray fine sand w/clay lenses	SP	8	24-25	22	103	125	OB	0	401	NP	NP		CONS
0.25				[Diagonal]	Very soft gray clay w/silty sand lenses	CH	9	29-30	64	62	102	UC	--	167				
30		0.25		[Diagonal]	Very soft gray silty clay w/sandy silt lenses	CL	10	34-35	46	75	110	OB	0	218				
		0.25		[Diagonal]			11	39-40	63	62	102	UC	--	255	77	26	51	
40		0.25		[Diagonal]	Soft gray clay w/silt lenses	CH	12	44-45	50	72	108	OB	0	332				
		0.25		[Diagonal]			13	48.5-49.5	56	67	105	UC	--	271				
50		0.50		[Diagonal]	w/silty sand lenses													

Comments: Latitude: 29° 14.408' N  
Longitude: 89° 32.205' W  
2 feet of water

# LOG OF BORING AND TEST RESULTS

STATE OF LOUISIANA

RIVERINE SAND MINING/SCOFIELD ISLAND RESTORATION (BA-40)  
PLAQUEMINES PARISH, LOUISIANA



Ground Elev.: -4.4 Datum: NAVD88 Gr. Water Depth: Job No.: 19292 Date Drilled: 10/07/08 Boring: 4 Refer to "Legends & Notes"

Scale In Feet	PP	SPT	S P L R	Symbol	Visual Classification	USC	Sample Number	Depth In Feet	Water Content Percent	Density		Shear Tests			Atterberg Limits			Other Tests
										Dry	Wet	Type	σ	C	LL	PL	PI	
0		9	X	[Dotted]	Loose gray fine sand w/silt & shells	SP	1	0-1.5										SV
		7	X	[Dotted]			2	2.5-4	24									
		6	X	[Dotted]			3	5.5-7										SV
		12	X	[Dotted]	Medium dense gray fine sand w/silt & shells	SP-SM	4	8.5-10	23									
10				[Vertical Lines]	Loose gray silty sand w/shells	SM												
		6	X	[Vertical Lines]			5	13.5-15										SV
				[Diagonal Lines]	Soft gray sandy clay w/shells	CL												
		2	X	[Diagonal Lines]			6	18.5-20	52									
				[Diagonal Lines]	Very soft to soft gray clay w/silty sand pockets	CH												
	0.25			[Diagonal Lines]			7	24-25	60	65	103	UC	--	234	76	20	56	
				[Diagonal Lines]	w/silty sand pockets & shell fragments													
	0.25			[Diagonal Lines]			8	29-30	64	62	101	OB	0	234				
				[Diagonal Lines]	w/sand lenses													
	0.50			[Dotted]	Loose gray fine sand w/clay lenses	SP	9	34-35	71	58	99	UC	--	335				
				[Dotted]														
	0.25			[Vertical Lines]	Loose gray silty sand w/clay lenses	SM	10	39-40	25	98	123	OB	0	464	16	15	1	
				[Vertical Lines]														
	0.25			[Vertical Lines]			11	44-45	30	92	120	UC	--	394				
				[Vertical Lines]														
				[Diagonal Lines]	Soft gray clay w/alternating silty sand lenses	CH												
				[Diagonal Lines]														
50		0.25		[Diagonal Lines]			12	49-50	43	79	112	OB	0	439	73	20	53	

Comments: Latitude: 29° 14.667' N  
Longitude: 89° 33.662' W

# LOG OF BORING AND TEST RESULTS

STATE OF LOUISIANA  
 RIVERINE SAND MINING/SCOFIELD ISLAND RESTORATION (BA-40)  
 PLAQUEMINES PARISH, LOUISIANA



Ground Elev.: -2.9 Datum: NAVD88 Gr. Water Depth: Job No.: 19292 Date Drilled: 10/11/08 Boring: 5 Refer to "Legends & Notes"

Scale In Feet	PP	SPT	S P L R	Symbol	Visual Classification	USC	Sample Number	Depth In Feet	Water Content Percent	Density		Shear Tests			Atterberg Limits			Other Tests	
										Dry	Wet	Type	σ	C	LL	PL	PI		
0		7	X		Loose gray fine sand w/shell fragments	SP	1	0-1.5										SV	
					Extremely soft gray clay w/organic matter	CH	2	4-5	116	41	88	OB	0	48	117	28	89	CONS, ORG CONT = 7.07 ORG CONT = 14.84 ORG CONT = 8.21 ORG CONT = 7.30	
					Very soft gray & brown organic clay w/wood	OH	3	7-8	117										
10					Extremely soft gray clay w/decayed wood & roots	CH	4	10-11	141	35	85	UC	--	71	138	31	107		
					Extremely soft gray clay	CH	5	13-14	106	44	90	OB	0	55					
					Loose to medium compact gray clayey silt w/silty sand layers	ML	6	18-19	98	97	92	UC	--	19	98	23	75		
					Loose to medium compact gray clayey silt w/silty sand layers	ML	7	23-24	35	86	116	OB	0	599					
30	0.10				Very soft gray clay w/silty sand lenses & layers	CH	8	28-29	58	66	104	UC	--	134					
					Soft gray silty clay w/decayed wood	CL	9	33-34	42	79	112	OB	0	278					
40	0.10				Very soft to soft gray clay	CH	10	38-39	55	68	106	UC	--	194	67	18	49		CONS
	0.25				Very soft to soft gray clay	CH	11	43-44	56	66	103	OB	0	141					
50	0.25				Very soft to soft gray clay	CH	12	48-49	64	61	100	UC	--	288					

Comments: Latitude: 29° 14.472' N'  
 Longitude: 89° 33.028' W  
 2 feet of water

# LOG OF BORING AND TEST RESULTS

STATE OF LOUISIANA

RIVERINE SAND MINING/SCOFIELD ISLAND RESTORATION (BA-40)  
PLAQUEMINES PARISH, LOUISIANA

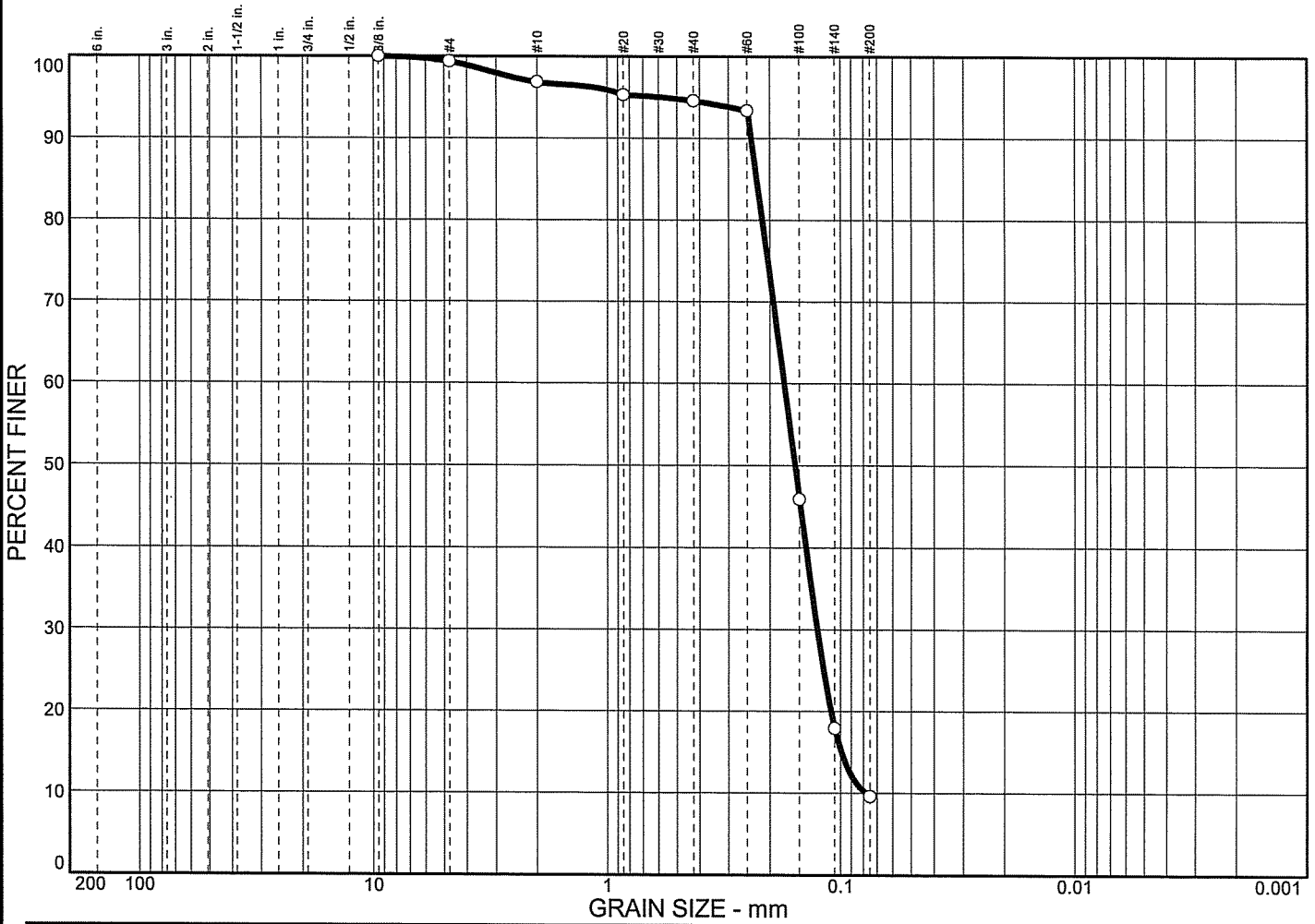


Ground Elev.: -4.3 Datum: NAVD88 Gr. Water Depth: Job No.: 19292 Date Drilled: 10/08/08 Boring: 6 Refer to "Legends & Notes"

Scale In Feet	PP	SPT	S P L R	Symbol	Visual Classification	USC	Sample Number	Depth In Feet	Water Content Percent	Density		Shear Tests			Atterberg Limits			Other Tests
										Dry	Wet	Type	φ	C	LL	PL	PI	
0		WOH	X		Extremely soft to very soft gray organic clay w/wood, decayed wood, & roots	OH	1	0-1.5	145									ORG CONT = 11.17 ORG CONT = 10.32 ORG CONT = 3.84
					Extremely soft gray silty clay w/decayed wood	CL	2	2-3	137	36	84	OB	0	79				
					Very soft gray sandy clay w/decayed wood	CL	3	5-6	58	74	109	UC	--	89				
							4	8-9	44	77	111	OB	0	190				
10							5	11-12	43	79	112	UC	--	166	41	23	18	
					Very soft to soft brown silty clay w/trace of decayed wood	CL	6	14-15	40	82	114	OB	0	190				
20	0.25						7	19-20	38	83	115	UC	--	323				
	0.50				Loose to medium dense gray fine sand	SP	8	24-25	39	82	114	OB	0	214	42	22	20	
30	0.50				Very soft to soft gray clay	CH	9	29-30	27	97	123	OB	0	5929				
	0.50				Medium stiff gray sandy clay	CL	10	34-35	61	64	103	OB	0	204				
40	0.50				Medium stiff gray silty clay	CL	11	39-40	27	98	124	UC	--	506	32	14	18	
	0.50				Soft gray silty clay	CL	12	44-45	33	90	119	OB	0	676				
50	0.50						13	49-50	39	82	114	UC	--	283				

Comments: Latitude: 29° 14.118' N  
Longitude: 89° 31.945' W

# Particle Size Distribution Report



% COBBLES	% GRAVEL		% SAND			% FINES			
	CRS.	FINE	CRS.	MEDIUM	FINE	SILT	CLAY		
○	0.0	0.6	2.5	2.3	85.0	9.6			
LL	PL	D85	D60	D50	D30	D15	D10	Cc	Cu
○		0.228	0.174	0.157	0.126	0.0994	0.0781	1.17	2.23

MATERIAL DESCRIPTION	USCS	AASHTO
○ LO G FISA W/ SI & SH FRAG	SP-SM	

**Project No.** 19292      **Client:** LOUISIANA, STATE OF  
**Project:** RIVERINE SAND MINING/SCOFIELD ISLAND RESTORATION (BA-40)  
  
 ○ **Source:** BR-1                      **Sample No.:** 2                      **Elev./Depth:** 2.5

**Remarks:**  
 ○



Figure

# Particle Size Distribution Report



%	% GRAVEL		% SAND			% FINES				
	CRS.	FINE	CRS.	MEDIUM	FINE	SILT	CLAY			
○	0.0	0.0	0.0	0.0	44.8	55.2				
×	LL	PL	D <sub>85</sub>	D <sub>60</sub>	D <sub>50</sub>	D <sub>30</sub>	D <sub>15</sub>	D <sub>10</sub>	C <sub>c</sub>	C <sub>u</sub>
○			0.152	0.0886						
MATERIAL DESCRIPTION								USCS	AASHTO	
○ LOG SASI								ML		

**Project No.** 19292      **Client:** LOUISIANA, STATE OF  
**Project:** RIVERINE SAND MINING/SCOFIELD ISLAND RESTORATION (BA-40)  
 ○ **Source:** BR-2                      **Sample No.:** 8                      **Elev./Depth:** 25.5

**Remarks:**  
 ○



Figure

# Particle Size Distribution Report



%	% GRAVEL		% SAND			% FINES				
	CRS.	FINE	CRS.	MEDIUM	FINE	SILT	CLAY			
○			5.8	4.6	75.3	13.5				
×	LL	PL	D <sub>85</sub>	D <sub>60</sub>	D <sub>50</sub>	D <sub>30</sub>	D <sub>15</sub>	D <sub>10</sub>	C <sub>c</sub>	C <sub>u</sub>
○			0.208	0.138	0.126	0.104	0.0783			

MATERIAL DESCRIPTION							USCS	AASHTO
○ VLO G FISA W/ SI							SP-SM	

<p><b>Project No.</b> 19292      <b>Client:</b> LOUISIANA, STATE OF</p> <p><b>Project:</b> RIVERINE SAND MINING/SCOFIELD ISLAND RESTORATION (BA-40)</p> <p>○ <b>Source:</b> BR-3                      <b>Sample No.:</b> 2                      <b>Elev./Depth:</b> 2.5</p>	<p><b>Remarks:</b></p> <p>○</p>
<b>EUSTIS</b> Metairie, Louisiana Lafayette, Louisiana Gulfport, Mississippi	

Figure

# Particle Size Distribution Report



% COBBLES	% GRAVEL		% SAND			% FINES			
	CRS.	FINE	CRS.	MEDIUM	FINE	SILT	CLAY		
○	0.0	0.0	2.3	4.5	87.5	5.1			
LL	PL	D85	D60	D50	D30	D15	D10	C <sub>c</sub>	C <sub>u</sub>
○		0.234	0.182	0.164	0.135	0.113	0.100	1.00	1.81

MATERIAL DESCRIPTION	USCS	AASHTO
○ LO G FISA W/ SI, SHELLS	SP-SM	

Project No. 19292	Client: LOUISIANA, STATE OF
Project: RIVERINE SAND MINING/SCOFIELD ISLAND RESTORATION (BA-40)	
○ Source: BR-4	Sample No.: 1
	Elev./Depth: 0
<b>EUSTIS</b> Metairie, Louisiana Lafayette, Louisiana Gulfport, Mississippi	

Remarks:
○

Figure



# Particle Size Distribution Report



% COBBLES	% GRAVEL		% SAND			% FINES	
	CRS.	FINE	CRS.	MEDIUM	FINE	SILT	CLAY
○	0.0	3.7	13.0	15.5	60.0	7.8	

LL	PL	D <sub>85</sub>	D <sub>60</sub>	D <sub>50</sub>	D <sub>30</sub>	D <sub>15</sub>	D <sub>10</sub>	C <sub>c</sub>	C <sub>u</sub>
○		2.22	0.221	0.183	0.137	0.111	0.0876	0.97	2.52

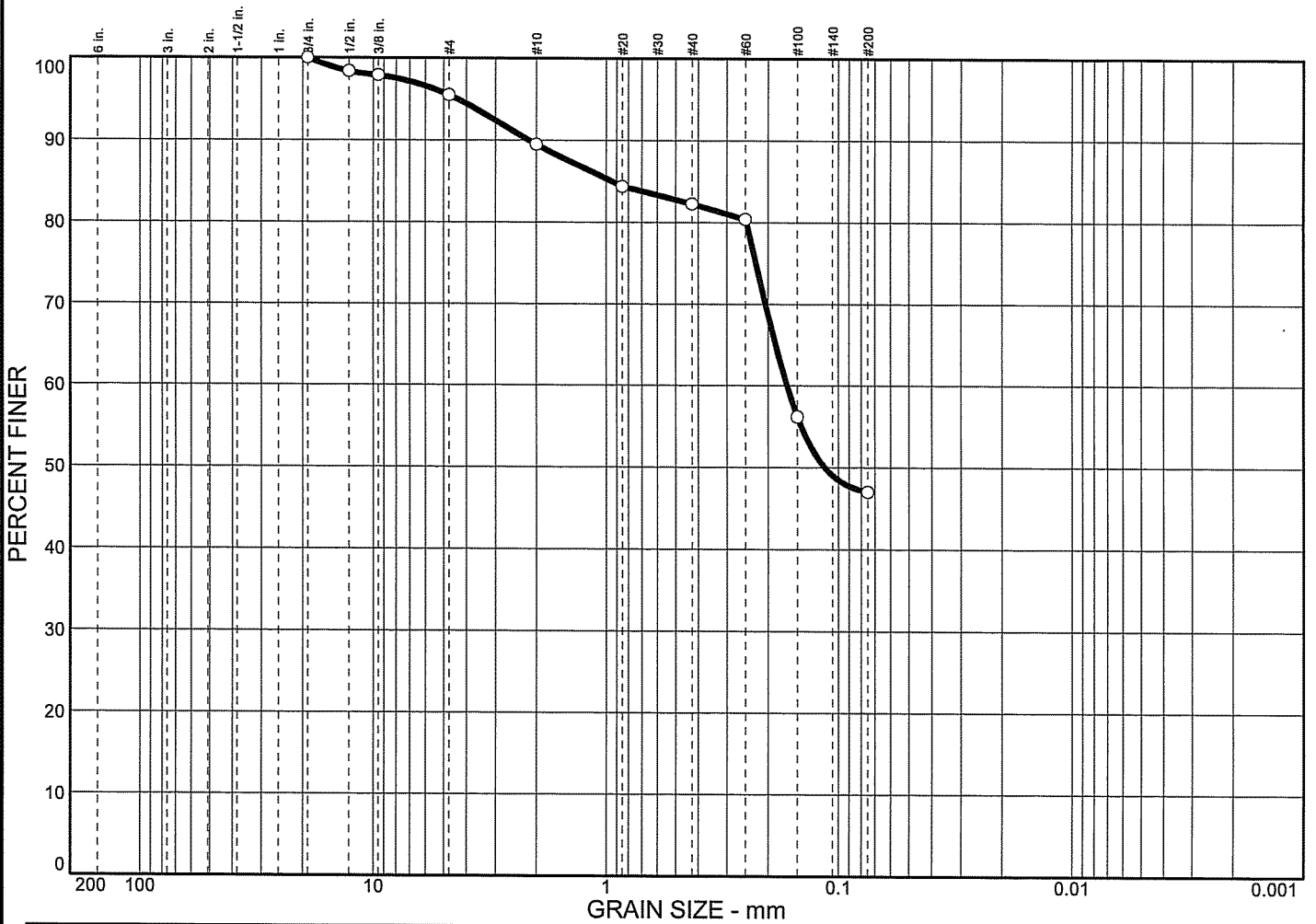
MATERIAL DESCRIPTION							USCS	AASHTO
○ LO G FISA W/ SI, SHELLS							SP-SM	

<b>Project No.</b> 19292 <b>Client:</b> LOUISIANA, STATE OF <b>Project:</b> RIVERINE SAND MINING/SCOFIELD ISLAND RESTORATION (BA-40)  ○ <b>Source:</b> BR-4 <b>Sample No.:</b> 3 <b>Elev./Depth:</b> 5.5	<b>Remarks:</b> ○
---	----------------------



Figure

# Particle Size Distribution Report



% COBBLES	% GRAVEL		% SAND			% FINES			
	CRS.	FINE	CRS.	MEDIUM	FINE	SILT	CLAY		
○	0.0	4.5	6.0	7.2	35.3	47.0			
LL	PL	D85	D60	D50	D30	D15	D10	C <sub>c</sub>	C <sub>u</sub>
○		0.937	0.166	0.114					

MATERIAL DESCRIPTION	USCS	AASHTO
○ LO G SISA W/ SHELLS	SM	

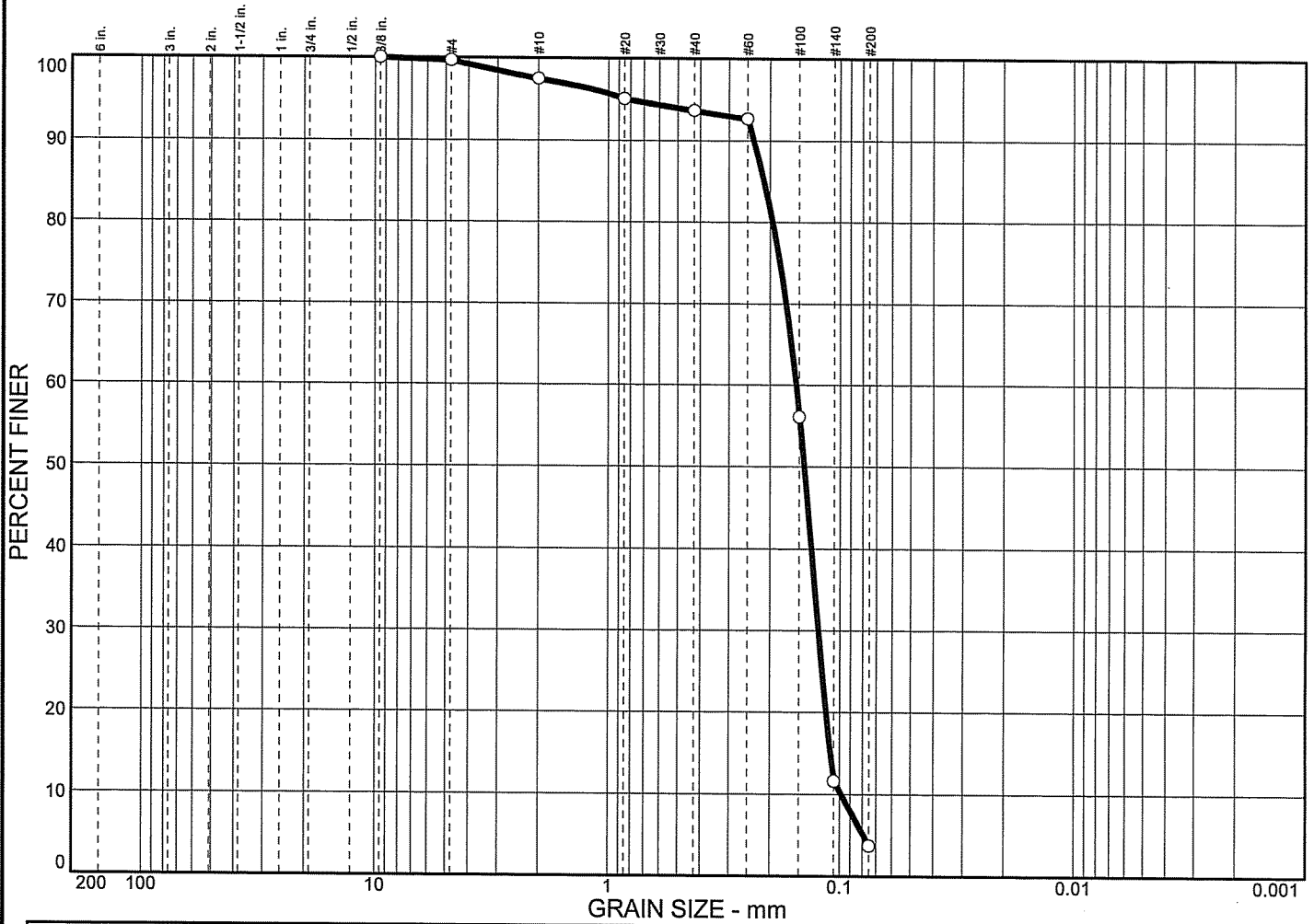
**Project No.** 19292      **Client:** LOUISIANA, STATE OF  
**Project:** RIVERINE SAND MINING/SCOFIELD ISLAND RESTORATION (BA-40)  
 ○ **Source:** BR-4      **Sample No.:** 5      **Elev./Depth:** 13.5

**Remarks:**  
 ○



Figure

# Particle Size Distribution Report



%	% GRAVEL		% SAND			% FINES				
	CRS.	FINE	CRS.	MEDIUM	FINE	SILT	CLAY			
○	0.0	0.3	2.2	3.8	90.0	3.7				
×	LL	PL	D <sub>85</sub>	D <sub>60</sub>	D <sub>50</sub>	D <sub>30</sub>	D <sub>15</sub>	D <sub>10</sub>	C <sub>c</sub>	C <sub>u</sub>
○			0.213	0.155	0.143	0.124	0.110	0.0988	1.01	1.57

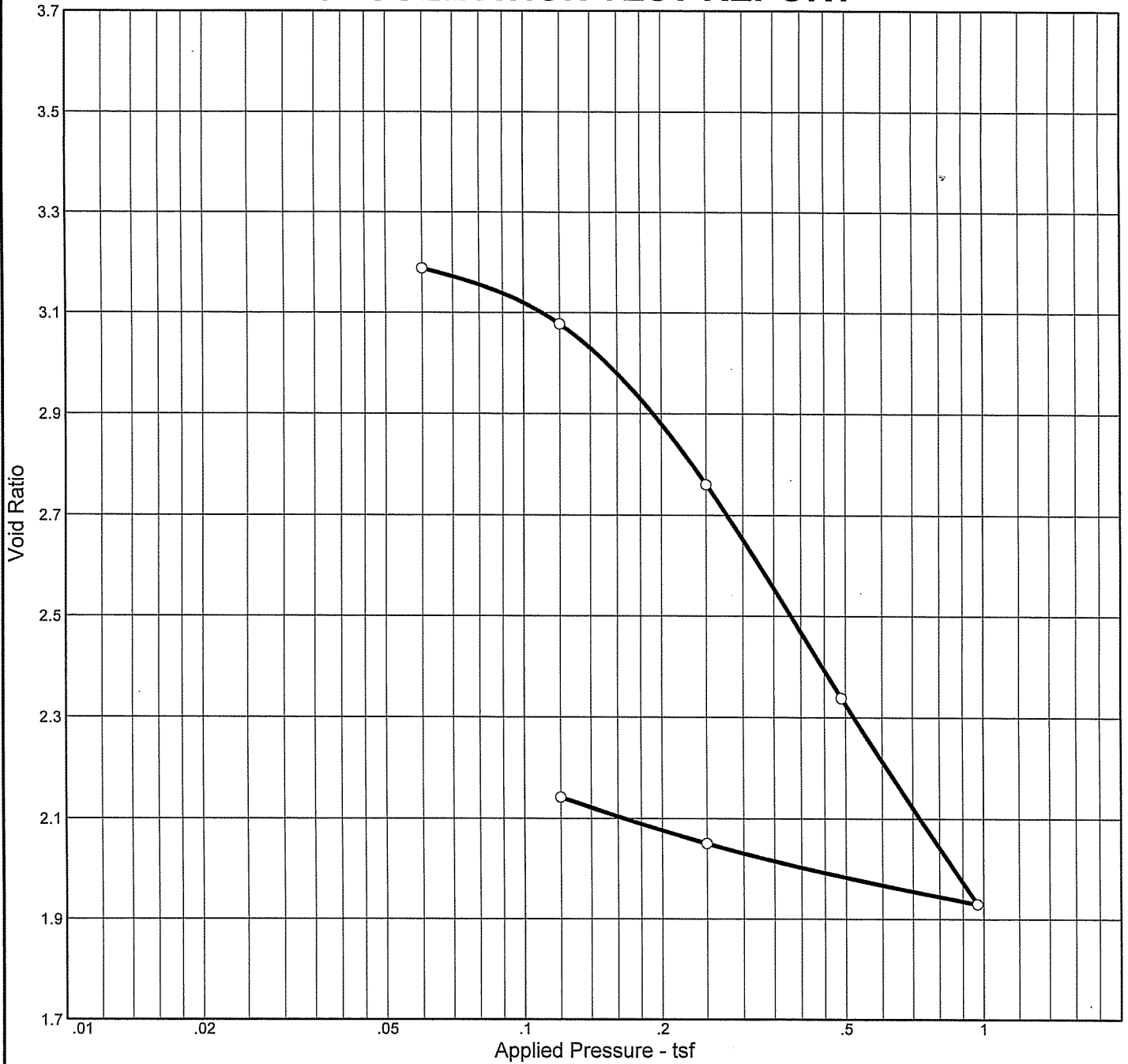
MATERIAL DESCRIPTION							USCS	AASHTO
○ LO G FISA W/ SH FRAG							SP	

Project No. 19292	Client: LOUISIANA, STATE OF	Remarks: ○
Project: RIVERINE SAND MINING/SCOFIELD ISLAND RESTORATION (BA-40)		
○ Source: BR-5	Sample No.: 1      Elev./Depth: 0	



Figure

# CONSOLIDATION TEST REPORT



Natural Sat.	Moist.	Dry Dens. (pcf)	LL	PI	Sp. Gr.	Overburden (tsf)	P <sub>c</sub> (tsf)	C <sub>c</sub>	C <sub>r</sub>	Swell Press. (tsf)	Swell %	e <sub>0</sub>
98.9 %	122.1 %	39.0	117	87	2.74		0.17	1.45	0.25			3.385

MATERIAL DESCRIPTION	USCS	AASHTO
XSO G CL W/ OM, WD, SH FRAG	CH	

**Project No.** 19292      **Client:** SJB GROUP, LLC  
**Project:** LOUISIANA, STATE OF - RIVERINE SAND MINING/SCOFIELD ISLAND RESTORATION (BA-40), PLAQUEMINES PARISH, LOUISIANA  
**Source:** BR-1      **Sample No.:** 6      **Elev./Depth:** 14'

**Remarks:**  
 TESTED BY: RR  
 CHECKED BY: LR



Figure

# Dial Reading vs. Time

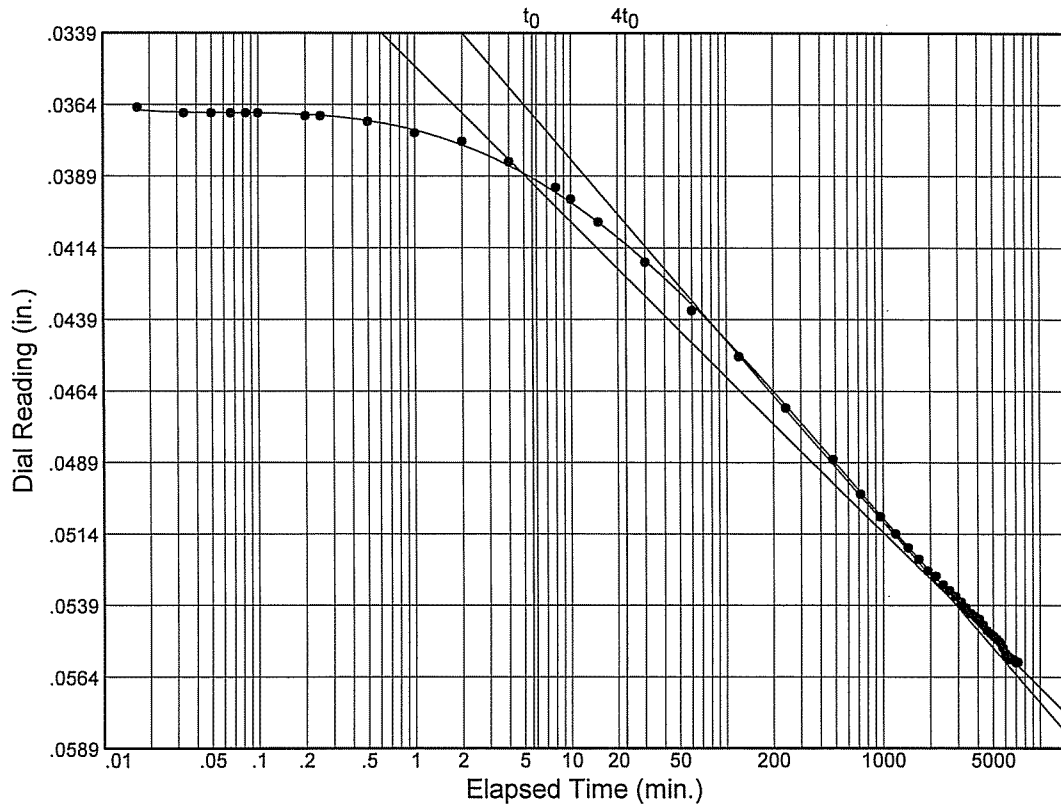
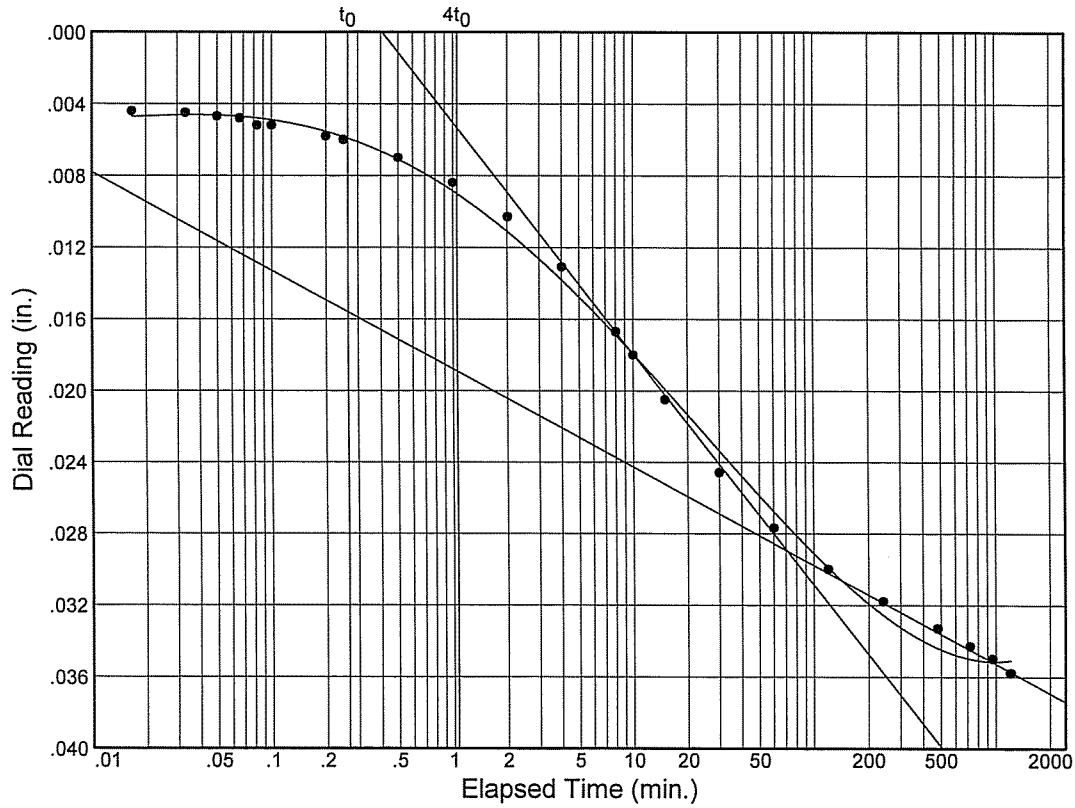
Project No.: 19292

Project: LOUISIANA, STATE OF - RIVERINE SAND MINING/SCOFIELD ISLAND RESTORATION (BA-

Source: BR-1

Sample No.: 6

Elev./Depth: 14'



# Dial Reading vs. Time

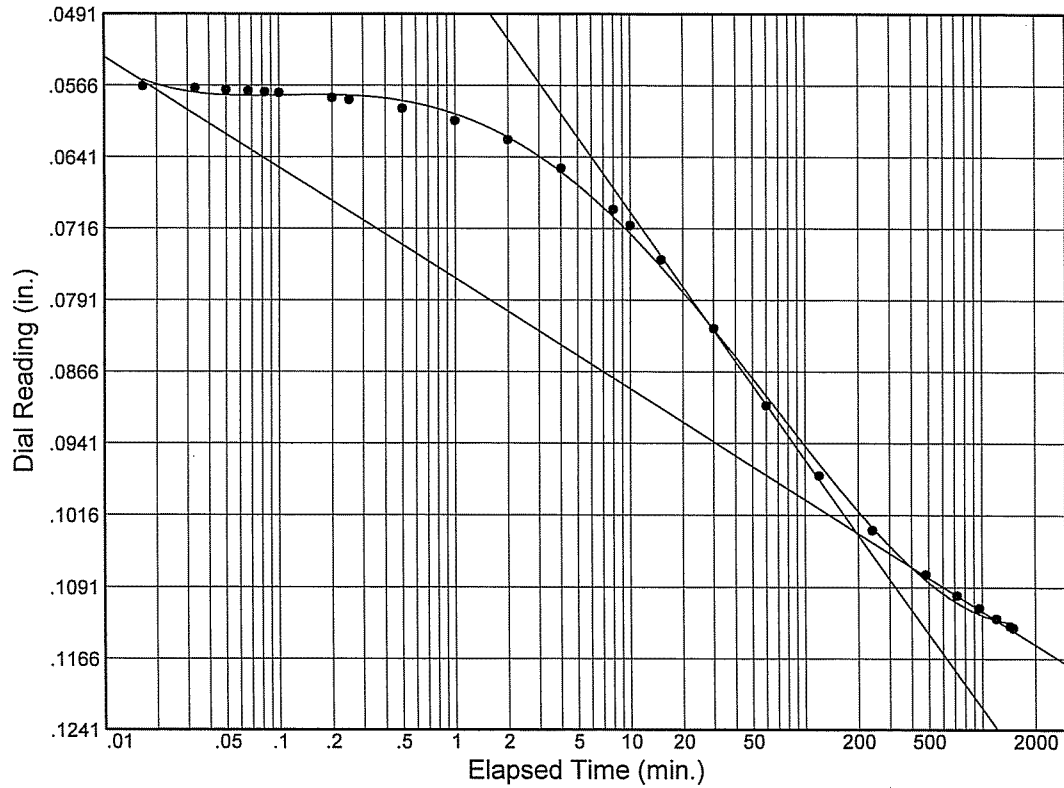
Project No.: 19292

Project: LOUISIANA, STATE OF - RIVERINE SAND MINING/SCOFIELD ISLAND RESTORATION (BA-

Source: BR-1

Sample No.: 6

Elev./Depth: 14'



Load No.= 3

Load= 0.25 tsf

$D_0 = 0.05600$

$D_{50} = 0.07970$

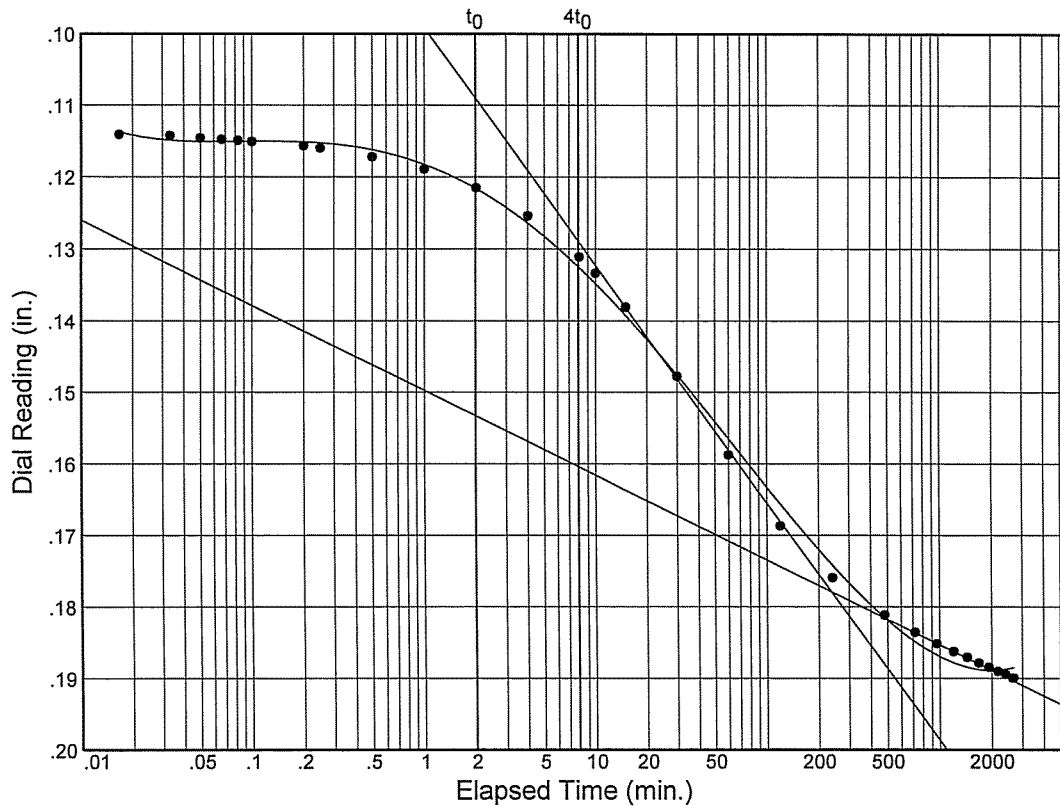
$D_{100} = 0.10339$

$T_{50} = 23.35$  min.

$C_v @ T_{50}$

0.01 ft.<sup>2</sup>/day

$C_\alpha = 0.016$



Load No.= 4

Load= 0.49 tsf

$D_0 = 0.11066$

$D_{50} = 0.14429$

$D_{100} = 0.17792$

$T_{50} = 22.93$  min.

$C_v @ T_{50}$

0.01 ft.<sup>2</sup>/day

$C_\alpha = 0.017$

# Dial Reading vs. Time

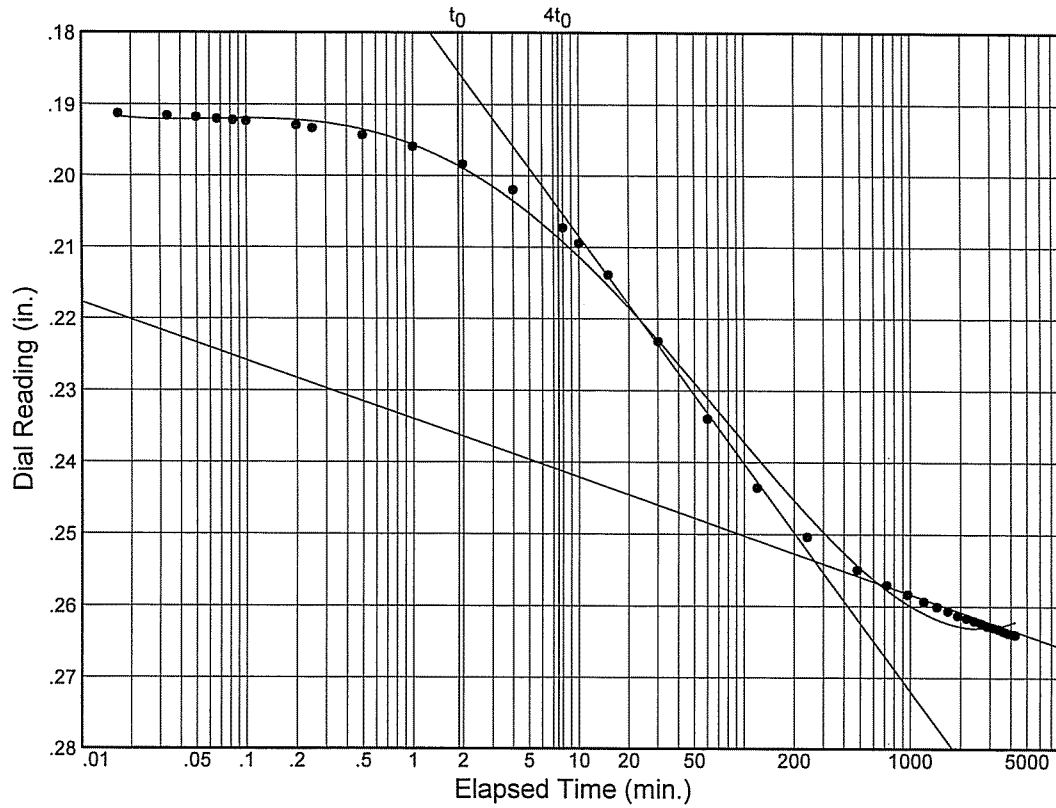
Project No.: 19292

Project: LOUISIANA, STATE OF - RIVERINE SAND MINING/SCOFIELD ISLAND RESTORATION (BA-

Source: BR-1

Sample No.: 6

Elev./Depth: 14'



Load No.= 5

Load= 0.97 tsf

$D_0 = 0.18853$

$D_{50} = 0.22111$

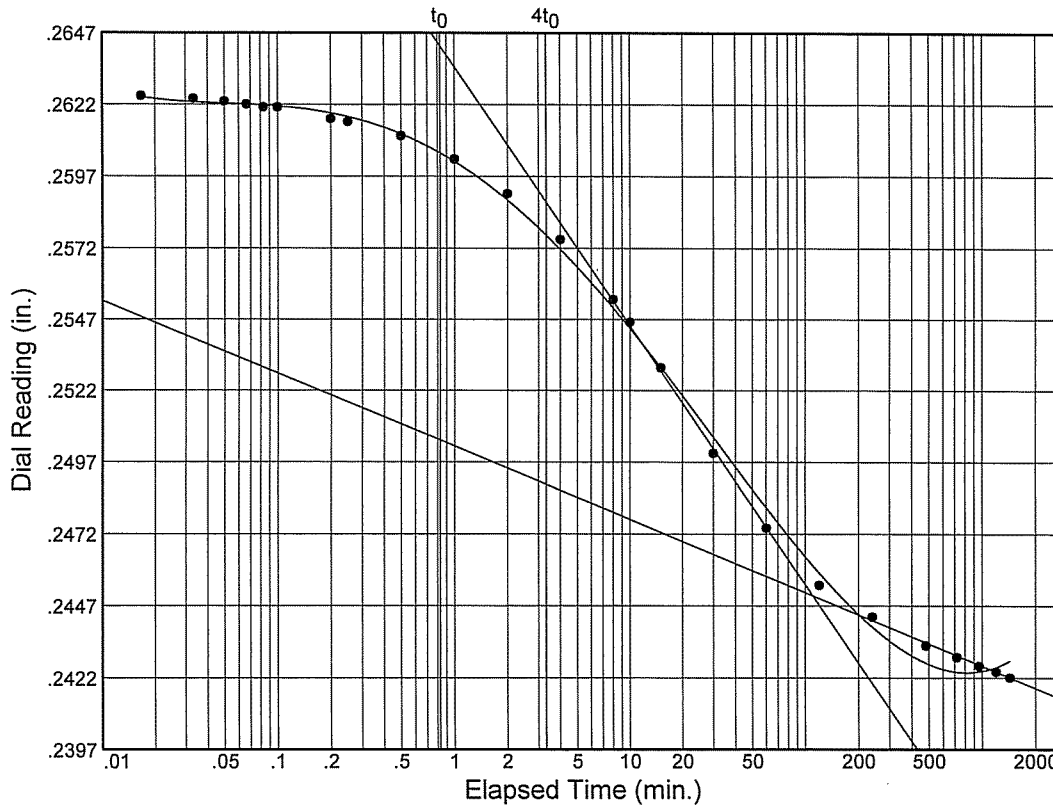
$D_{100} = 0.25370$

$T_{50} = 25.37$  min.

$C_v @ T_{50}$

0.01 ft.<sup>2</sup>/day

$C_\alpha = 0.013$



Load No.= 6

Load= 0.25 tsf

$D_0 = 0.26336$

$D_{50} = 0.25421$

$D_{100} = 0.24505$

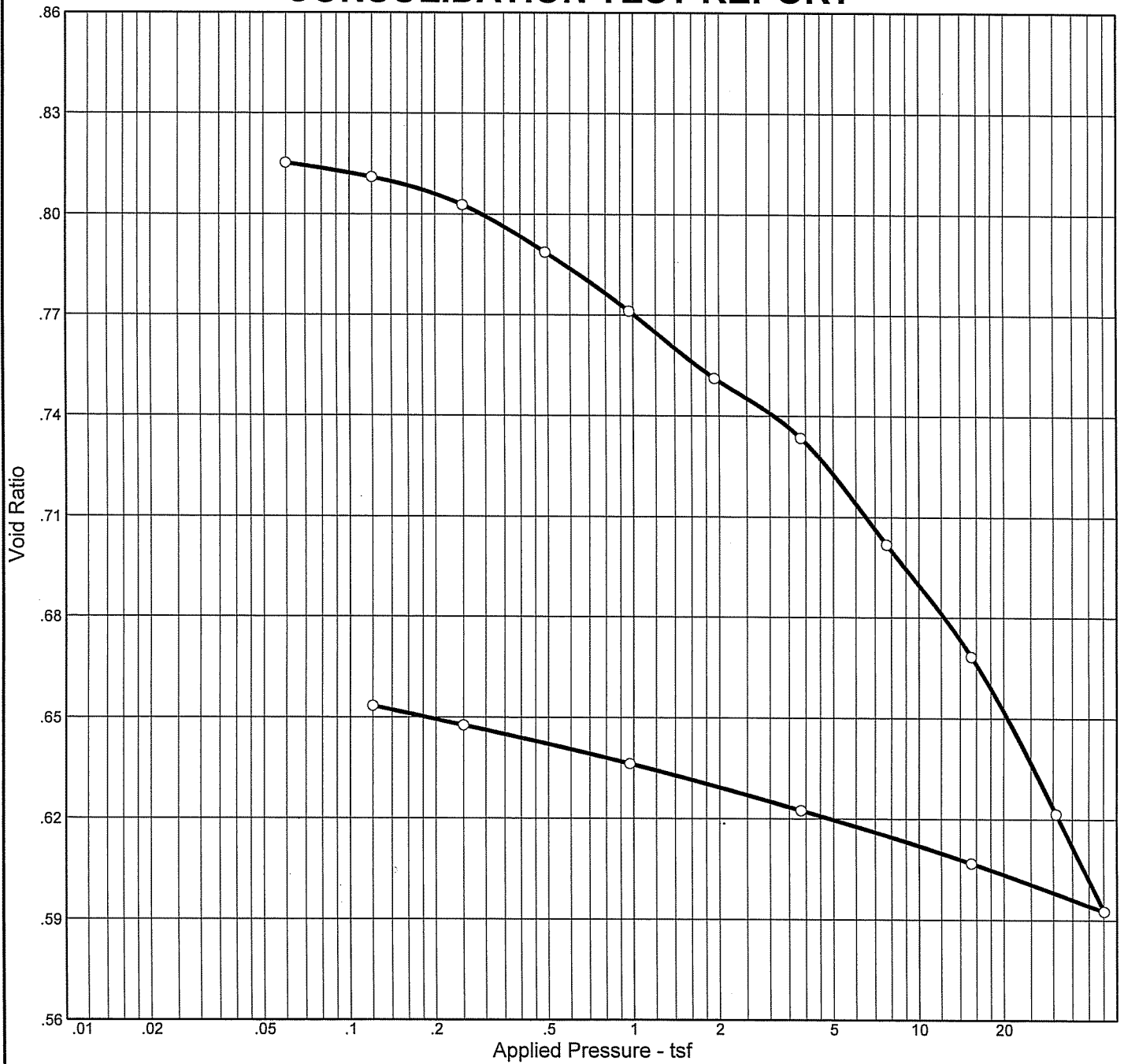
$T_{50} = 10.53$  min.

$C_v @ T_{50}$

0.01 ft.<sup>2</sup>/day



# CONSOLIDATION TEST REPORT



Natural		Dry Dens. (pcf)	LL	PI	Sp. Gr.	Overburden (tsf)	$P_c$ (tsf)	$C_c$	$C_r$	Swell Press. (tsf)	Swell %	$e_o$
Sat.	Moist.											
94.5 %	29.3 %	90.8	NP	NP	2.65	6.76	0.17	0.02				0.821

<b>MATERIAL DESCRIPTION</b>										<b>USCS</b>	<b>AASHTO</b>
LO G FISA W/ CL LEN										SP	

<b>Project No.</b> 19292 <b>Project:</b> LOUISIANA, STATE OF - RIVERINE SAND MINING/SCOFIELD ISLAND RESTORATION (BA-40), PLAQUEMINES PARISH, LOUISIANA <b>Source:</b> BR-3	<b>Client:</b> SJB GROUP, LLC  <b>Sample No.:</b> 8  <b>Elev./Depth:</b> 24.0'	<b>Remarks:</b> TESTED BY: RR CHECKED BY: LR
		<b>Figure</b>

# Dial Reading vs. Time

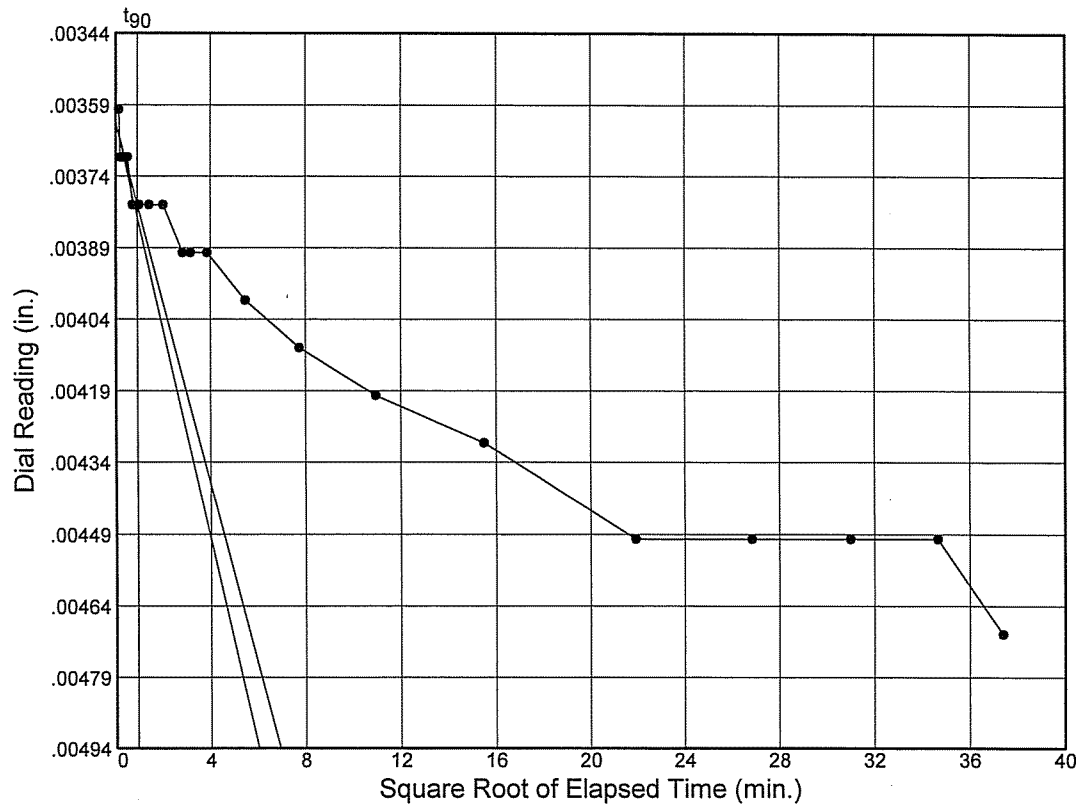
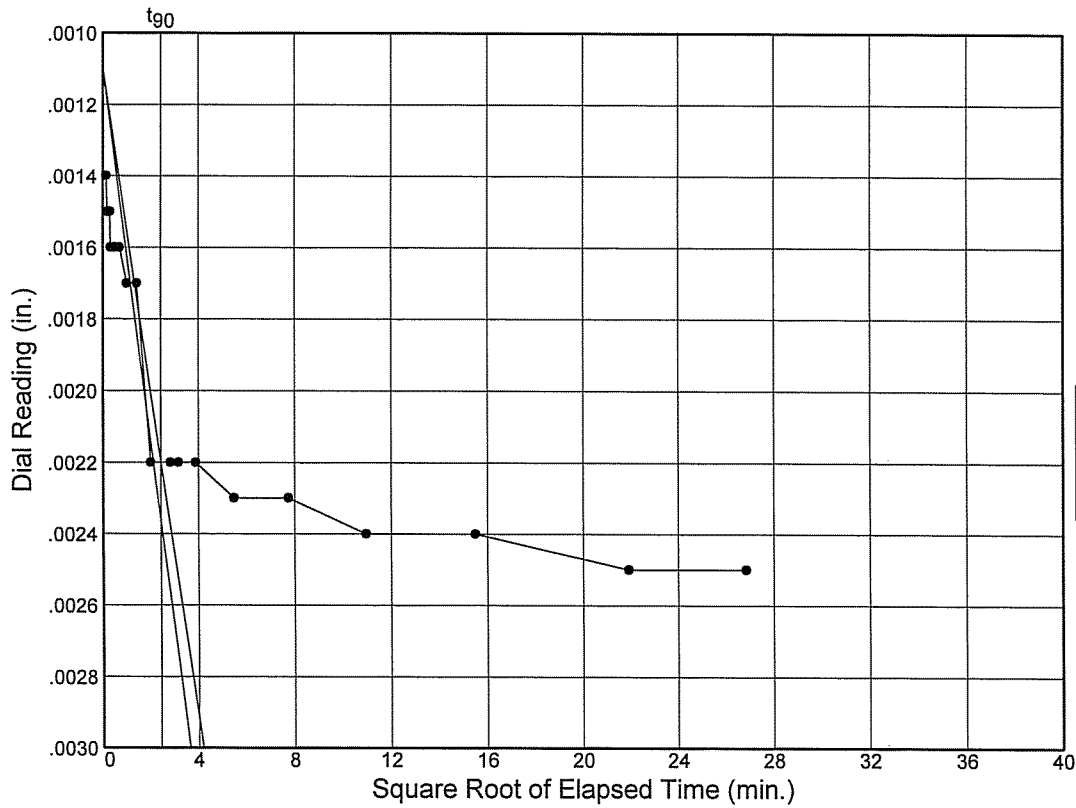
Project No.: 19292

Project: LOUISIANA, STATE OF - RIVERINE SAND MINING/SCOFIELD ISLAND RESTORATION (BA-

Source: BR-3

Sample No.: 8

Elev./Depth: 24.0'



# Dial Reading vs. Time

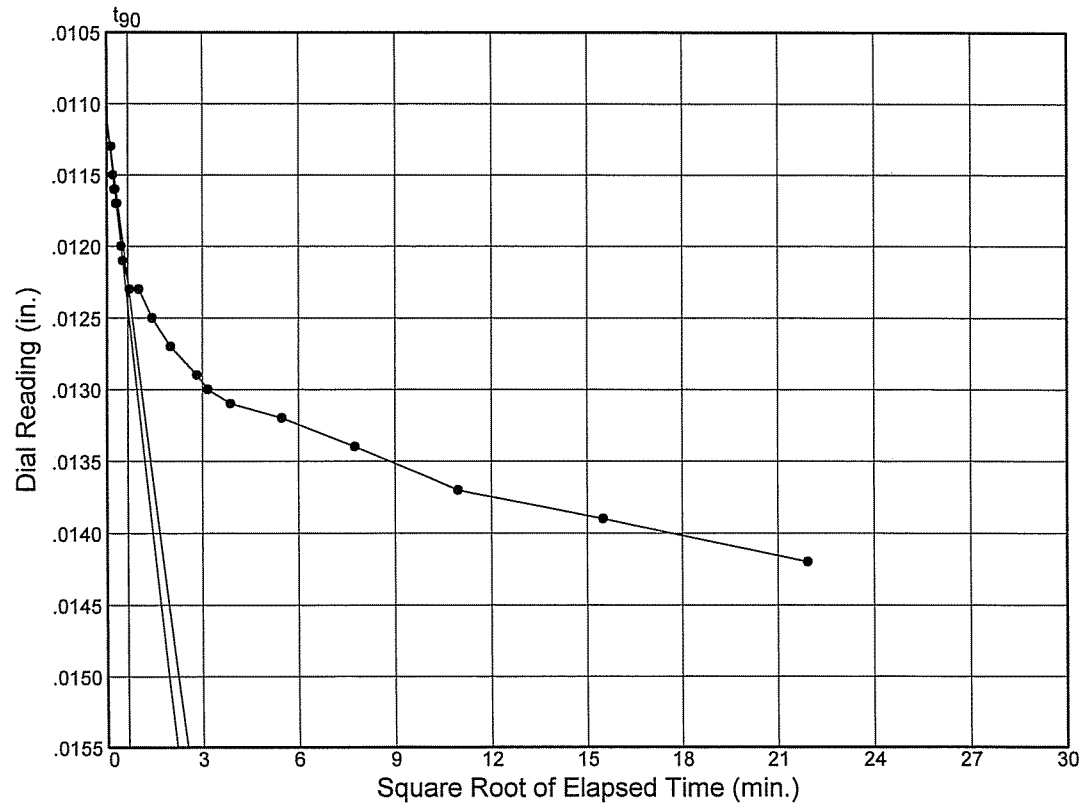
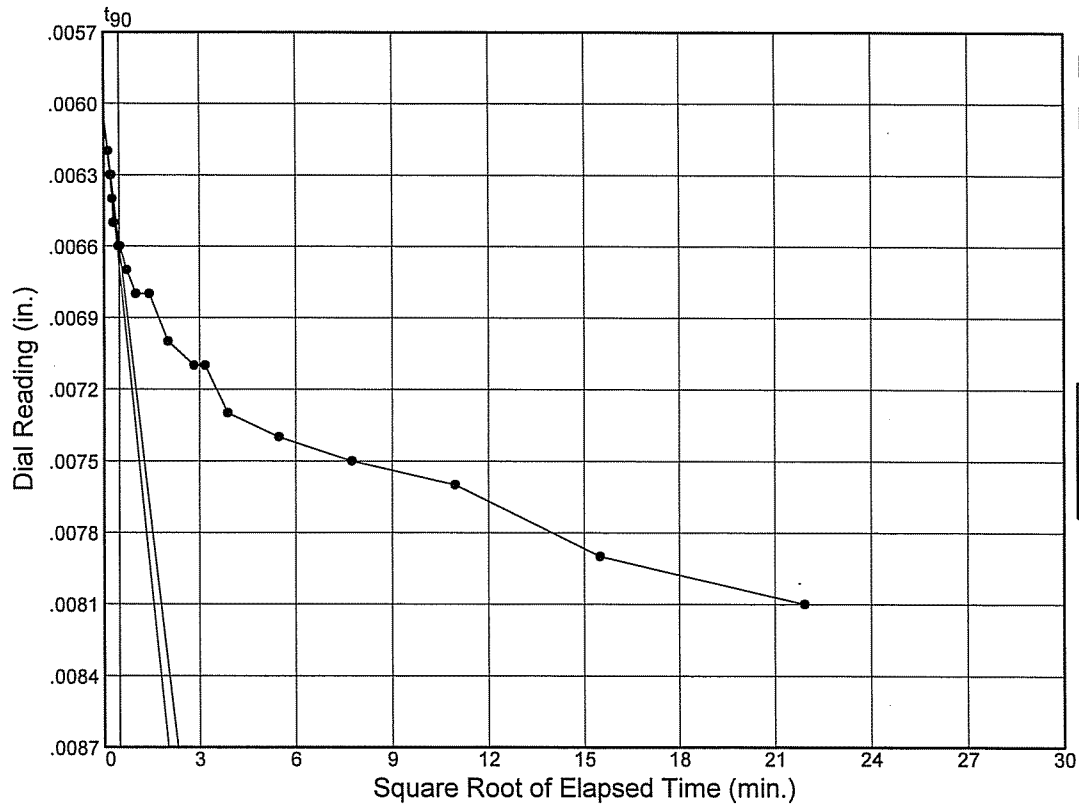
Project No.: 19292

Project: LOUISIANA, STATE OF - RIVERINE SAND MINING/SCOFIELD ISLAND RESTORATION (BA-

Source: BR-3

Sample No.: 8

Elev./Depth: 24.0'



# Dial Reading vs. Time

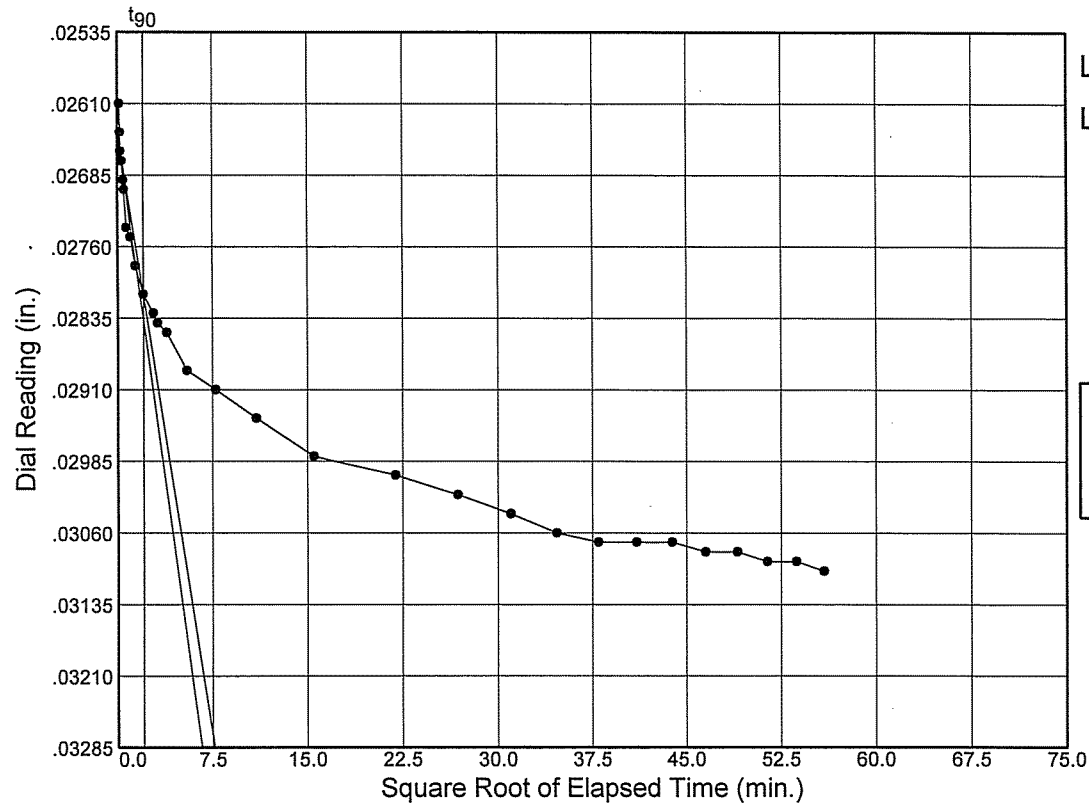
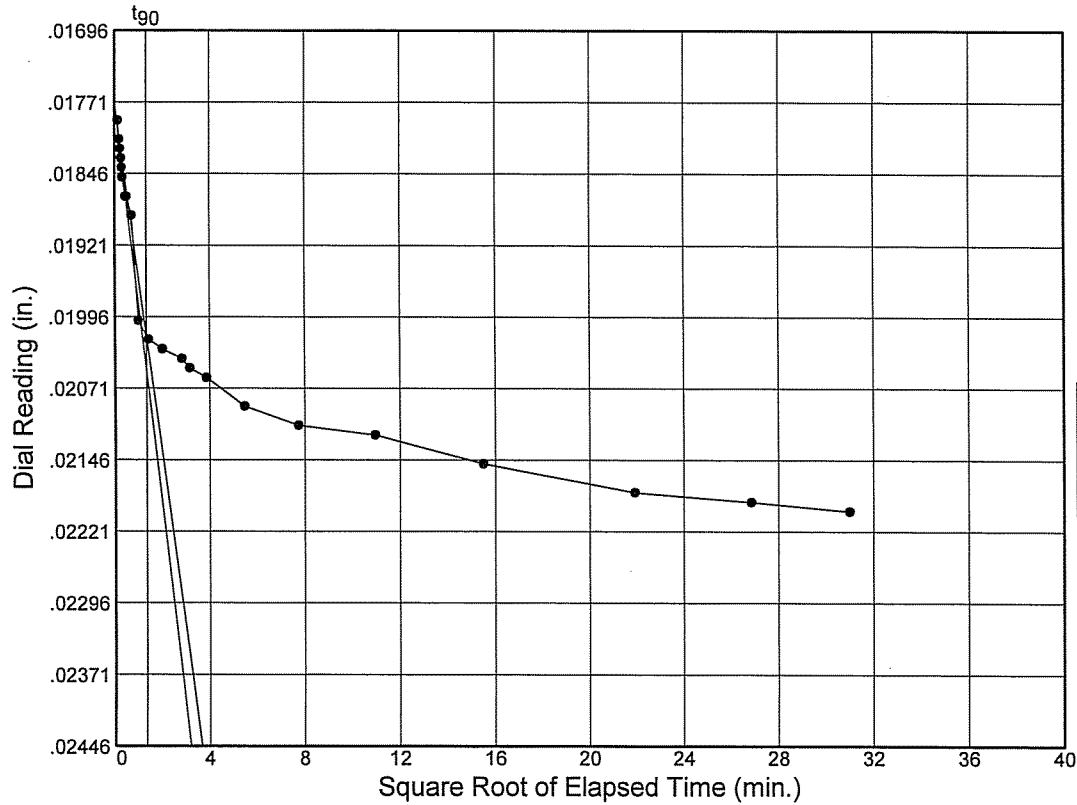
Project No.: 19292

Project: LOUISIANA, STATE OF - RIVERINE SAND MINING/SCOFIELD ISLAND RESTORATION (BA-

Source: BR-3

Sample No.: 8

Elev./Depth: 24.0'



# Dial Reading vs. Time

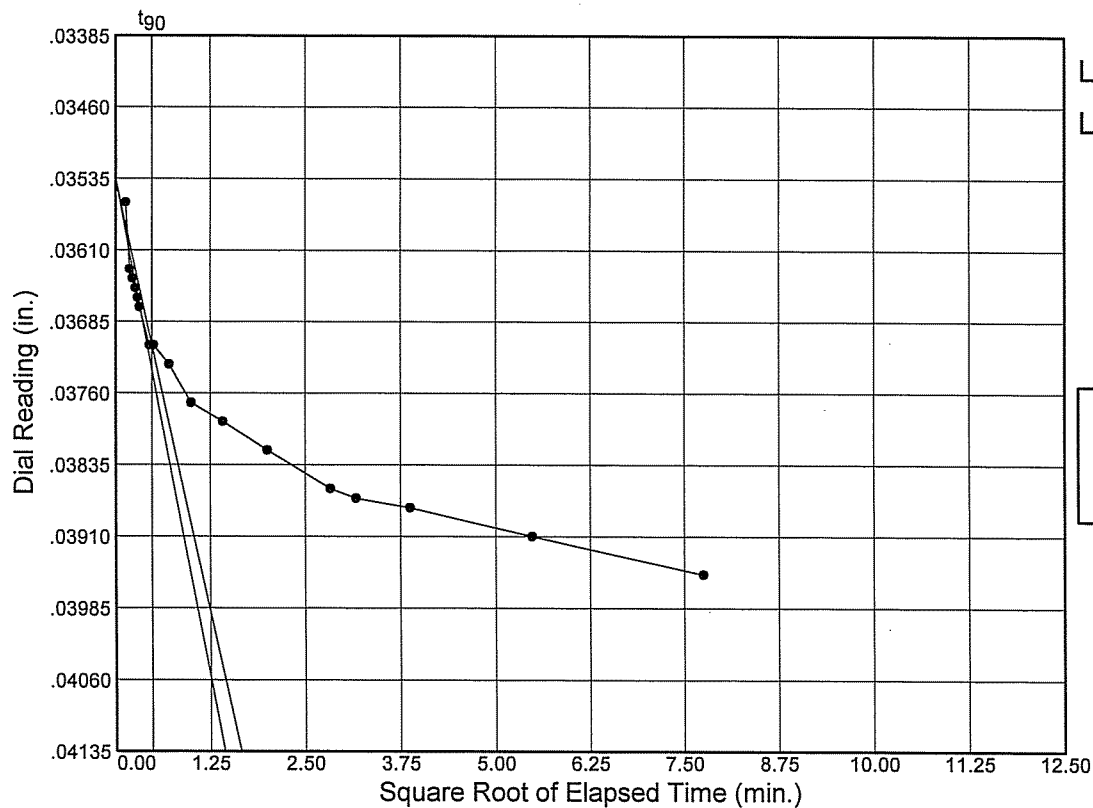
Project No.: 19292

Project: LOUISIANA, STATE OF - RIVERINE SAND MINING/SCOFIELD ISLAND RESTORATION (BA-

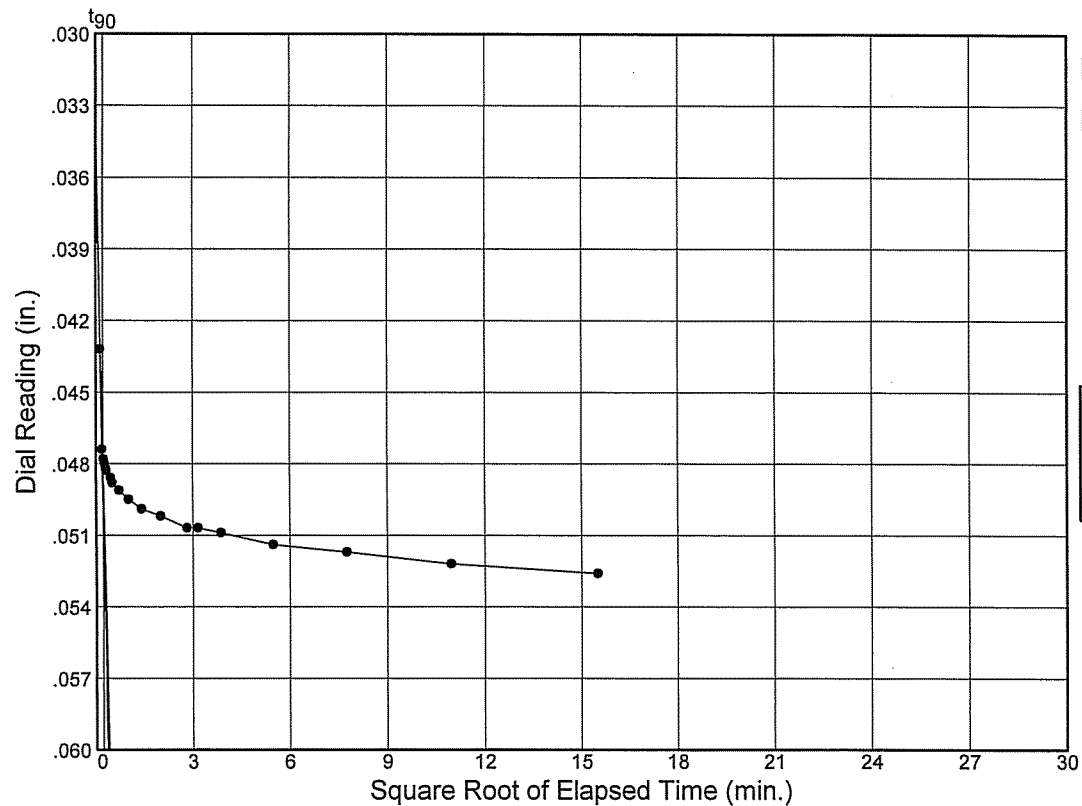
Source: BR-3

Sample No.: 8

Elev./Depth: 24.0'



$C_v @ T_{90}$   
5.61 ft.<sup>2</sup>/day



$C_v @ T_{90}$   
27.32 ft.<sup>2</sup>/day

# Dial Reading vs. Time

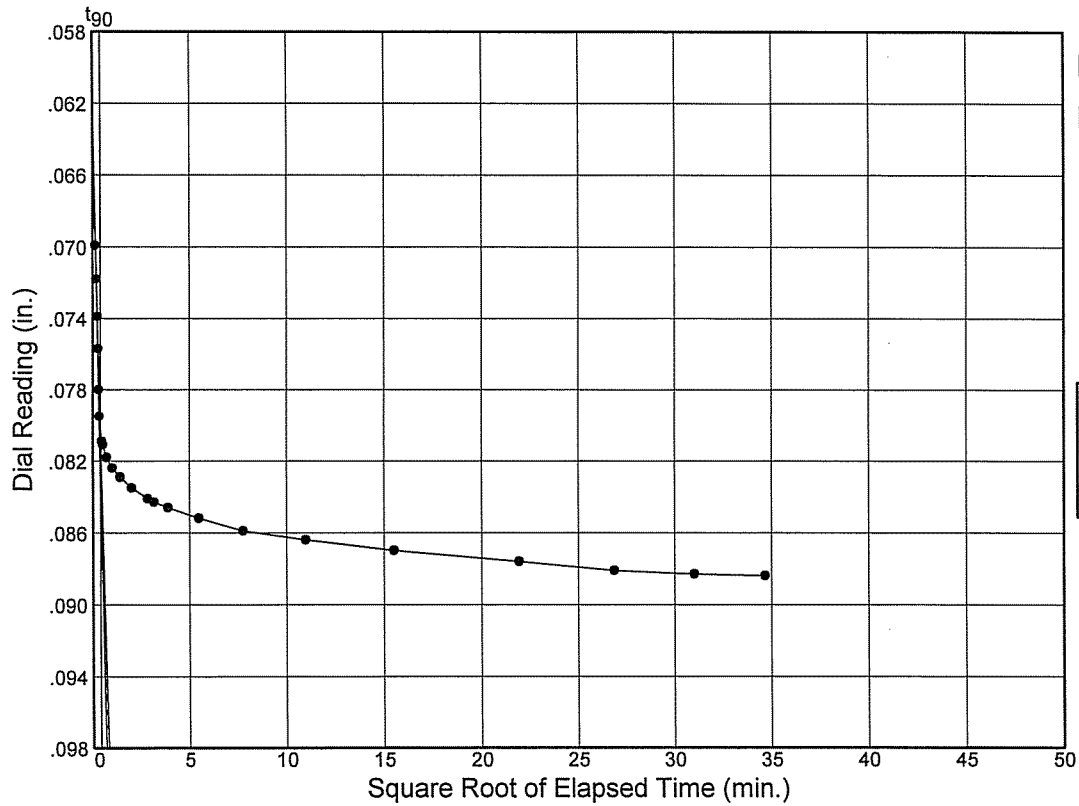
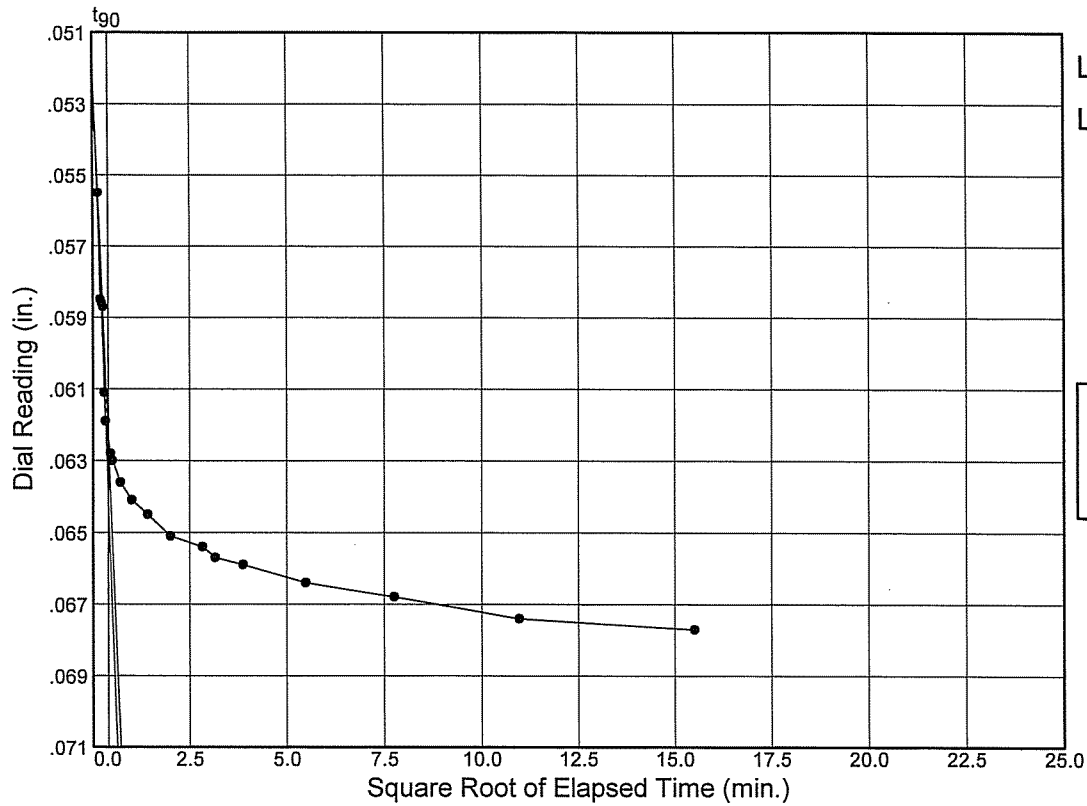
Project No.: 19292

Project: LOUISIANA, STATE OF - RIVERINE SAND MINING/SCOFIELD ISLAND RESTORATION (BA-

Source: BR-3

Sample No.: 8

Elev./Depth: 24.0'



# Dial Reading vs. Time

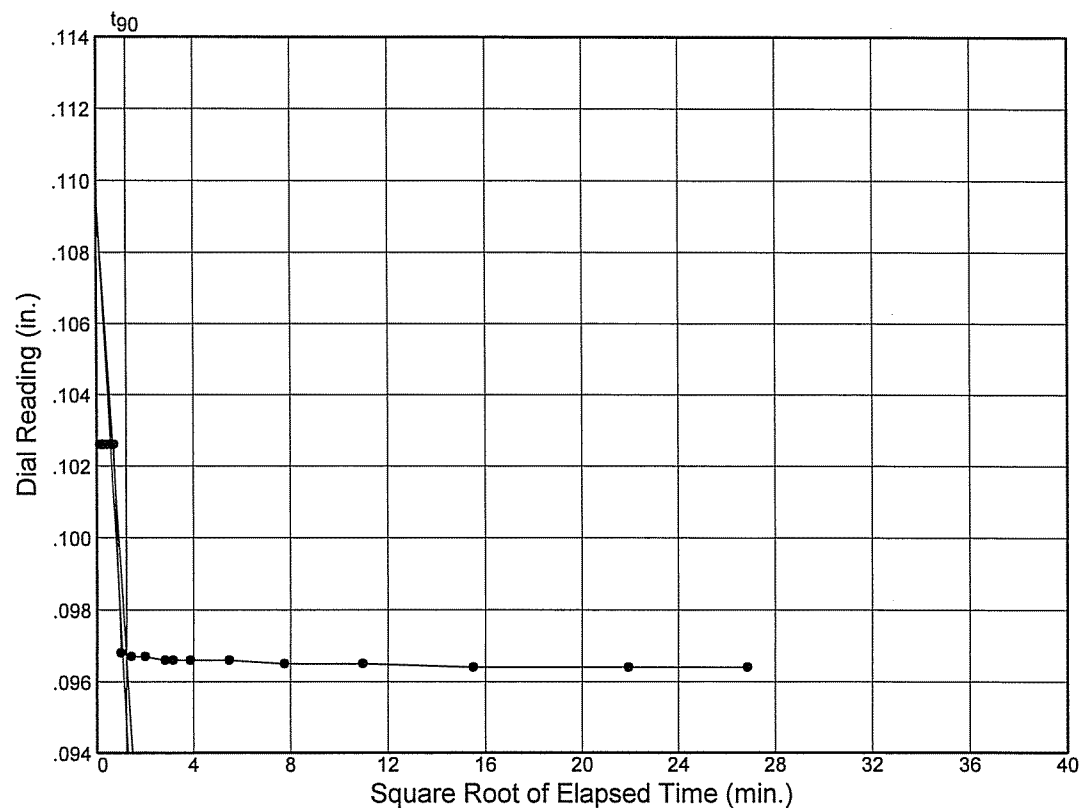
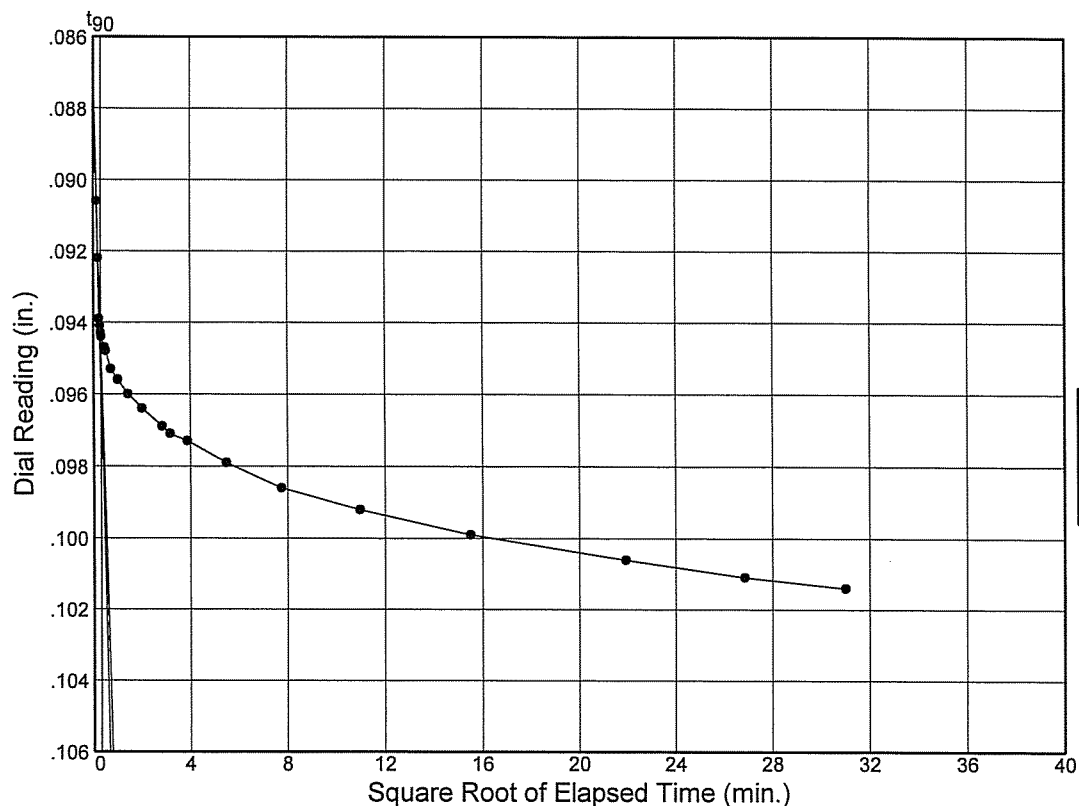
Project No.: 19292

Project: LOUISIANA, STATE OF - RIVERINE SAND MINING/SCOFIELD ISLAND RESTORATION (BA-

Source: BR-3

Sample No.: 8

Elev./Depth: 24.0'





# Dial Reading vs. Time

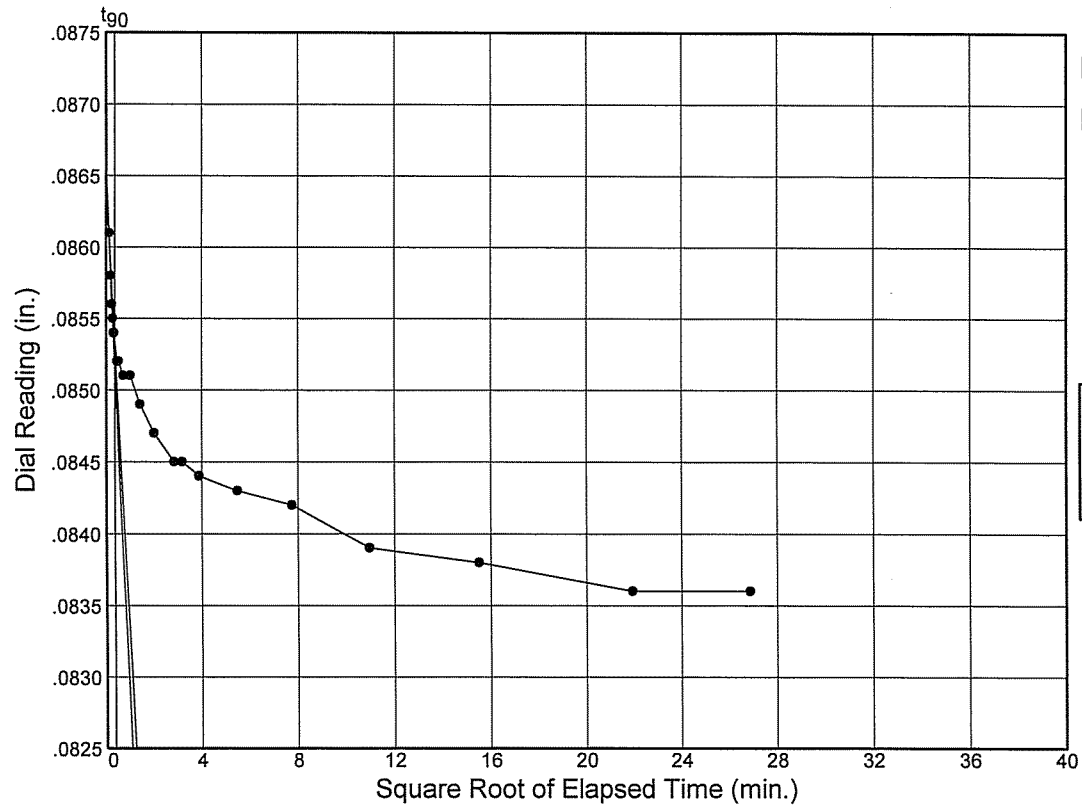
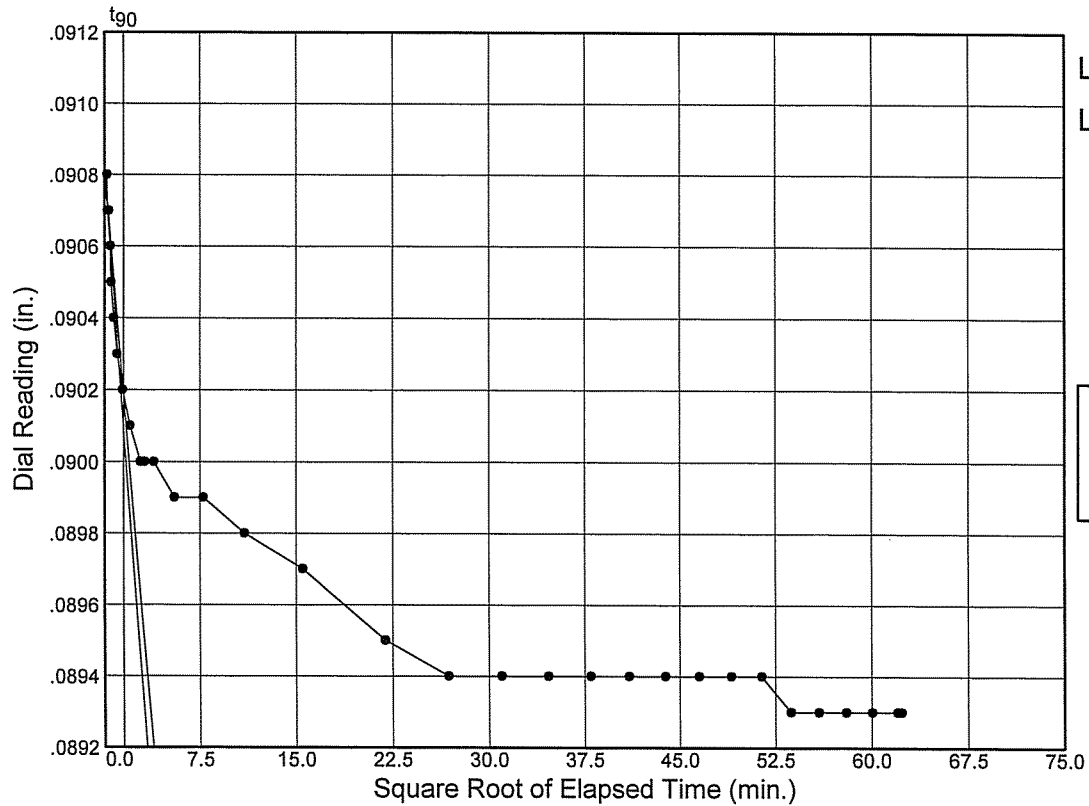
Project No.: 19292

Project: LOUISIANA, STATE OF - RIVERINE SAND MINING/SCOFIELD ISLAND RESTORATION (BA-

Source: BR-3

Sample No.: 8

Elev./Depth: 24.0'



# Dial Reading vs. Time

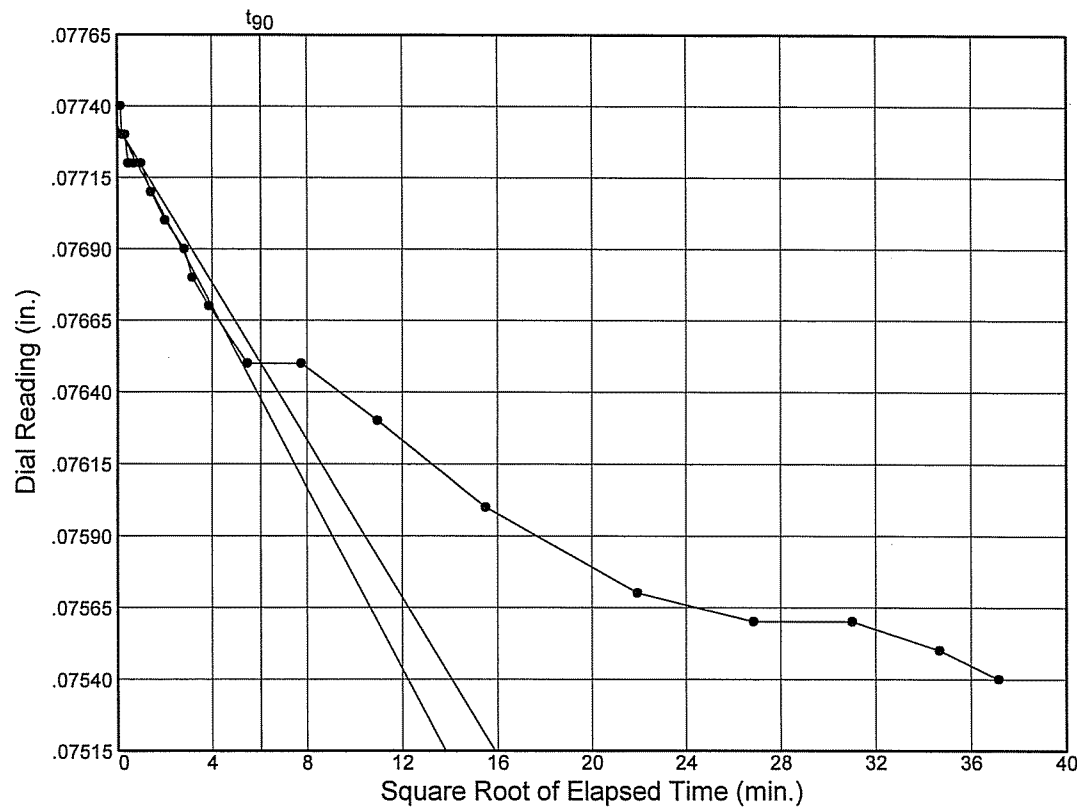
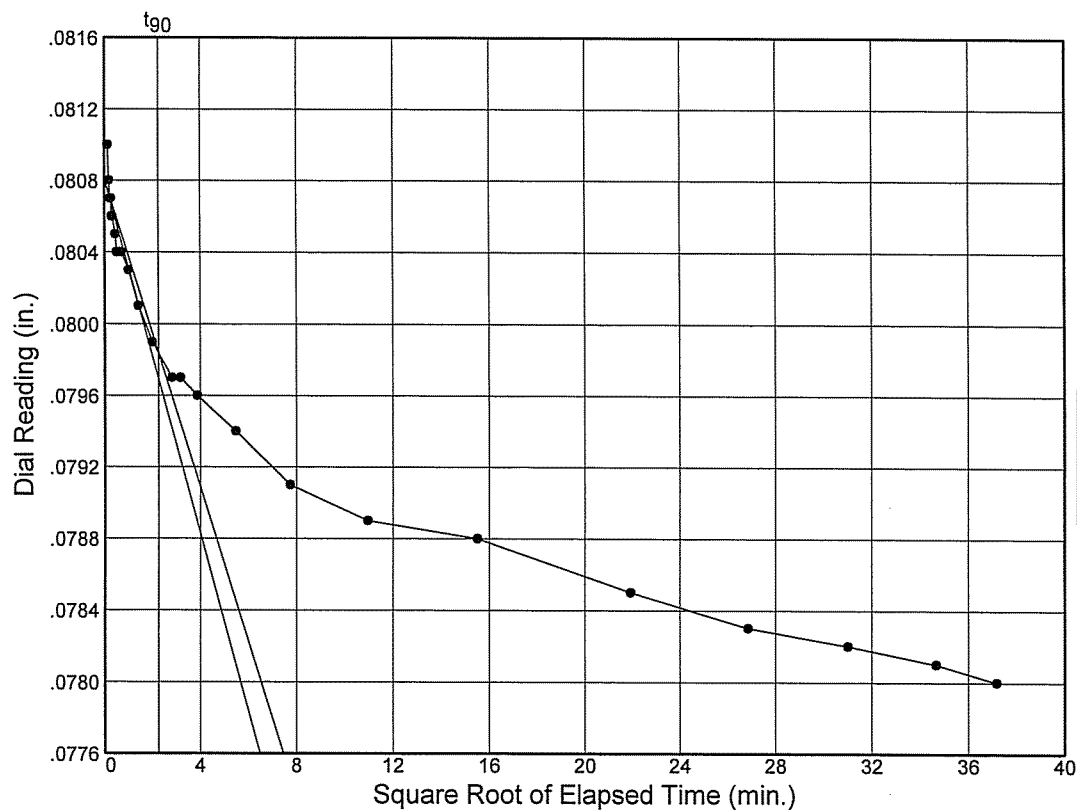
Project No.: 19292

Project: LOUISIANA, STATE OF - RIVERINE SAND MINING/SCOFIELD ISLAND RESTORATION (BA-

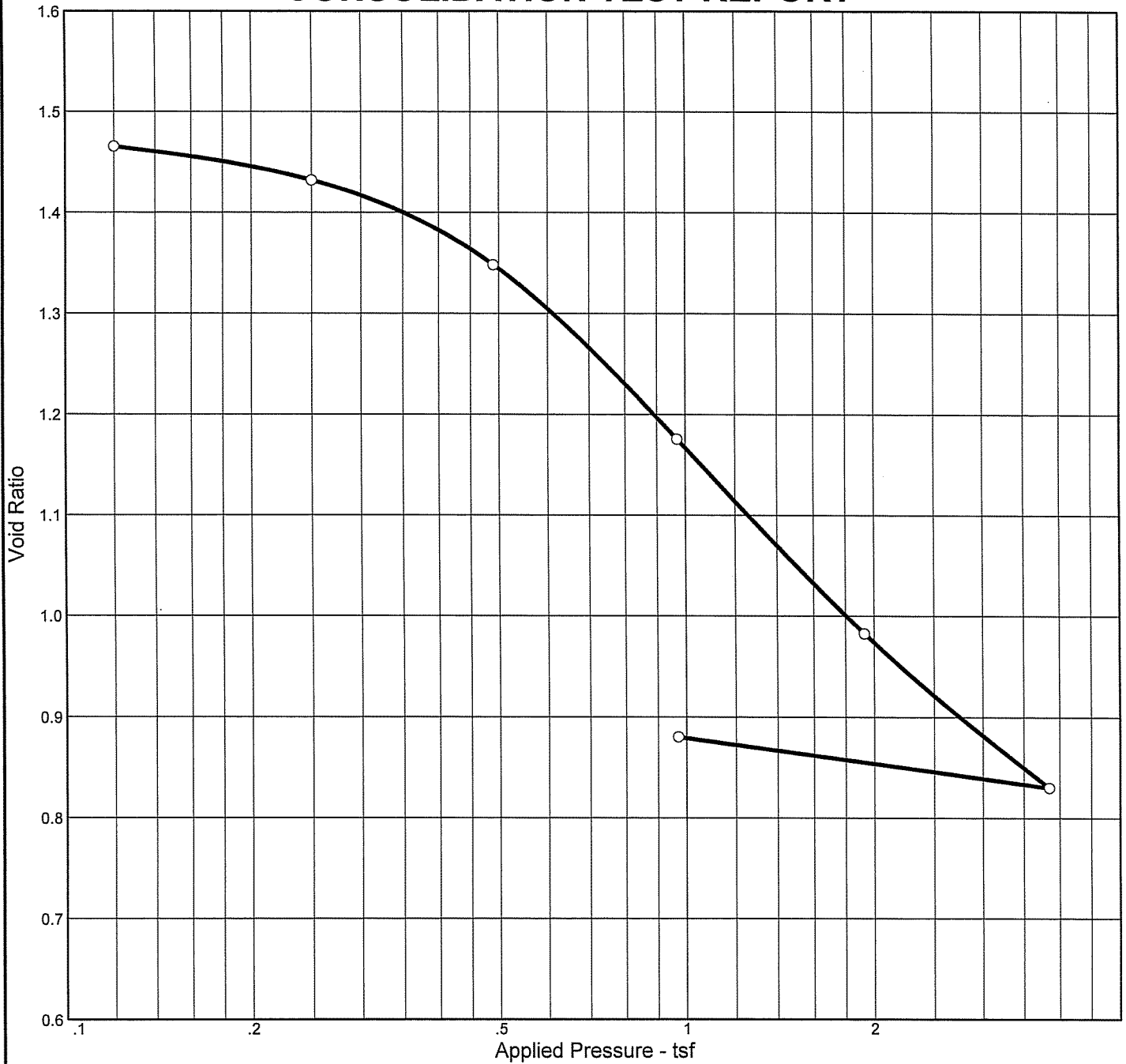
Source: BR-3

Sample No.: 8

Elev./Depth: 24.0'




# CONSOLIDATION TEST REPORT



Natural		Dry Dens. (pcf)	LL	PI	Sp. Gr.	Overburden (tsf)	P <sub>c</sub> (tsf)	C <sub>c</sub>	C <sub>r</sub>	Swell Press. (tsf)	Swell %	e <sub>0</sub>
Sat.	Moist.											
99.9 %	55.4 %	67.9	67	49	2.74		0.49	0.66	0.08			1.521

MATERIAL DESCRIPTION										USCS	AASHTO
VSO G CL										CH	

<b>Project No.</b> 19292 <b>Project:</b> LOUISIANA, STATE OF - RIVERINE SAND MINING/SCOFIELD ISLAND RESTORATION (BA-40), PLAQUEMINES PARISH, LOUISIANA <b>Source:</b> BR-5	<b>Client:</b> SJB GROUP, LLC  <b>Sample No.:</b> 10 <b>Elev./Depth:</b> 38.0'	<b>Remarks:</b> TESTED BY: RR CHECKED BY: LR
 <b>EUSTIS</b> Metairie, Louisiana Lafayette, Louisiana Gulfport, Mississippi		<b>Figure</b>

# Dial Reading vs. Time

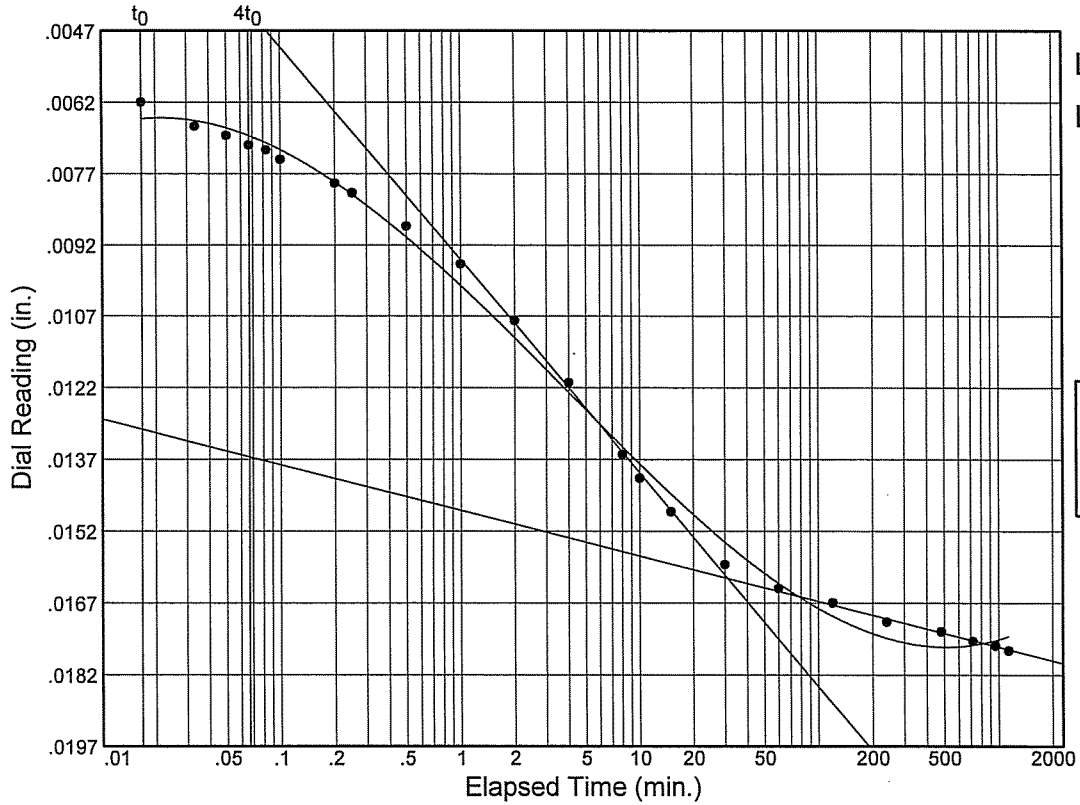
Project No.: 19292

Project: LOUISIANA, STATE OF - RIVERINE SAND MINING/SCOFIELD ISLAND RESTORATION (BA-

Source: BR-5

Sample No.: 10

Elev./Depth: 38.0'



Load No.= 1

Load= 0.12 tsf

$D_0 = 0.00619$

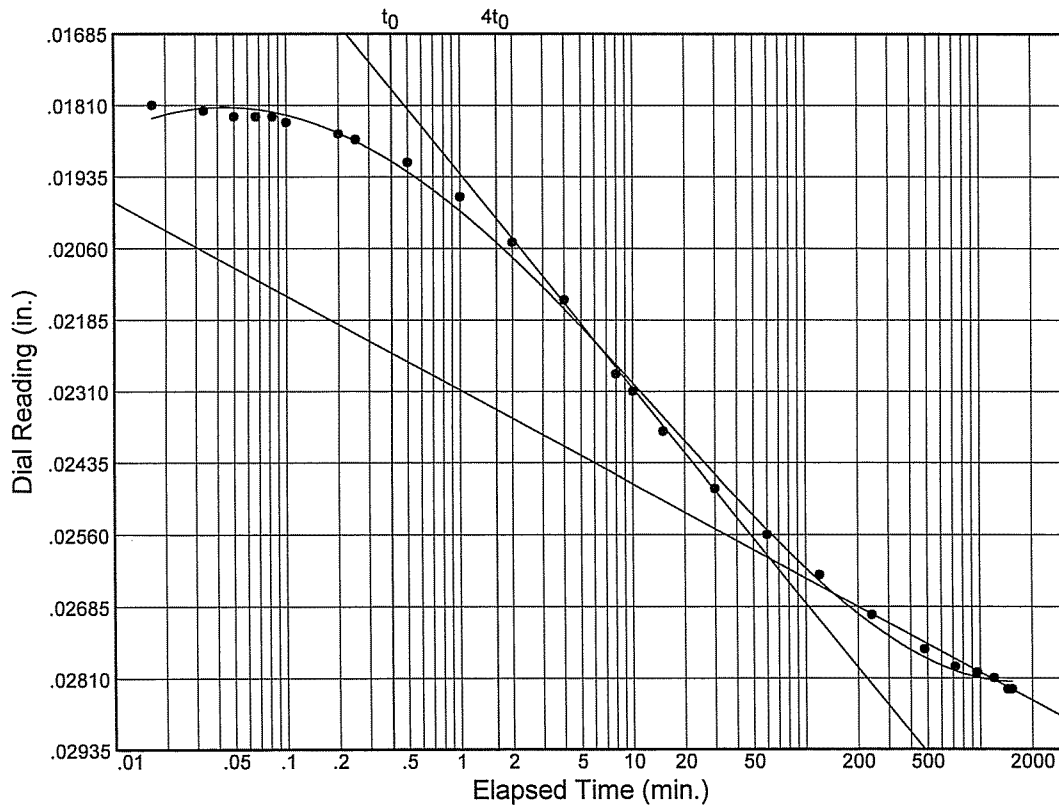
$D_{50} = 0.01119$

$D_{100} = 0.01619$

$T_{50} = 2.03 \text{ min.}$

$C_v @ T_{50}$   
0.15 ft.<sup>2</sup>/day

$C_\alpha = 0.001$



Load No.= 2

Load= 0.25 tsf

$D_0 = 0.01767$

$D_{50} = 0.02185$

$D_{100} = 0.02603$

$T_{50} = 4.58 \text{ min.}$

$C_v @ T_{50}$   
0.06 ft.<sup>2</sup>/day

$C_\alpha = 0.002$

# Dial Reading vs. Time

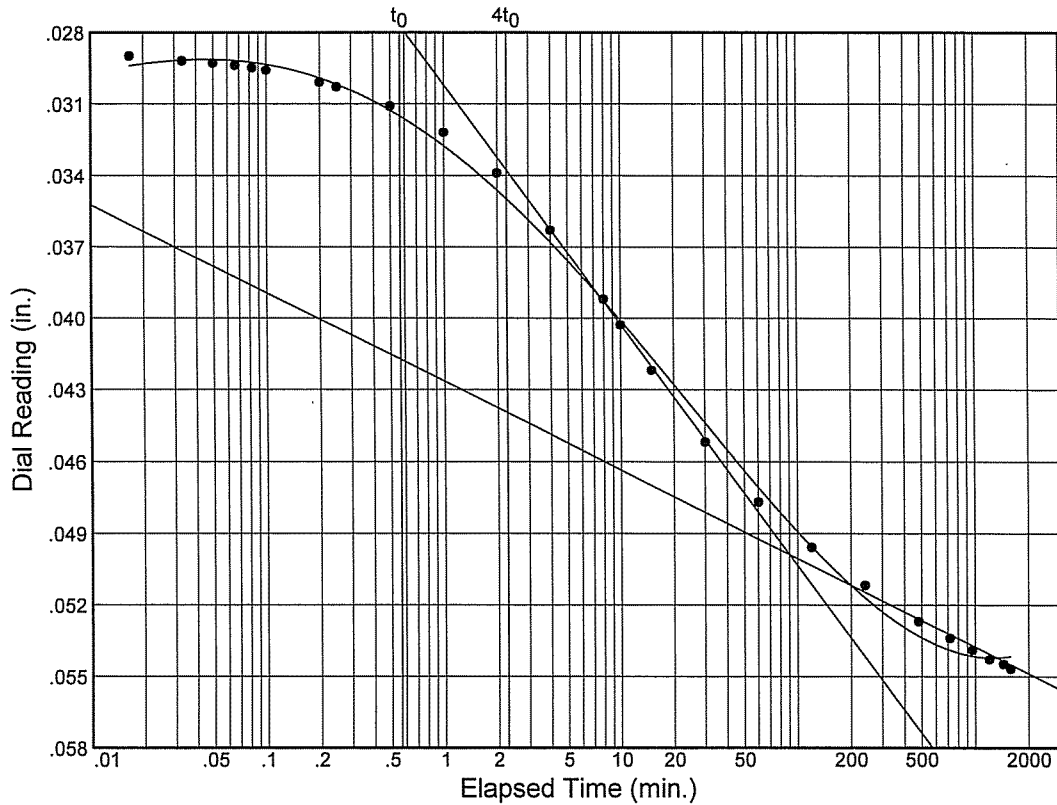
Project No.: 19292

Project: LOUISIANA, STATE OF - RIVERINE SAND MINING/SCOFIELD ISLAND RESTORATION (BA-

Source: BR-5

Sample No.: 10

Elev./Depth: 38.0'



Load No. = 3

Load = 0.49 tsf

$D_0 = 0.02803$

$D_{50} = 0.03896$

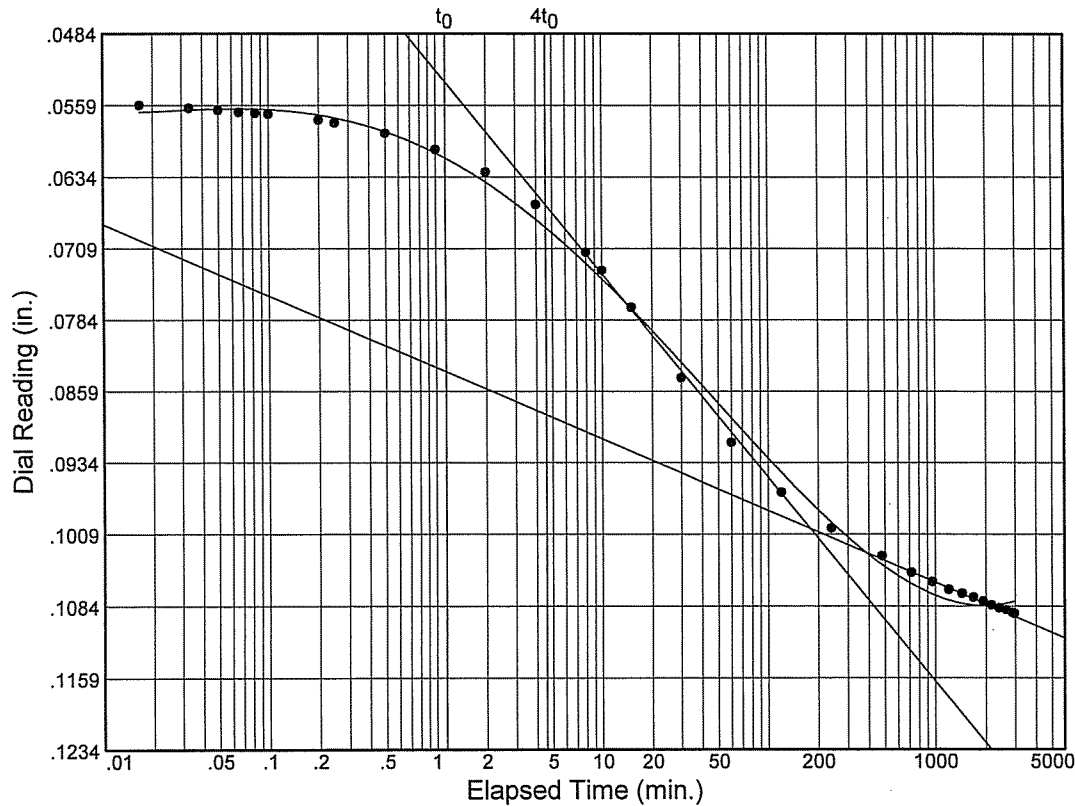
$D_{100} = 0.04990$

$T_{50} = 7.37$  min.

$C_v @ T_{50}$

0.04 ft.<sup>2</sup>/day

$C_\alpha = 0.005$



Load No. = 4

Load = 0.97 tsf

$D_0 = 0.05412$

$D_{50} = 0.07721$

$D_{100} = 0.10030$

$T_{50} = 14.96$  min.

$C_v @ T_{50}$

0.02 ft.<sup>2</sup>/day

$C_\alpha = 0.010$

# Dial Reading vs. Time

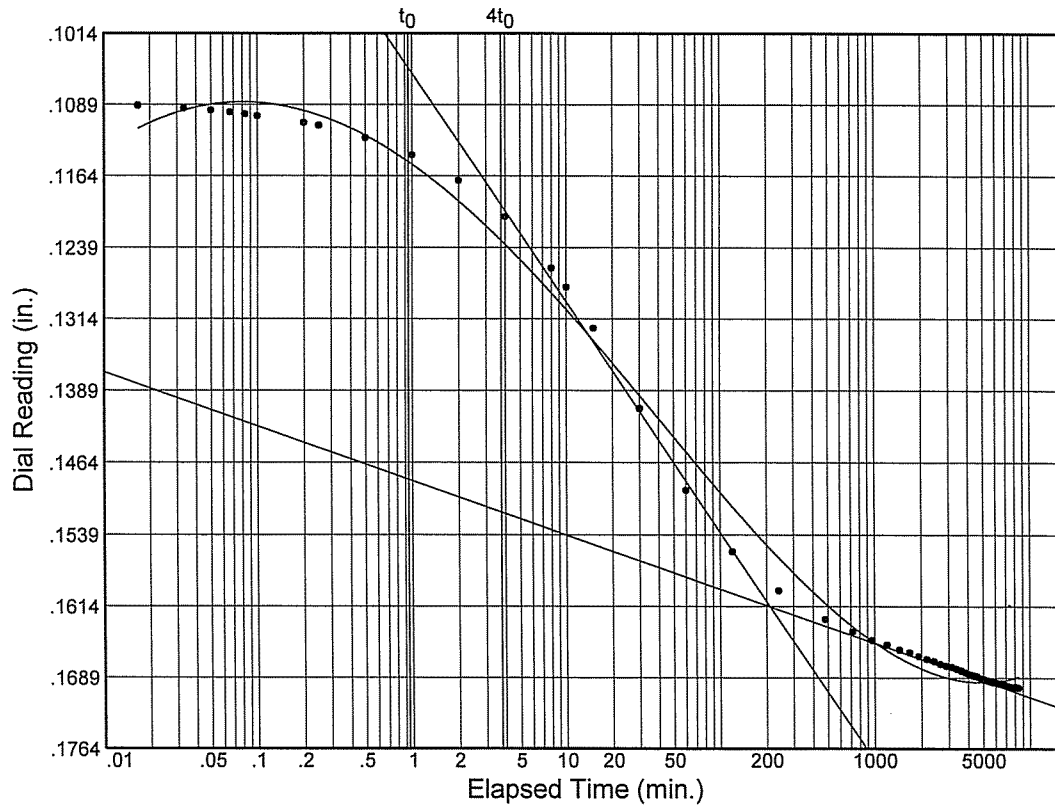
Project No.: 19292

Project: LOUISIANA, STATE OF - RIVERINE SAND MINING/SCOFIELD ISLAND RESTORATION (BA-

Source: BR-5

Sample No.: 10

Elev./Depth: 38.0'



Load No.= 5

Load= 1.93 tsf

$D_0 = 0.10663$

$D_{50} = 0.13405$

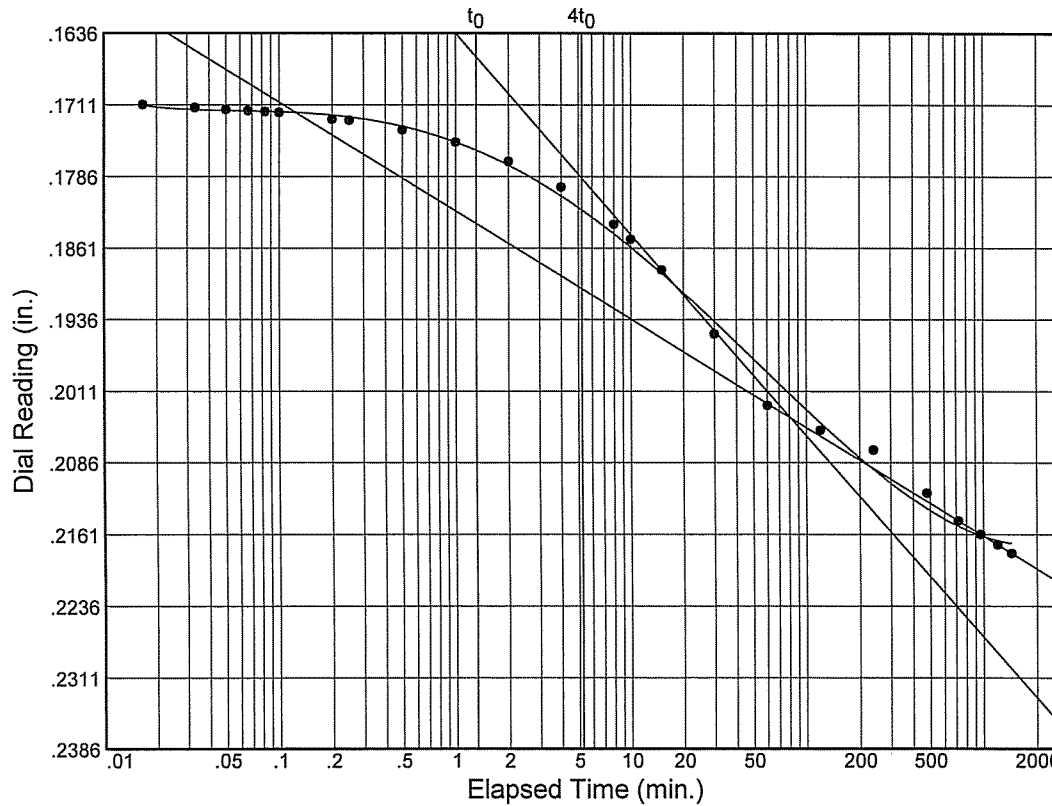
$D_{100} = 0.16148$

$T_{50} = 15.63 \text{ min.}$

$C_v @ T_{50}$

0.01 ft.<sup>2</sup>/day

$C_\alpha = 0.008$



Load No.= 6

Load= 3.85 tsf

$D_0 = 0.16959$

$D_{50} = 0.18676$

$D_{100} = 0.20393$

$T_{50} = 11.18 \text{ min.}$

$C_v @ T_{50}$

0.02 ft.<sup>2</sup>/day

$C_\alpha = 0.018$

# Dial Reading vs. Time

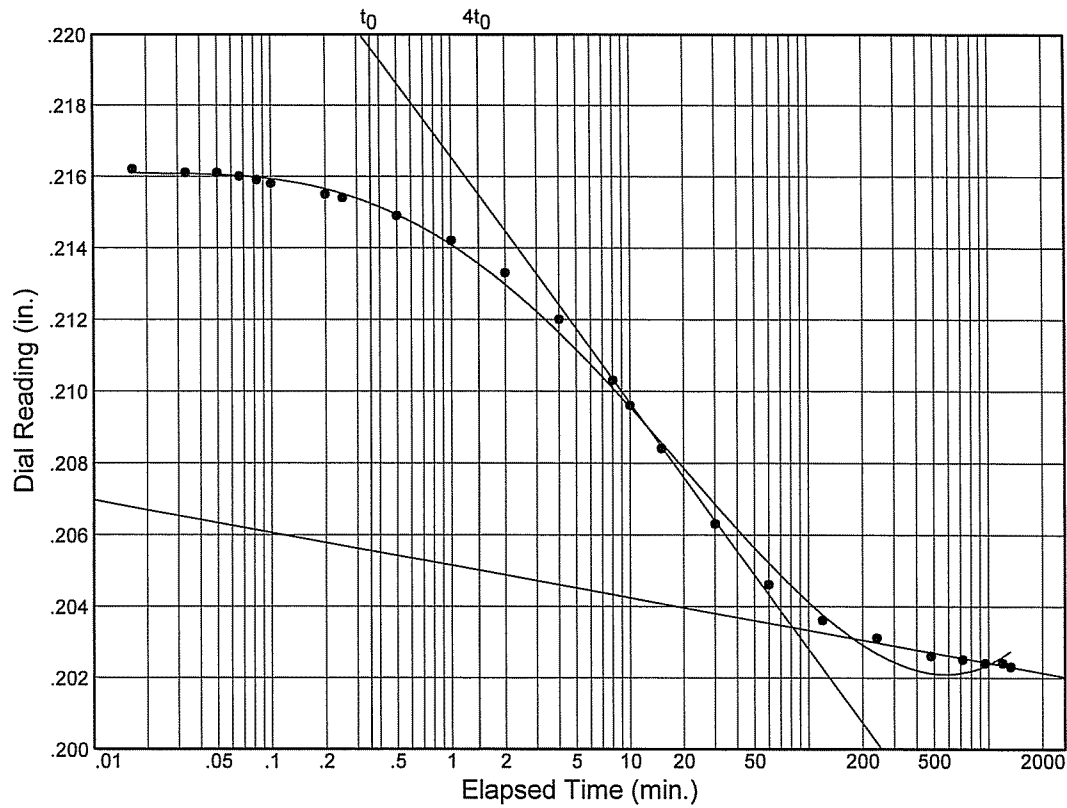
Project No.: 19292

Project: LOUISIANA, STATE OF - RIVERINE SAND MINING/SCOFIELD ISLAND RESTORATION (BA-

Source: BR-5

Sample No.: 10

Elev./Depth: 38.0'



Load No. = 7

Load = 0.97 tsf

$D_0 = 0.21694$

$D_{50} = 0.21017$

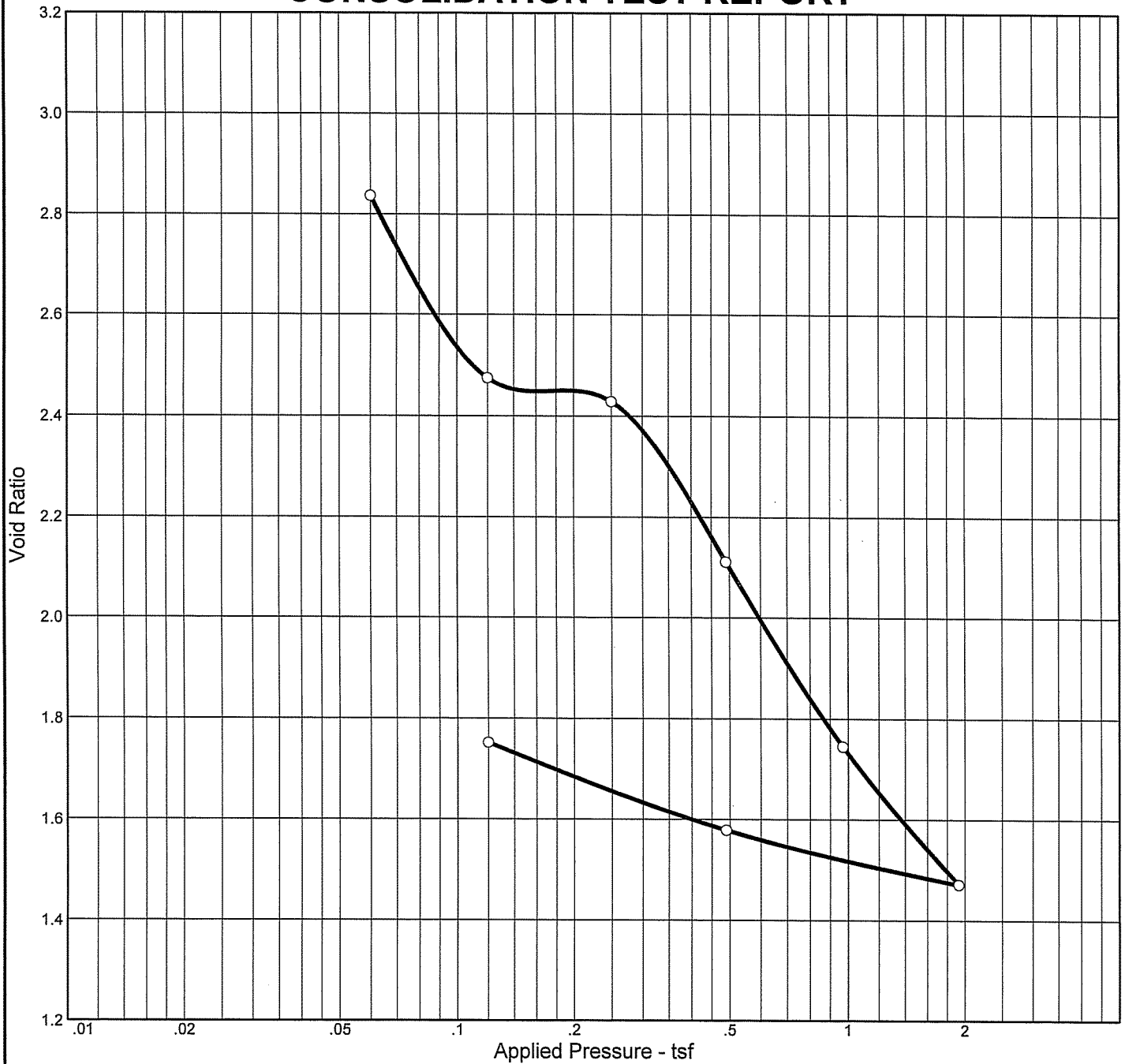
$D_{100} = 0.20340$

$T_{50} = 7.69 \text{ min.}$

$C_v @ T_{50}$

0.02 ft.<sup>2</sup>/day

# CONSOLIDATION TEST REPORT



Natural		Dry Dens. (pcf)	LL	PI	Sp. Gr.	Overburden (tsf)	P <sub>c</sub> (tsf)	C <sub>c</sub>	C <sub>r</sub>	Swell Press. (tsf)	Swell %	e <sub>0</sub>
Sat.	Moist.											
99.8 %	110.6 %	42.4	117	89	2.74		0.28	1.27	0.23			3.037

MATERIAL DESCRIPTION										USCS	AASHTO
XSO G CL W/ OM										CH	

Project No. 19292	Client: SJB GROUP, LLC	<b>Remarks:</b> TESTED BY: RR CHECKED BY: LR
Project: LOUISIANA, STATE OF - RIVERINE SAND MINING/SCOFIELD ISLAND RESTORATION (BA-40), PLAQUEMINES PARISH, LOUISIANA		
Source: BR-5	Sample No.: 2      Elev./Depth: 4.0'	



Figure



# Dial Reading vs. Time

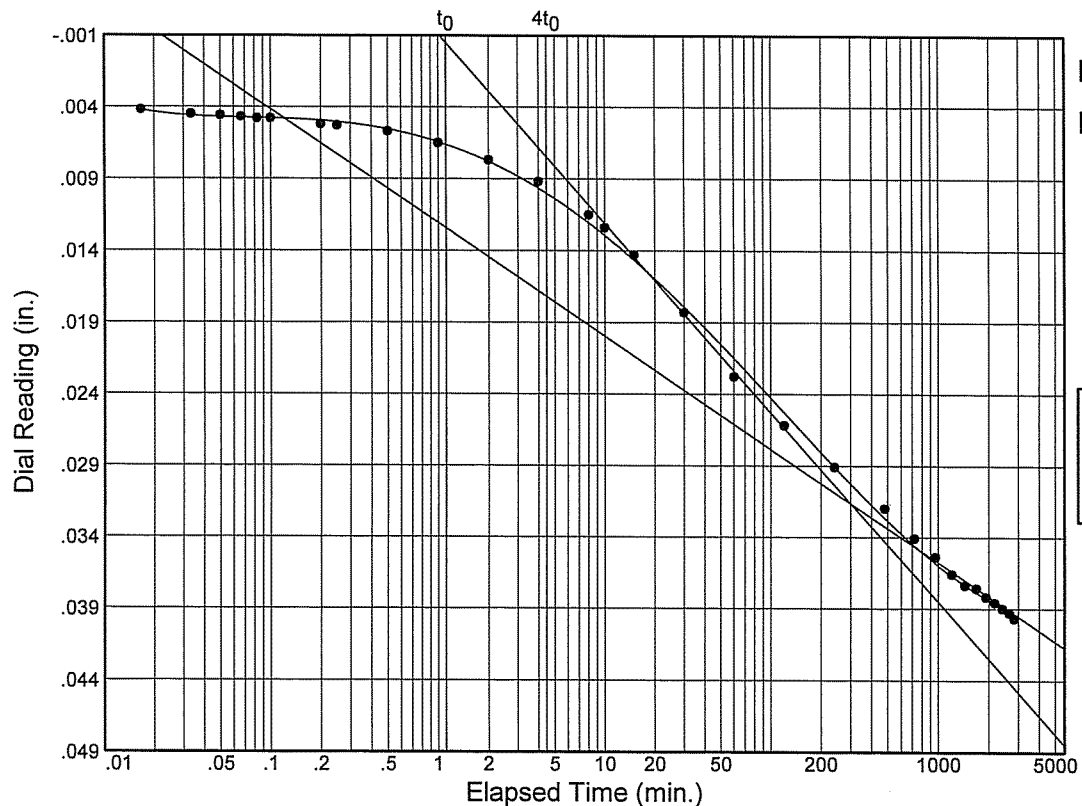
Project No.: 19292

Project: LOUISIANA, STATE OF - RIVERINE SAND MINING/SCOFIELD ISLAND RESTORATION (BA-

Source: BR-5

Sample No.: 2

Elev./Depth: 4.0'



Load No.= 1

Load= 0.06 tsf

$D_0 = 0.00321$

$D_{50} = 0.01744$

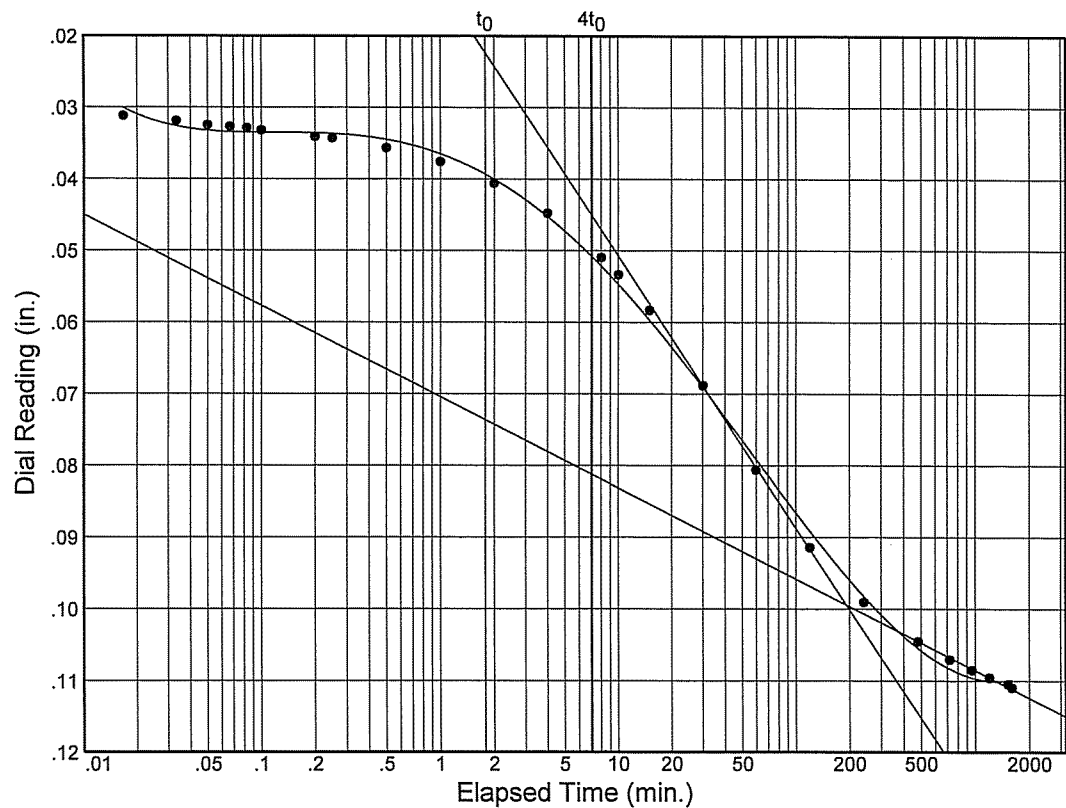
$D_{100} = 0.03168$

$T_{50} = 27.30$  min.

$C_v @ T_{50}$

0.01 ft.<sup>2</sup>/day

$C_\alpha = 0.010$



Load No.= 2

Load= 0.12 tsf

$D_0 = 0.02778$

$D_{50} = 0.06363$

$D_{100} = 0.09948$

$T_{50} = 20.17$  min.

$C_v @ T_{50}$

0.01 ft.<sup>2</sup>/day

$C_\alpha = 0.017$

# Dial Reading vs. Time

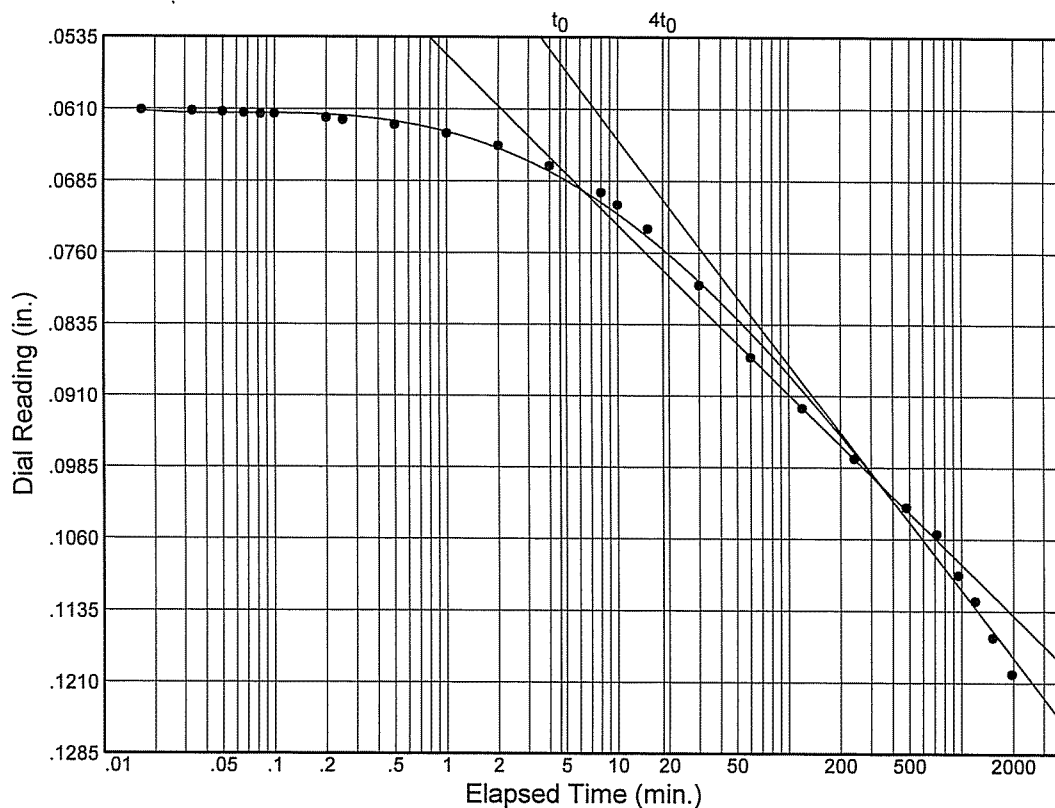
Project No.: 19292

Project: LOUISIANA, STATE OF - RIVERINE SAND MINING/SCOFIELD ISLAND RESTORATION (BA-

Source: BR-5

Sample No.: 2

Elev./Depth: 4.0'



Load No.= 3

Load= 0.25 tsf

$D_0 = 0.06058$

$D_{50} = 0.08048$

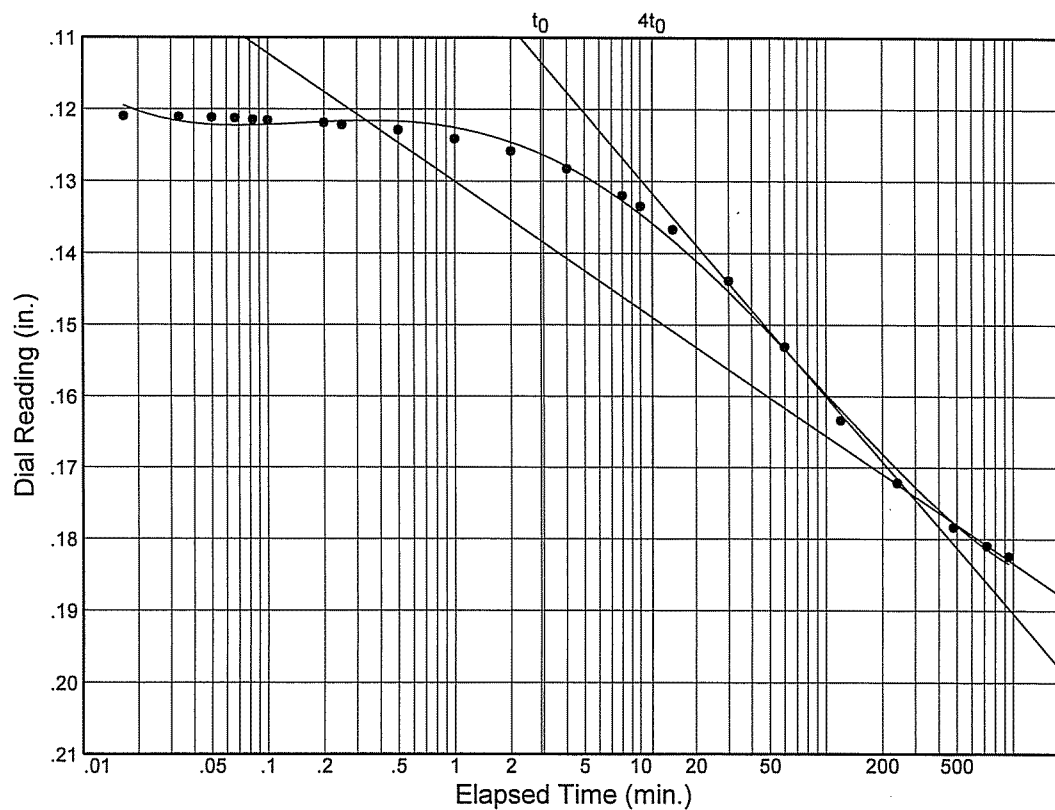
$D_{100} = 0.10037$

$T_{50} = 36.58 \text{ min.}$

$C_v @ T_{50}$

0.01 ft.<sup>2</sup>/day

$C_\alpha = 0.024$



Load No.= 4

Load= 0.49 tsf

$D_0 = 0.11653$

$D_{50} = 0.14503$

$D_{100} = 0.17352$

$T_{50} = 28.98 \text{ min.}$

$C_v @ T_{50}$

0.01 ft.<sup>2</sup>/day

$C_\alpha = 0.026$

# Dial Reading vs. Time

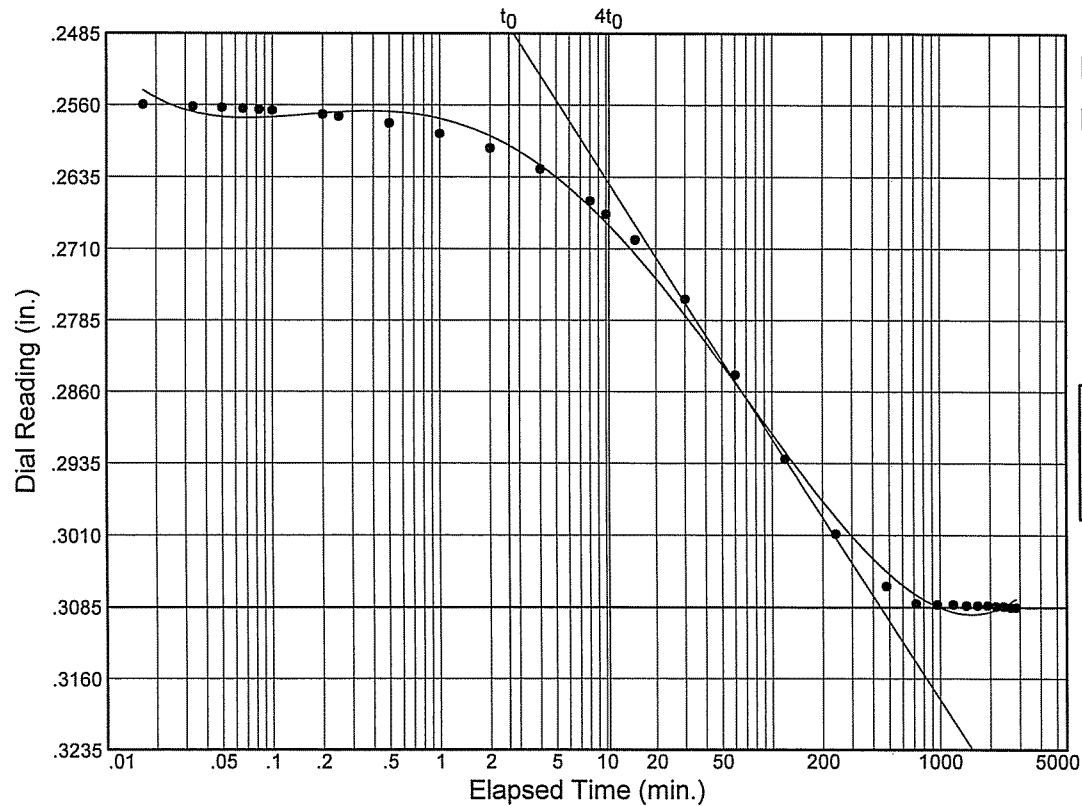
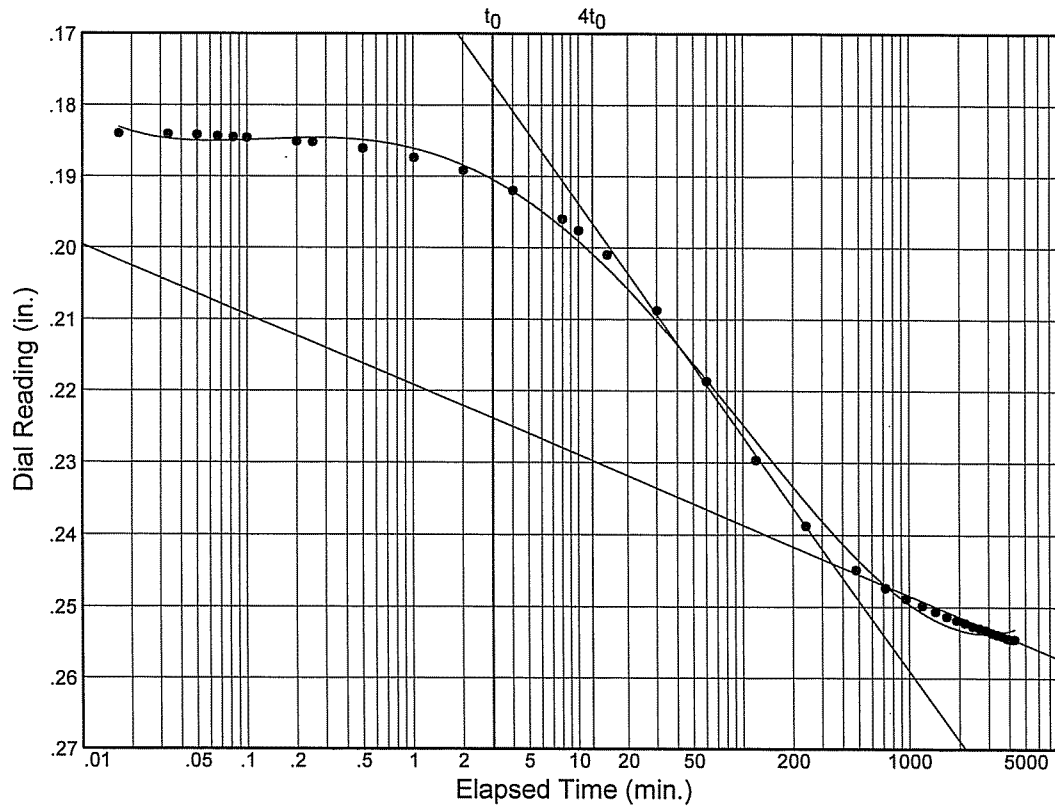
Project No.: 19292

Project: LOUISIANA, STATE OF - RIVERINE SAND MINING/SCOFIELD ISLAND RESTORATION (BA-

Source: BR-5

Sample No.: 2

Elev./Depth: 4.0'



# Dial Reading vs. Time

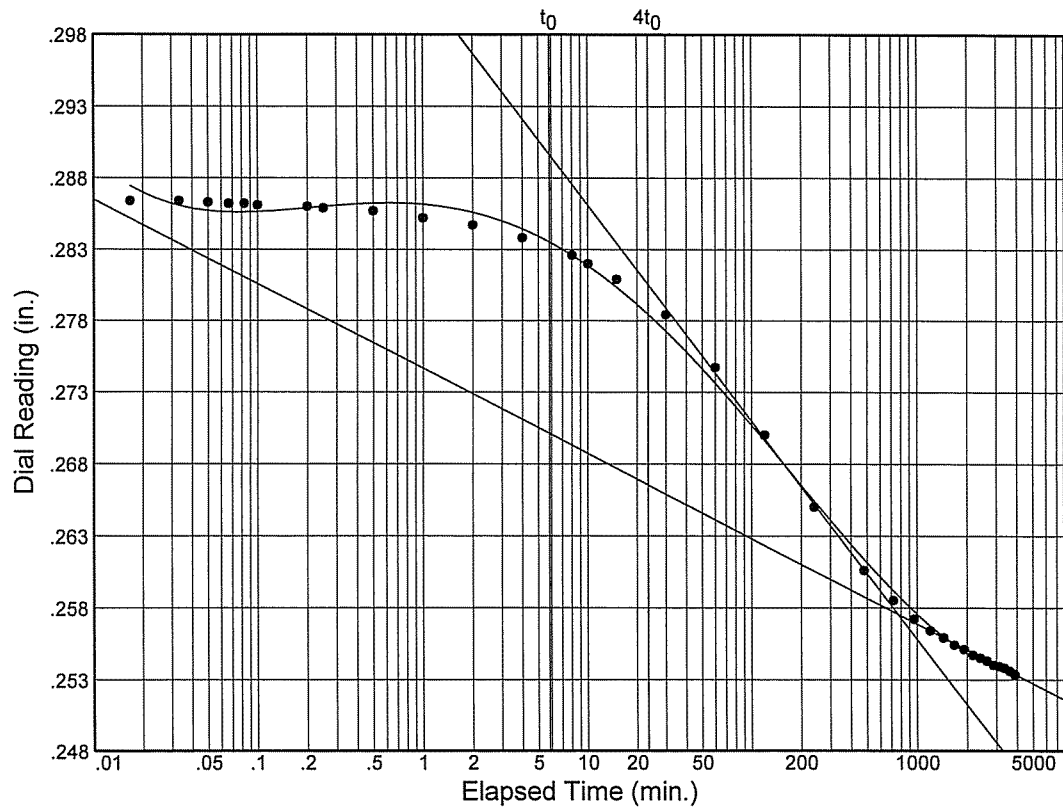
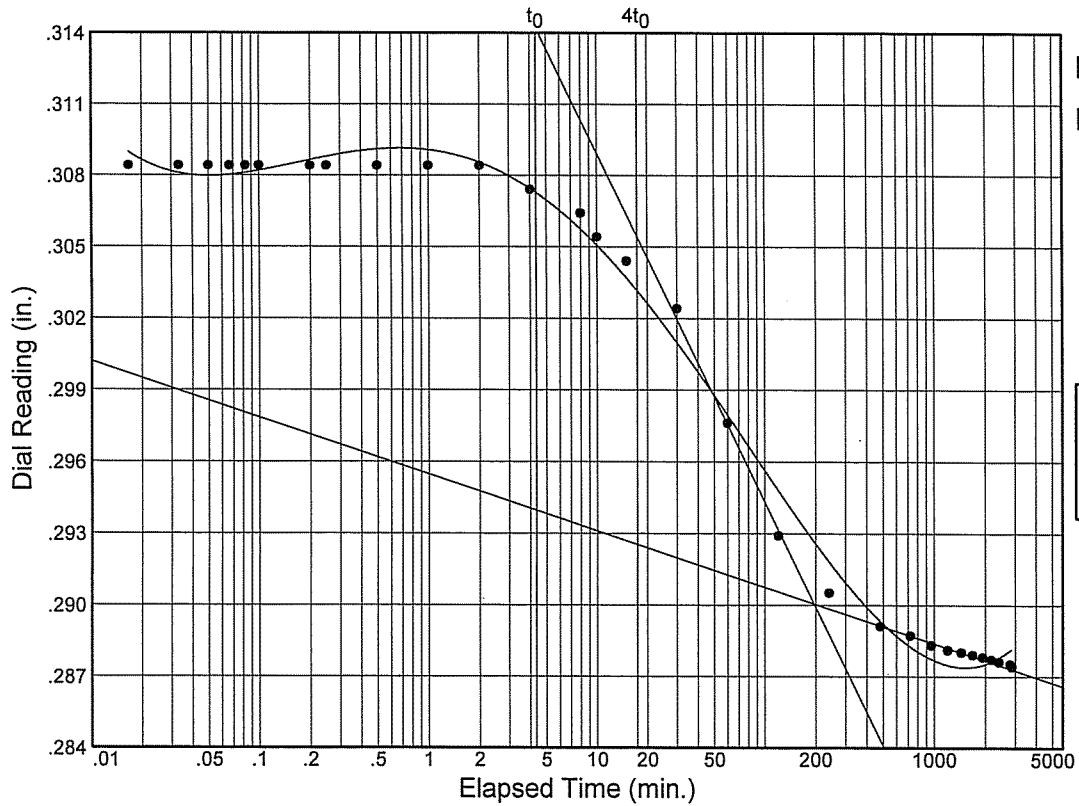
Project No.: 19292

Project: LOUISIANA, STATE OF - RIVERINE SAND MINING/SCOFIELD ISLAND RESTORATION (BA-

Source: BR-5

Sample No.: 2

Elev./Depth: 4.0'



## **ANNEX K2**

# **SLOPE STABILITY ANALYSIS**



# EUSTIS ENGINEERING COMPANY, INC.

Geotechnical Engineers  
Lafayette, Louisiana

Date 3-16-09

Job 19292

Project Scotfield Island

By SRS

Subject Reach 2 - Insitu Containment Dike - Summary of Slope/W Runs.

Checked By \_\_\_\_\_

Reach 2

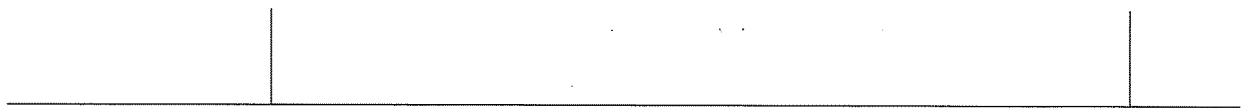
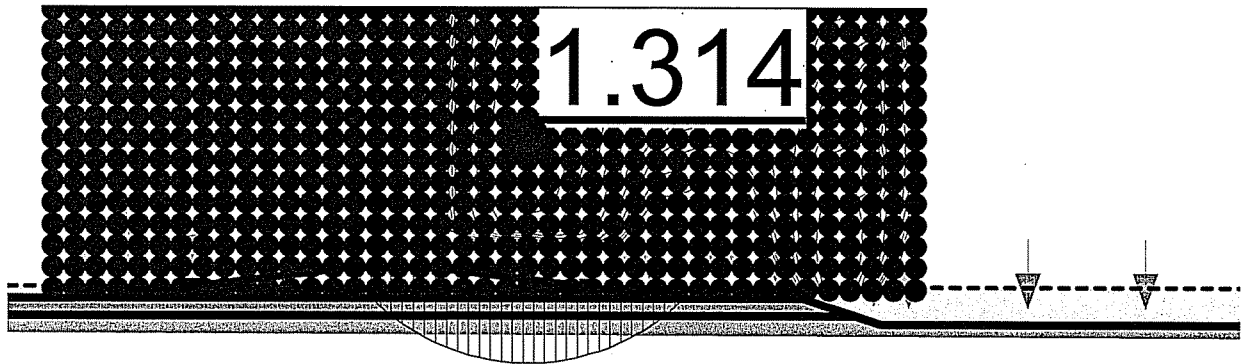
Mudline	Crown EL	Borrow Cut	Borrow Setback	Borrow Side Slopes	Factor of Safety	Slip #	Filename
-2	+6	-8	30'	1V:3H	1.171 (Dike) 1.297 (into Borrow) 1.409 (Borrow sideslopes)	(C) 22099 8129	reach 2 insitu dike crown 6c.gsz " "
-2	+5	-8	30'	1V:3H	✓ 1.314 (Dike) ✓ 1.412 (into Borrow)	30854 (C) 20499	reach 2 insitu dike crown 5c.gsz "
-2	+4	-8	30	1V:3H	1.525 (Dike) 1.553 (into Borrow)	29293 18939	reach 2 insitu dike crown 4c.gsz "
0	+6	-6	30	1V:3H	✓ 1.328 (Dike) 1.188 (into Borrow)	276151 18979	reach 2 insitu dike crown 6d.gsz "
0	+6	-6	35	1V:3H	1.251 (into Borrow)	17259	reach 2 insitu dike crown 6e.gsz
0	+6	-6	40	1V:3H	✓ 1.313 (into Borrow)	17219	reach 2 insitu dike crown 6f.gsz
0	+5	-6	30	1V:3H	1.514 (Dike) 1.269 (into Borrow)	26013 17419	reach 2 insitu dike crown 5d.gsz "
0	+5	-6	35	1V:3H	1.338 (into Borrow)	15659	reach 2 insitu dike crown 5e.gsz
0	+4	-6	30	1V:3H	1.793 (Dike) 1.364 (into Borrow)	26053 15819	reach 2 insitu dike crown 4d.gsz "

reach 2 insitu dike crown 5c .95z

Mudline EL -2

Crown +5

slip # 30854



6.0

6.2

Distance (x 1000)

# SLOPE/W Analysis

---

Report generated using GeoStudio 2007, version 7.13. Copyright © 1991-2008 GEO-SLOPE International Ltd.

## File Information

Created By: Gwen Sanders  
Revision Number: 53  
Last Edited By: Shaun Simon  
Date: 3/16/2009  
Time: 16:57:27  
File Name: reach2insitudikecrown5c.gsz  
Directory: U:\sys\WPDATA\Projects\9292\Slope Stability\Reach 2 - Thin Barrier Beach\mudline el-2\  
Last Solved Date: 3/16/2009  
Last Solved Time: 4:59:28 PM

## Project Settings

Length(L) Units: feet  
Time(t) Units: Seconds  
Force(F) Units: lbf  
Pressure(p) Units: psf  
Strength Units: psf  
Unit Weight of Water: 62.4 pcf  
View: 2D

## Analysis Settings

### SLOPE/W Analysis

Description: EE 19292 Scofield Island - Beach and Dune Features - Marsh Reach

Kind: SLOPE/W

Method: Spencer

#### Settings

Apply Phreatic Correction: No

PWP Conditions Source: Piezometric Line

Use Staged Rapid Drawdown: No

#### SlipSurface

Direction of movement: Left to Right

Use Passive Mode: No

Slip Surface Option: Grid and Radius

Critical slip surfaces saved: 1

Optimize Critical Slip Surface Location: No

#### Tension Crack

Tension Crack Option: (none)



## FOS Distribution

FOS Calculation Option: Constant

### Advanced

Number of Slices: 30

Optimization Tolerance: 0.01

Minimum Slip Surface Depth: 0.1 ft

Optimization Maximum Iterations: 2000

Optimization Convergence Tolerance: 1e-007

Starting Optimization Points: 8

Ending Optimization Points: 16

Complete Passes per Insertion: 1

Driving Side Maximum Convex Angle: 5 °

Resisting Side Maximum Convex Angle: 1 °

## Materials

### In situ Containment Dike

Model: Undrained (Phi=0)

Unit Weight: 88 pcf

Cohesion: 100 psf

Pore Water Pressure

Piezometric Line: 1

### Existing Barrier Beach

Model: Mohr-Coulomb

Unit Weight: 120 pcf

Cohesion: 0 psf

Phi: 25 °

Phi-B: 0 °

Pore Water Pressure

Piezometric Line: 1

### Humus/Clay - Marsh (-6 to -22)

Model: S=f(depth)

Unit Weight: 88 pcf

C-Top of Layer: 50 psf

C-Rate of Increase: 4.375

Limiting C: 120 psf

Pore Water Pressure

Piezometric Line: 1

### Silt - Intradelata (-22 to -31)

Model: Mohr-Coulomb

Unit Weight: 116 pcf

Cohesion: 200 psf

Phi: 15 °

Phi-B: 0 °  
Pore Water Pressure  
Piezometric Line: 1

### **Clay 1 - Interdistributary (-31 to -42)**

Model: Mohr-Coulomb  
Unit Weight: 106 pcf  
Cohesion: 220 psf  
Phi: 0 °  
Phi-B: 0 °  
Pore Water Pressure  
Piezometric Line: 1

### **Clay 2 - Interdistributary (-42 to -52)**

Model: Mohr-Coulomb  
Unit Weight: 106 pcf  
Cohesion: 300 psf  
Phi: 0 °  
Phi-B: 0 °  
Pore Water Pressure  
Piezometric Line: 1

### **Slip Surface Grid**

Upper Left: (5949.04, 200) ft  
Lower Left: (6149.99, 200) ft  
Lower Right: (6149.99, 0) ft  
Grid Horizontal Increment: 40  
Grid Vertical Increment: 40  
Left Projection Angle: 0 °  
Right Projection Angle: 0 °

### **Slip Surface Radius**

Upper Left Coordinate: (5900, -4) ft  
Upper Right Coordinate: (6400, -4) ft  
Lower Left Coordinate: (5900, -43) ft  
Lower Right Coordinate: (6400, -43) ft  
Number of Increments: 39  
Left Projection: No  
Left Projection Angle: 135 °  
Right Projection: No  
Right Projection Angle: 45 °  
UsePoints: 0

## Slip Surface Limits

Left Coordinate: (4800, -6.5) ft

Right Coordinate: (7400, -8) ft

## Piezometric Lines

### Piezometric Line 1

#### Coordinates

	X (ft)	Y (ft)
	4800	0.55
	5982.4	0.55
	6073.6	0.55
	7400	0.55

## Regions

	Material	Points	Area (ft <sup>2</sup> )
Region 1	Existing Barrier Beach	1,2,14,21,22,23	2655
Region 2	Humus/Clay - Marsh (-6 to -22)	1,23,24,25,7,6	38744
Region 3	Silt - Intradelta (-22 to -31)	6,7,9,8	23400
Region 4	Clay 1 - Interdistributary (-31 to -42)	8,9,11,10	28600
Region 5	Clay 2 - Interdistributary (-42 to -52)	10,11,13,12	26000
Region 6	In situ Containment Dike	14,15,16,17,19,21	532

## Points

	X (ft)	Y (ft)
Point 1	4800	-6.5
Point 2	5100	-6
Point 3	7400	-2
Point 4	4800	-6
Point 5	7400	-6
Point 6	4800	-22
Point 7	7400	-22
Point 8	4800	-31
Point 9	7400	-31

Point 10	4800	-42
Point 11	7400	-42
Point 12	4800	-52
Point 13	7400	-52
Point 14	5962	-2
Point 15	5982.4	0.55
Point 16	6018	5
Point 17	6038	5
Point 18	4800	0.55
Point 19	6073.6	0.55
Point 20	7400	0.55
Point 21	6094	-2
Point 22	6124	-2
Point 23	6136	-6
Point 24	6142	-8
Point 25	7400	-8

### Critical Slip Surfaces

	Number	FOS	Center (ft)	Radius (ft)	Entry (ft)	Exit (ft)
1	30854	1.314	(6059.56, 35)	52	(6017.16, 4.89538)	(6096.1, -2)

### Slices of Slip Surface: 30854

	Slip Surface	X (ft)	Y (ft)	PWP (psf)	Base Normal Stress (psf)	Frictional Strength (psf)	Cohesive Strength (psf)
1	30854	6017.5815	4.322723	-235.42026	45.762964	0	100
2	30854	6019.3055	2.1500315	-99.840761	154.79833	0	100
3	30854	6021.818	-0.725	79.559158	414.62813	0	100
4	30854	6024.1765	-3.0701685	225.89714	607.99497	178.17515	0
5	30854	6026.479	-5.0904475	351.9695	833.39865	224.4941	0
6	30854	6028.9265	-6.986372	470.27047	1134.9497	0	54.14
7	30854	6031.519	-8.7628205	581.11835	1291.2695	0	61.916

8	30854	6034.111 5	-10.321378	678.37882	1429.9095	0	68.739
9	30854	6036.704	-11.683995	763.40294	1552.4767	0	74.705
10	30854	6039.271 5	-12.85776	836.64769	1645.453	0	79.844
11	30854	6041.814 5	-13.858685	899.11737	1709.9678	0	84.228
12	30854	6044.357 5	-14.70945	952.18075	1761.6848	0	87.954
13	30854	6046.9	-15.41768	996.36551	1801.2235	0	91.056
14	30854	6049.442 5	-15.989335	1032.0727	1829.0399	0	93.562
15	30854	6051.985 5	-16.428985	1059.4852	1845.3841	0	95.489
16	30854	6054.528 5	-16.74	1078.8833	1850.5486	0	96.854
17	30854	6057.071 5	-16.924695	1090.4312	1844.6591	0	97.666
18	30854	6059.614 5	-16.984425	1094.1626	1827.7466	0	97.932
19	30854	6062.157 5	-16.91962	1090.0893	1799.7824	0	97.652
20	30854	6064.7	-16.72981	1078.2796	1760.7647	0	96.826
21	30854	6067.242 5	-16.41361	1058.5464	1710.3754	0	95.447
22	30854	6069.785 5	-15.96866	1030.7672	1648.4256	0	93.504
23	30854	6072.328 5	-15.39154	994.7438	1574.5345	0	90.984
24	30854	6074.880 5	-14.67461	950.00478	1497.8864	0	87.851
25	30854	6077.441 5	-13.81079	896.13108	1417.8444	0	84.076
26	30854	6080.002	-12.794235	832.67119	1323.7788	0	79.633
27	30854	6082.562 5	-11.61491	759.08532	1214.5855	0	74.478
28	30854	6085.123 5	-10.260033	674.54256	1088.9656	0	68.554

29	30854	6087.684 5	-8.713204	578.00867	945.17555	0	61.791
30	30854	6090.245 5	-6.953069	468.20086	780.91264	0	54.095
31	30854	6092.763	-4.989458	345.664	616.98396	126.51858	0
32	30854	6095.05	-2.9811355	220.34144	316.88586	45.019402	0

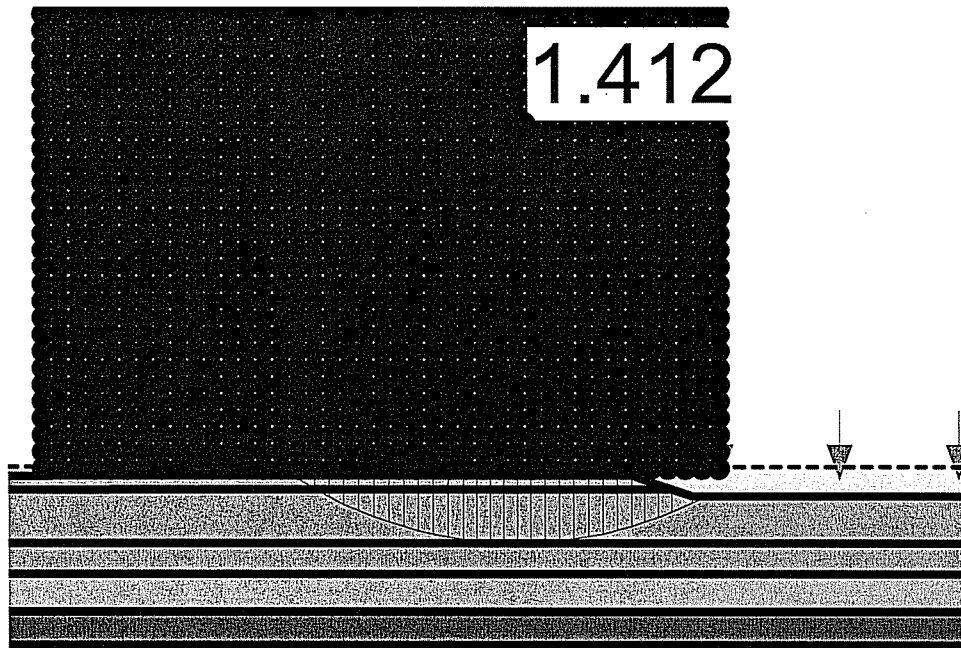
reach 2 insitu dike crown 5c.gsz

Mudline EL - 2

Crown + 5

Borrow Cut to EL - 8  
w/v: 3H 5/5

Slip # 20499



6.0

6.2

Distance (x 1000)

# SLOPE/W Analysis

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## File Information

Created By: Gwen Sanders  
Revision Number: 54  
Last Edited By: Rebecca Scherer  
Date: 3/23/2009  
Time: 12:43:35 PM  
File Name: reach2insitudikecrown5c.gsz  
Directory: U:\sys\WPDATA\Projects\9292\Slope Stability\Reach 2 - Thin Barrier Beach\mudline el-2\  
Last Solved Date: 3/23/2009  
Last Solved Time: 12:43:50 PM

## Project Settings

Length(L) Units: feet  
Time(t) Units: Seconds  
Force(F) Units: lbf  
Pressure(p) Units: psf  
Strength Units: psf  
Unit Weight of Water: 62.4 pcf  
View: 2D

## Analysis Settings

### SLOPE/W Analysis-Reported FS(into borrow)

Description: EE 19292 Scofield Island - Beach and Dune Features - Marsh Reach  
Kind: SLOPE/W  
Method: Spencer  
Settings  
    Apply Phreatic Correction: No  
    PWP Conditions Source: Piezometric Line  
    Use Staged Rapid Drawdown: No  
SlipSurface  
    Direction of movement: Left to Right  
    Use Passive Mode: No  
    Slip Surface Option: Grid and Radius  
    Critical slip surfaces saved: 1  
    Optimize Critical Slip Surface Location: No  
Tension Crack  
    Tension Crack Option: (none)



## FOS Distribution

FOS Calculation Option: Constant

### Advanced

Number of Slices: 30

Optimization Tolerance: 0.01

Minimum Slip Surface Depth: 0.1 ft

Optimization Maximum Iterations: 2000

Optimization Convergence Tolerance: 1e-007

Starting Optimization Points: 8

Ending Optimization Points: 16

Complete Passes per Insertion: 1

Driving Side Maximum Convex Angle: 5 °

Resisting Side Maximum Convex Angle: 1 °

## Materials

### In situ Containment Dike

Model: Undrained (Phi=0)

Unit Weight: 88 pcf

Cohesion: 100 psf

Pore Water Pressure

Piezometric Line: 1

### Existing Barrier Beach

Model: Mohr-Coulomb

Unit Weight: 120 pcf

Cohesion: 0 psf

Phi: 25 °

Phi-B: 0 °

Pore Water Pressure

Piezometric Line: 1

### Humus/Clay - Marsh (-6 to -22)

Model: S=f(depth)

Unit Weight: 88 pcf

C-Top of Layer: 50 psf

C-Rate of Increase: 4.375

Limiting C: 120 psf

Pore Water Pressure

Piezometric Line: 1

### Silt - Intradelta (-22 to -31)

Model: Mohr-Coulomb

Unit Weight: 116 pcf

Cohesion: 200 psf

Phi: 15 °

Phi-B: 0 °  
Pore Water Pressure  
Piezometric Line: 1

### **Clay 1 - Interdistributary (-31 to -42)**

Model: Mohr-Coulomb  
Unit Weight: 106 pcf  
Cohesion: 220 psf  
Phi: 0 °  
Phi-B: 0 °  
Pore Water Pressure  
Piezometric Line: 1

### **Clay 2 - Interdistributary (-42 to -52)**

Model: Mohr-Coulomb  
Unit Weight: 106 pcf  
Cohesion: 300 psf  
Phi: 0 °  
Phi-B: 0 °  
Pore Water Pressure  
Piezometric Line: 1

### **Slip Surface Grid**

Upper Left: (6089.7, 100) ft  
Lower Left: (6089.7, 100) ft  
Lower Right: (6089.7, 100) ft  
Grid Horizontal Increment: 0  
Grid Vertical Increment: 0  
Left Projection Angle: 0 °  
Right Projection Angle: 0 °

### **Slip Surface Radius**

Upper Left Coordinate: (5900, -22) ft  
Upper Right Coordinate: (6400, -22) ft  
Lower Left Coordinate: (5900, -22) ft  
Lower Right Coordinate: (6400, -22) ft  
Number of Increments: 0  
Left Projection: No  
Left Projection Angle: 135 °  
Right Projection: No  
Right Projection Angle: 45 °  
UsePoints: 0

## Slip Surface Limits

Left Coordinate: (4800, -6.5) ft

Right Coordinate: (7400, -8) ft

## Piezometric Lines

### Piezometric Line 1

#### Coordinates

	X (ft)	Y (ft)
	4800	0.55
	5982.4	0.55
	6073.6	0.55
	7400	0.55

## Regions

	Material	Points	Area (ft <sup>2</sup> )
Region 1	Existing Barrier Beach	1,2,14,21,22,23	2655
Region 2	Humus/Clay - Marsh (-6 to -22)	1,23,24,25,7,6	38744
Region 3	Silt - Intradelta (-22 to -31)	6,7,9,8	23400
Region 4	Clay 1 - Interdistributary (-31 to -42)	8,9,11,10	28600
Region 5	Clay 2 - Interdistributary (-42 to -52)	10,11,13,12	26000
Region 6	In situ Containment Dike	14,15,16,17,19,21	532

## Points

	X (ft)	Y (ft)
Point 1	4800	-6.5
Point 2	5100	-6
Point 3	7400	-2
Point 4	4800	-6
Point 5	7400	-6
Point 6	4800	-22
Point 7	7400	-22
Point 8	4800	-31
Point 9	7400	-31

Point 10	4800	-42
Point 11	7400	-42
Point 12	4800	-52
Point 13	7400	-52
Point 14	5962	-2
Point 15	5982.4	0.55
Point 16	6018	5
Point 17	6038	5
Point 18	4800	0.55
Point 19	6073.6	0.55
Point 20	7400	0.55
Point 21	6094	-2
Point 22	6124	-2
Point 23	6136	-6
Point 24	6142	-8
Point 25	7400	-8

### Critical Slip Surfaces

	Number	FOS	Center (ft)	Radius (ft)	Entry (ft)	Exit (ft)
1	1	1.412	(6089.7, 100)	122	(6013.81, 4.47631)	(6146.44, -8)

### Slices of Slip Surface: 1

	Slip Surface	X (ft)	Y (ft)	PWP (psf)	Base Normal Stress (psf)	Frictional Strength (psf)	Cohesive Strength (psf)
1	1	6015.905	2.8845565	-145.67707	108.89338	0	100
2	1	6018.517	0.9214035	-23.175419	302.84202	0	100
3	1	6020.9005	-0.725	79.560953	447.10734	0	100
4	1	6026.069	-4.019953	285.16377	753.03541	218.17213	0
5	1	6031.5285	-7.2103555	484.24596	1161.6076	0	55.124
6	1	6035.843	-9.4423475	623.51419	1355.4846	0	64.896
7	1	6040.225	-11.49125	751.36526	1510.7618	0	73.867
8	1	6044.675	-13.36233	868.12289	1627.6025	0	82.061
9	1	6049.125	-15.030875	972.25519	1727.6931	0	89.368

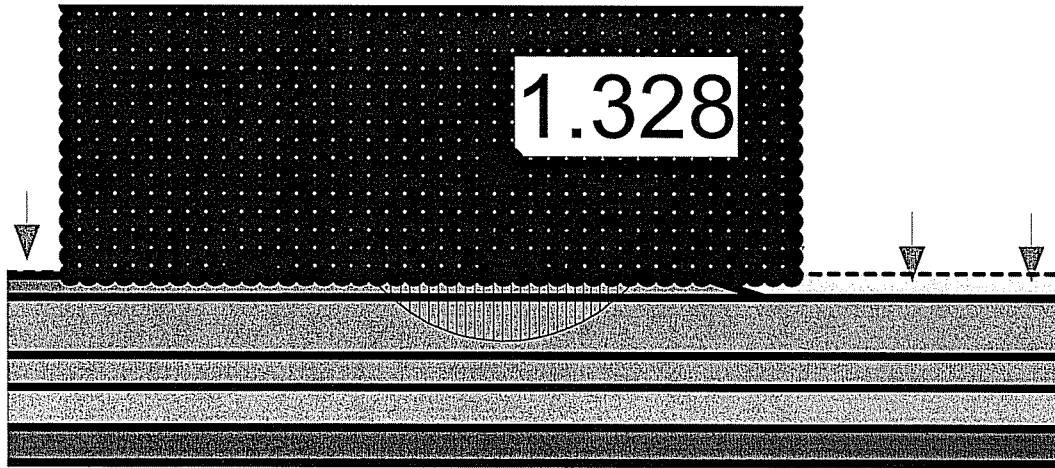
10	1	6053.575	-16.50561	1064.26	1811.6073	0	95.827
11	1	6058.025	-17.793825	1144.6638	1879.8076	0	101.47
12	1	6062.475	-18.90159	1213.7798	1932.7248	0	106.32
13	1	6066.925	-19.833925	1271.9509	1970.6795	0	110.41
14	1	6071.375	-20.5949	1319.438	1993.8757	0	113.75
15	1	6075.612 5	-21.16698	1355.1495	2018.4847	0	116.26
16	1	6079.637 5	-21.56754	1380.1296	2045.9543	0	118.02
17	1	6083.662 5	-21.83385	1396.7506	2061.7505	0	119.19
18	1	6087.687 5	-21.96679	1405.0412	2065.8791	0	119.78
19	1	6091.85	-21.9621	1404.7536	2057.6141	0	119.76
20	1	6096.143	-21.81086	1395.3196	2042.242	0	119.11
21	1	6100.428 5	-21.508315	1376.4449	2020.9474	0	117.79
22	1	6104.714	-21.05333	1348.0441	1986.0124	0	115.81
23	1	6109	-20.44418	1310.0442	1937.2231	0	113.15
24	1	6113.286	-19.678505	1262.2512	1874.2955	0	109.81
25	1	6117.571 5	-18.75327	1204.5196	1796.8184	0	105.77
26	1	6121.857	-17.66471	1136.6046	1704.3892	0	101.01
27	1	6126	-16.455665	1061.1439	1564.1491	0	95.727
28	1	6130	-15.132185	978.55882	1372.5921	0	89.943
29	1	6134	-13.65255	886.2311	1166.1671	0	83.477
30	1	6139	-11.547085	754.86148	914.8187	0	69.893
31	1	6144.222 5	-9.110595	602.82388	663.82069	0	54.859

reach 2 inside dike crown bd. gsz

Mudline EL 0

Crown +6

Slip # 27615



6.0

6.2

Distance (x 1000)

# SLOPE/W Analysis

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## File Information

Created By: Gwen Sanders  
Revision Number: 55  
Last Edited By: Rebecca Scherer  
Date: 3/23/2009  
Time: 12:36:26 PM  
File Name: reach2insitudikecrown6d.gsz  
Directory: U:\sys\WPDATA\Projects\9292\Slope Stability\Reach 2 - Thin Barrier Beach\mudline  
el0\  
Last Solved Date: 3/23/2009  
Last Solved Time: 12:36:39 PM

## Project Settings

Length(L) Units: feet  
Time(t) Units: Seconds  
Force(F) Units: lbf  
Pressure(p) Units: psf  
Strength Units: psf  
Unit Weight of Water: 62.4 pcf  
View: 2D

## Analysis Settings

### SLOPE/W Analysis-Reported FS(dike)

Description: EE 19292 Scofield Island - Beach and Dune Features - Marsh Reach

Kind: SLOPE/W

Method: Spencer

#### Settings

Apply Phreatic Correction: No

PWP Conditions Source: Piezometric Line

Use Staged Rapid Drawdown: No

#### SlipSurface

Direction of movement: Left to Right

Use Passive Mode: No

Slip Surface Option: Grid and Radius

Critical slip surfaces saved: 1

Optimize Critical Slip Surface Location: No

Tension Crack

Tension Crack Option: (none)

## FOS Distribution

FOS Calculation Option: Constant

### Advanced

Number of Slices: 30

Optimization Tolerance: 0.01

Minimum Slip Surface Depth: 0.1 ft

Optimization Maximum Iterations: 2000

Optimization Convergence Tolerance: 1e-007

Starting Optimization Points: 8

Ending Optimization Points: 16

Complete Passes per Insertion: 1

Driving Side Maximum Convex Angle: 5 °

Resisting Side Maximum Convex Angle: 1 °

## Materials

### In situ Containment Dike

Model: Undrained (Phi=0)

Unit Weight: 88 pcf

Cohesion: 100 psf

Pore Water Pressure

Piezometric Line: 1

### Existing Barrier Beach

Model: Mohr-Coulomb

Unit Weight: 120 pcf

Cohesion: 0 psf

Phi: 25 °

Phi-B: 0 °

Pore Water Pressure

Piezometric Line: 1

### Humus/Clay - Marsh (-6 to -22)

Model: S=f(depth)

Unit Weight: 88 pcf

C-Top of Layer: 50 psf

C-Rate of Increase: 4.375

Limiting C: 120 psf

Pore Water Pressure

Piezometric Line: 1

### Silt - Intradelata (-22 to -31)

Model: Mohr-Coulomb

Unit Weight: 116 pcf

Cohesion: 0 psf

Phi: 15 °



Phi-B: 0 °  
Pore Water Pressure  
Piezometric Line: 1

### **Clay 1 - Interdistributary (-31 to -42)**

Model: Mohr-Coulomb  
Unit Weight: 106 pcf  
Cohesion: 220 psf  
Phi: 0 °  
Phi-B: 0 °  
Pore Water Pressure  
Piezometric Line: 1

### **Clay 2 - Interdistributary (-42 to -52)**

Model: Mohr-Coulomb  
Unit Weight: 106 pcf  
Cohesion: 300 psf  
Phi: 0 °  
Phi-B: 0 °  
Pore Water Pressure  
Piezometric Line: 1

### **Slip Surface Grid**

Upper Left: (6069.61, 30) ft  
Lower Left: (6069.61, 30) ft  
Lower Right: (6069.61, 30) ft  
Grid Horizontal Increment: 0  
Grid Vertical Increment: 0  
Left Projection Angle: 0 °  
Right Projection Angle: 0 °

### **Slip Surface Radius**

Upper Left Coordinate: (5900, -18) ft  
Upper Right Coordinate: (6400, -18) ft  
Lower Left Coordinate: (5900, -18) ft  
Lower Right Coordinate: (6400, -18) ft  
Number of Increments: 0  
Left Projection: No  
Left Projection Angle: 135 °  
Right Projection: No  
Right Projection Angle: 45 °  
UsePoints: 0

## Slip Surface Limits

Left Coordinate: (4800, -6.5) ft

Right Coordinate: (7400, -6) ft

## Piezometric Lines

### Piezometric Line 1

#### Coordinates

	X (ft)	Y (ft)
	4800	0.55
	5982.4	0.55
	6089.6	0.55
	7400	0.55

## Regions

	Material	Points	Area (ft <sup>2</sup> )
Region 1	Existing Barrier Beach	1,2,14,21,22,23	3824.5
Region 2	Humus/Clay - Marsh (-6 to -22)	1,23,5,7,6	41264.5
Region 3	Silt - Intradelta (-22 to -31)	6,7,9,8	23400
Region 4	Clay 1 - Interdistributary (-31 to -42)	8,9,11,10	28600
Region 5	Clay 2 - Interdistributary (-42 to -52)	10,11,13,12	26000
Region 6	In situ Containment Dike	14,15,16,17,19,21	408

## Points

	X (ft)	Y (ft)
Point 1	4800	-6.5
Point 2	5100	-6
Point 3	7400	-2
Point 4	4800	-6
Point 5	7400	-6
Point 6	4800	-22
Point 7	7400	-22
Point 8	4800	-31
Point 9	7400	-31

Point 10	4800	-42
Point 11	7400	-42
Point 12	4800	-52
Point 13	7400	-52
Point 14	5978	0
Point 15	5982.4	0.55
Point 16	6026	6
Point 17	6046	6
Point 18	4800	0.55
Point 19	6089.6	0.55
Point 20	7400	0.55
Point 21	6094	0
Point 22	6124	0
Point 23	6142	-6

### Critical Slip Surfaces

	Number	FOS	Center (ft)	Radius (ft)	Entry (ft)	Exit (ft)
1	1	1.328	(6069.61, 30)	48	(6028.04, 6)	(6107.08, 0)

### Slices of Slip Surface: 1

	Slip Surface	X (ft)	Y (ft)	PWP (psf)	Base Normal Stress (psf)	Frictional Strength (psf)	Cohesive Strength (psf)
1	1	6029.8735	3.275	-170.03807	125.74869	0	100
2	1	6031.923	0.275	17.159412	400.31966	0	100
3	1	6033.5815	-1.641107	136.72473	550.53882	192.96468	0
4	1	6036.464	-4.660499	325.14137	886.64205	261.83207	0
5	1	6039.254	-7.1414595	479.95597	1297.0806	0	54.826
6	1	6041.9525	-9.1961275	608.15529	1478.1468	0	63.82
7	1	6044.651	-10.97004	718.85318	1636.2431	0	71.585

8	1	6047.2825	- 12.46619	812.22559	1757.113	0	78.135
9	1	6049.847	- 13.72008 5	890.43987	1843.4472	0	83.625
10	1	6052.4115	- 14.79212	957.35492	1914.3458	0	88.319
11	1	6054.9765	- 15.69515	1013.6988	1970.8438	0	92.274
12	1	6057.5415	- 16.43907 5	1060.1151	2013.6601	0	95.533
13	1	6060.106	- 17.03148	1097.0943	2043.3793	0	98.129
14	1	6062.6705	- 17.47805	1124.9419	2060.4516	0	100.09
15	1	6065.2355	- 17.78288 5	1143.9634	2065.1681	0	101.42
16	1	6067.8	- 17.94869 5	1154.3199	2057.6786	0	102.15
17	1	6070.3645	- 17.97693	1156.091	2038.0714	0	102.28
18	1	6072.9295	- 17.86783	1149.263	2006.2897	0	101.81
19	1	6075.494	- 17.62045	1133.8475	1962.2244	0	100.73
20	1	6078.0585	- 17.23262	1109.6509	1905.6472	0	99.038
21	1	6080.6235	- 16.70082	1076.4457	1836.0865	0	96.716
22	1	6083.1885	- 16.02004	1033.952	1753.1335	0	93.742
23	1	6085.753	- 15.18352 5	981.75935	1656.0075	0	90.086
24	1	6088.3175	-14.1824	919.30846	1543.8379	0	85.711

25	1	6090.7	- 13.10118 5	851.85226	1434.3977	0	80.984
26	1	6092.9	- 11.95227	780.15929	1328.7467	0	75.961
27	1	6095.2235	- 10.56904 5	693.83019	1205.2626	0	69.913
28	1	6097.671	- 8.914125 5	590.54968	1060.3578	0	62.677
29	1	6100.1185	- 7.023455	472.58005	893.03312	0	54.409
30	1	6102.7765	- 4.641703 5	323.96541	751.02674	199.14197	0
31	1	6105.6455	- 1.634129 5	136.28892	305.77686	79.033524	0

reach 2 insitu like crown bf. qst

Mudline EL 0

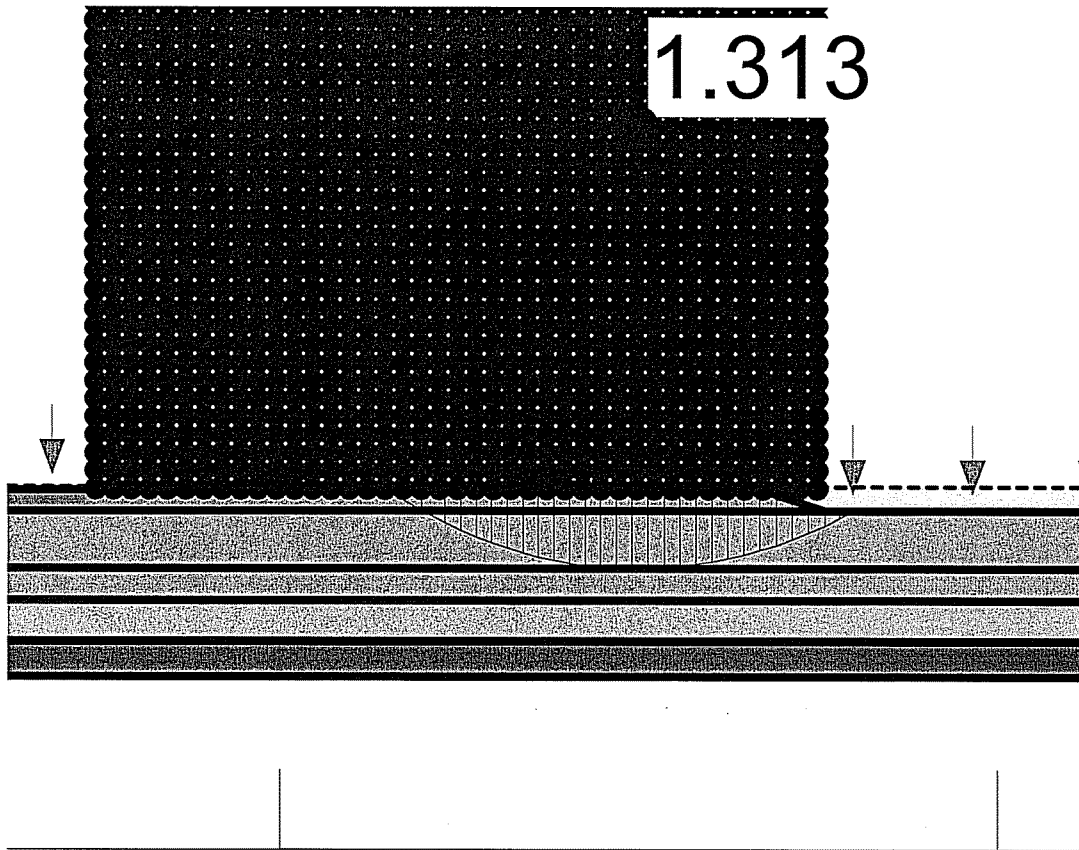
Crown +6

Borrow Cut to EL -6

w/1V:3H S/S

40' setback

Slip # 17219



6.0

6.2

Distance (x 1000)

# SLOPE/W Analysis

---

Report generated using GeoStudio 2007, version 7.13. Copyright © 1991-2008 GEO-SLOPE International Ltd.

## File Information

Created By: Gwen Sanders  
Revision Number: 59  
Last Edited By: Rebecca Scherer  
Date: 3/23/2009  
Time: 12:39:42 PM  
File Name: reach2insitudikecrown6f.gsz  
Directory: U:\sys\WPDATA\Projects\9292\Slope Stability\Reach 2 - Thin Barrier Beach\mudline  
el0\  
Last Solved Date: 3/23/2009  
Last Solved Time: 12:39:54 PM

## Project Settings

Length(L) Units: feet  
Time(t) Units: Seconds  
Force(F) Units: lbf  
Pressure(p) Units: psf  
Strength Units: psf  
Unit Weight of Water: 62.4 pcf  
View: 2D

## Analysis Settings

### SLOPE/W Analysis-Reported FS(into borrow)

Description: EE 19292 Scofield Island - Beach and Dune Features - Marsh Reach

Kind: SLOPE/W

Method: Spencer

#### Settings

Apply Phreatic Correction: No

PWP Conditions Source: Piezometric Line

Use Staged Rapid Drawdown: No

#### SlipSurface

Direction of movement: Left to Right

Use Passive Mode: No

Slip Surface Option: Grid and Radius

Critical slip surfaces saved: 1

Optimize Critical Slip Surface Location: No

#### Tension Crack

Tension Crack Option: (none)

## FOS Distribution

FOS Calculation Option: Constant

### Advanced

Number of Slices: 30

Optimization Tolerance: 0.01

Minimum Slip Surface Depth: 0.1 ft

Optimization Maximum Iterations: 2000

Optimization Convergence Tolerance: 1e-007

Starting Optimization Points: 8

Ending Optimization Points: 16

Complete Passes per Insertion: 1

Driving Side Maximum Convex Angle: 5 °

Resisting Side Maximum Convex Angle: 1 °

## Materials

### In situ Containment Dike

Model: Undrained ( $\Phi=0$ )

Unit Weight: 88 pcf

Cohesion: 100 psf

Pore Water Pressure

Piezometric Line: 1

### Existing Barrier Beach

Model: Mohr-Coulomb

Unit Weight: 120 pcf

Cohesion: 0 psf

$\Phi$ : 25 °

$\Phi$ -B: 0 °

Pore Water Pressure

Piezometric Line: 1

### Humus/Clay - Marsh (-6 to -22)

Model:  $S=f(\text{depth})$

Unit Weight: 88 pcf

C-Top of Layer: 50 psf

C-Rate of Increase: 4.375

Limiting C: 120 psf

Pore Water Pressure

Piezometric Line: 1

### Silt - Intradelata (-22 to -31)

Model: Mohr-Coulomb

Unit Weight: 116 pcf

Cohesion: 0 psf

$\Phi$ : 15 °



Phi-B: 0 °  
Pore Water Pressure  
Piezometric Line: 1

### **Clay 1 - Interdistributary (-31 to -42)**

Model: Mohr-Coulomb  
Unit Weight: 106 pcf  
Cohesion: 220 psf  
Phi: 0 °  
Phi-B: 0 °  
Pore Water Pressure  
Piezometric Line: 1

### **Clay 2 - Interdistributary (-42 to -52)**

Model: Mohr-Coulomb  
Unit Weight: 106 pcf  
Cohesion: 300 psf  
Phi: 0 °  
Phi-B: 0 °  
Pore Water Pressure  
Piezometric Line: 1

### **Slip Surface Grid**

Upper Left: (6099.75, 100) ft  
Lower Left: (6099.75, 100) ft  
Lower Right: (6099.75, 100) ft  
Grid Horizontal Increment: 0  
Grid Vertical Increment: 0  
Left Projection Angle: 0 °  
Right Projection Angle: 0 °

### **Slip Surface Radius**

Upper Left Coordinate: (5900, -22) ft  
Upper Right Coordinate: (6400, -22) ft  
Lower Left Coordinate: (5900, -22) ft  
Lower Right Coordinate: (6400, -22) ft  
Number of Increments: 0  
Left Projection: No  
Left Projection Angle: 135 °  
Right Projection: No  
Right Projection Angle: 45 °  
UsePoints: 0

## Slip Surface Limits

Left Coordinate: (4800, -6.5) ft

Right Coordinate: (7400, -6) ft

## Piezometric Lines

### Piezometric Line 1

#### Coordinates

	X (ft)	Y (ft)
	4800	0.55
	5982.4	0.55
	6089.6	0.55
	7400	0.55

## Regions

	Material	Points	Area (ft <sup>2</sup> )
Region 1	Existing Barrier Beach	1,2,14,21,22,23	3887
Region 2	Humus/Clay - Marsh (-6 to -22)	1,23,5,7,6	41262
Region 3	Silt - Intradelta (-22 to -31)	6,7,9,8	23400
Region 4	Clay 1 - Interdistributary (-31 to -42)	8,9,11,10	28600
Region 5	Clay 2 - Interdistributary (-42 to -52)	10,11,13,12	26000
Region 6	In situ Containment Dike	14,15,16,17,19,21	408

## Points

	X (ft)	Y (ft)
Point 1	4800	-6.5
Point 2	5100	-6
Point 3	7400	-2
Point 4	4800	-6
Point 5	7400	-6
Point 6	4800	-22
Point 7	7400	-22
Point 8	4800	-31
Point 9	7400	-31

Point 10	4800	-42
Point 11	7400	-42
Point 12	4800	-52
Point 13	7400	-52
Point 14	5978	0
Point 15	5982.4	0.55
Point 16	6026	6
Point 17	6046	6
Point 18	4800	0.55
Point 19	6089.6	0.55
Point 20	7400	0.55
Point 21	6094	0
Point 22	6134	0
Point 23	6152	-6

## Critical Slip Surfaces

	Number	FOS	Center (ft)	Radius (ft)	Entry (ft)	Exit (ft)
1	1	1.313	(6099.75, 100)	122	(6022.51, 5.56391)	(6160.15, -6)

## Slices of Slip Surface: 1

	Slip Surface	X (ft)	Y (ft)	PWP (psf)	Base Normal Stress (psf)	Frictional Strength (psf)	Cohesive Strength (psf)
1	1	6024.2555	4.189419	-227.10044	80.593978	0	100
2	1	6027.542	1.682466	-70.666677	318.35157	0	100
3	1	6029.474	0.275	17.159589	441.68347	0	100
4	1	6032.254	-1.587941	133.40712	600.06283	217.60513	0
5	1	6037.034	-4.6087575	321.91196	937.60155	287.10077	0
6	1	6042.712	-7.7815175	519.88471	1358.5528	0	57.617
7	1	6048.18	-10.538446	691.91415	1575.3627	0	69.688
8	1	6052.54	-12.470555	812.47289	1698.1821	0	78.148
9	1	6056.9	-14.20357	920.63239	1804.6517	0	85.737
10	1	6061.26	-15.746455	1016.9004	1895.3601	0	92.494
11	1	6065.62	-17.106735	1101.7762	1970.7825	0	98.452
12	1	6069.98	-18.290715	1175.6568	2031.3593	0	103.64

13	1	6074.34	-19.30366	1238.8636	2077.4172	0	108.08
14	1	6078.7	-20.1499	1291.6794	2109.2272	0	111.79
15	1	6083.06	-20.832945	1334.3045	2126.9625	0	114.78
16	1	6087.42	-21.35555	1366.9079	2130.7964	0	117.08
17	1	6091.8	-21.72073	1389.6981	2137.8994	0	118.68
18	1	6096.875	-21.93221	1402.8971	2156.2703	0	119.61
19	1	6102.196 5	-21.950925	1404.0502	2165.17	0	119.7
20	1	6107.089 5	-21.75438	1391.7921	2154.5419	0	118.85
21	1	6111.982 5	-21.36033	1367.1983	2126.423	0	117.14
22	1	6116.875	-20.76684	1330.1618	2080.4139	0	114.55
23	1	6121.767 5	-19.970945	1280.5114	2016.0516	0	111.07
24	1	6126.660 5	-18.968575	1217.9678	1932.8927	0	106.7
25	1	6131.553 5	-17.75445	1142.2067	1830.3641	0	101.39
26	1	6136.25	-16.3881	1056.9335	1670.6827	0	95.422
27	1	6140.75	-14.87947	962.8077	1453.5774	0	88.829
28	1	6145.25	-13.171855	856.23489	1217.4873	0	81.366
29	1	6149.75	-11.25608	736.69017	961.33557	0	72.992
30	1	6154.037	-9.2324515	610.42619	732.30043	0	64.142
31	1	6158.111 5	-7.1100015	477.98475	541.28357	0	54.856

## **ANNEX K3**

### **SETTLEMENT ANALYSES**

Project Title: Scofield Island

Subject: Summary - consolidation tests Job No: 19292

By: GPS Date: \_\_\_\_\_ Checked By: \_\_\_\_\_ Date: \_\_\_\_\_



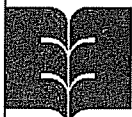
**EUSTIS**  
Metairie, Louisiana  
Lafayette, Louisiana  
Gulfport, Mississippi

Page: \_\_\_\_\_

BORING No. (REACH)	SAMPLE No.	EST. ELEV.	USC (GEOLOGY)	w%	CR	P <sub>c</sub> (psf)	C <sub>v</sub> (ft <sup>2</sup> /day) (@ P <sub>c</sub> )
1 (Reach 2)	6	-17.4	CH (MARSH)	122	0.331	340	0.01 (3.7)
3 (Reach 2)	8	-25.6	SP (Intra Delta)	29	0.093	13,570	22 (8030)
5 (Reach 2)	10	-40.9	CH (Interdistributary)	55	0.262	980	0.04 (14.6)
5 (R-2)	2*	-6.9	CH (Marsh)	111	0.315	560	0.01 (3.7)

\* poor test results, ? dist. sample

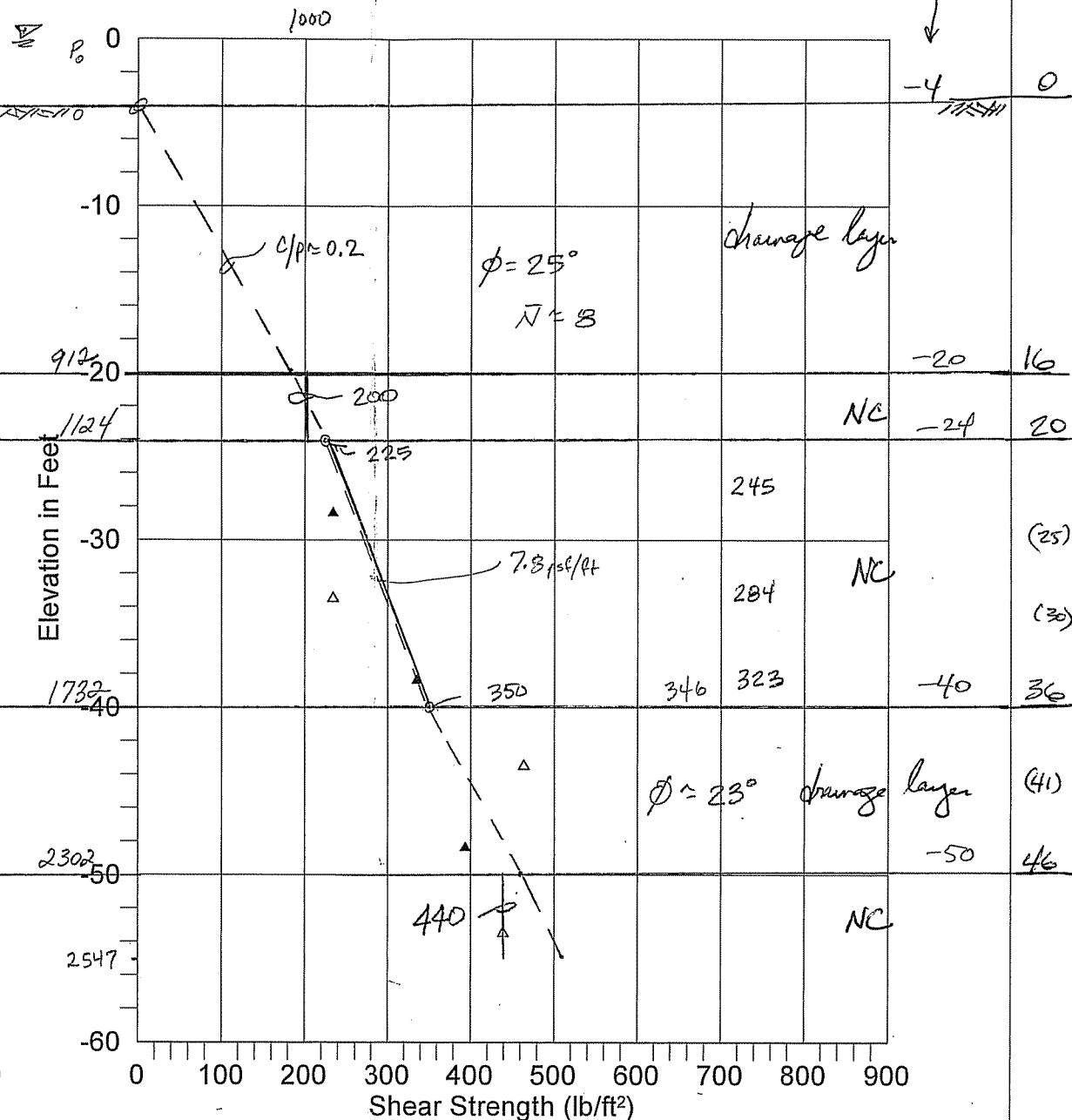
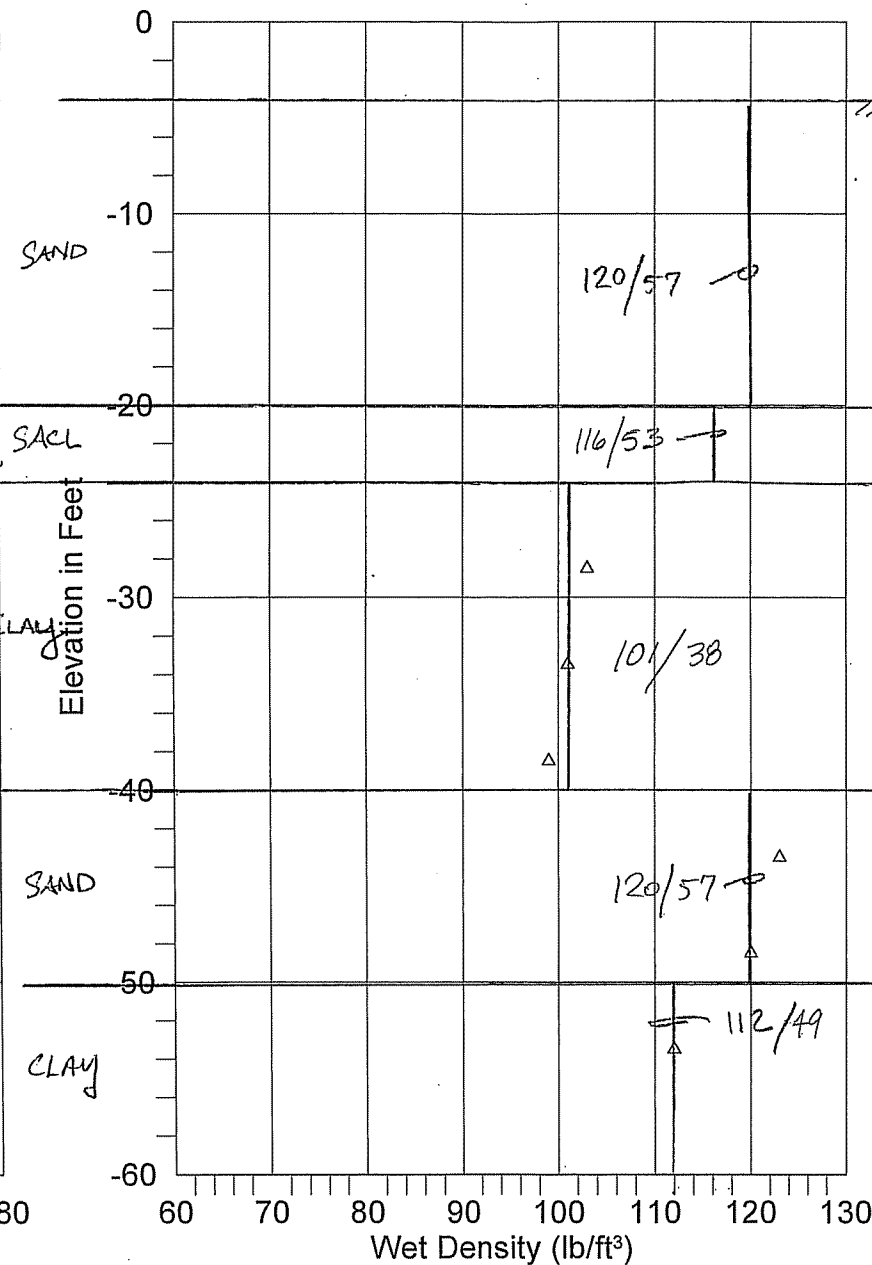
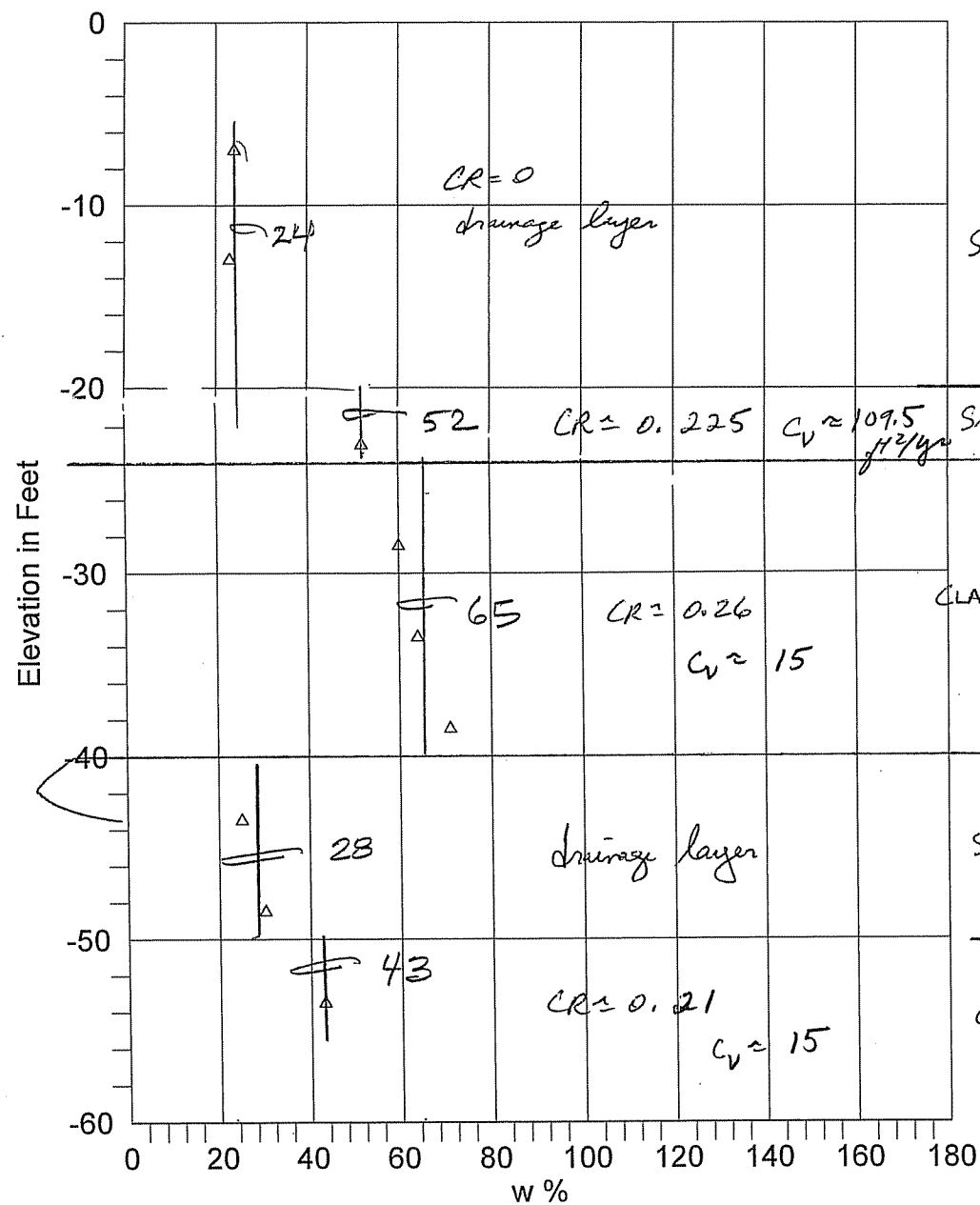
Project Title: Scofield Island  
 Subject: Settlement design values Job No: 19292  
 By: GPS Date: 3/15/09 Checked By: \_\_\_\_\_ Date: \_\_\_\_\_



**EUSTIS**  
 Metairie, Louisiana  
 Lafayette, Louisiana  
 Gulfport, Mississippi

Page: \_\_\_\_\_

GEOLOGIC UNIT	USE	$C_v$	
		ft <sup>2</sup> /day	ft <sup>2</sup> /yr
Barrier Beach	SP	- drainage layer -	
Marsh	OH	0.02	7.3
	CH	0.03	11
Intra delta	ML/SP	- drainage layer - alt 22 8030	
Interdistributary	CH	0.04	15
	CL	0.3	109.5



- B3-OB
- △ B4-OB
- B3
- ▲ B4

# REACH 1



EUSTIS ENGINEERING COMPANY, INC.  
 GEOTECHNICAL ENGINEERS  
 3011 28TH STREET METAIRIE, LOUISIANA

SOIL DESIGN PARAMETERS  
 B-4 (BEST BEACH)

RIVERINE SAND MINING  
 SCOFIELD ISLAND RESTORATION (BA-40)  
 PLAQUEMINES PARISH, LOUISIANA

DRAWN BY: J.T.H.	5 JAN 2009	FILE: COMBINEBORING DATA.GRF
CHECKED BY: G.P.S.	JOB NO.: 19292	



Recompression Virgin total 10% total  
 TOTAL S (ft) = 0.000 0.000 0.000 0.000  
 (in) = 0.000 0.000 0.000 0.000  
 REDUCED S (ft) 0.000 0.000 0.000 0.000  
 (in) = 0.000 0.000 0.000 0.000

Initial Ld  
 s = 0.00  
 s' = 0.00  
 s'' = 0.00

If you would like to compute the capacity by imposing the load 2/3 down the length of the pile place a 1 here

0.00

SETTLEMENT ANALYSIS - WESTERGAARD THEORY

Water Table Depth 0.00  
 Bearing Area Width, Xb (ft) = 500.00 500.00  
 Bearing Area Length, Yb (ft) = 3000.00 3000.00  
 Abscissa for Analysis, Xa (ft) = 250.00 250.00  
 Ordinate for Analysis, Ya (ft) = 1500.00 1500.00  
 Net Applied Pressure (psf) = 0.00 0.00  
 Depth of Application, Da (ft) = 0.00 0.00  
 Depth of Foundation, Df (ft) = 0.00 0.00  
 Poisson's Ratio, u = 0.00 0.00  
 Recompression Factor = 0.15 0.15  
 Contact Pressure Multiplier = 1.00 1.00  
 Total Settlement Multiplier = 1.00 1.00  
 Applied Pressure Criteria = 0.00 0.00%

Region dX dY  
 (ft) (ft)  
 1 250.0 1500.0  
 2 250.0 -1500.0  
 3 -250.0 1500.0  
 4 -250.0 -1500.0  
 Strain Factor = 0.50

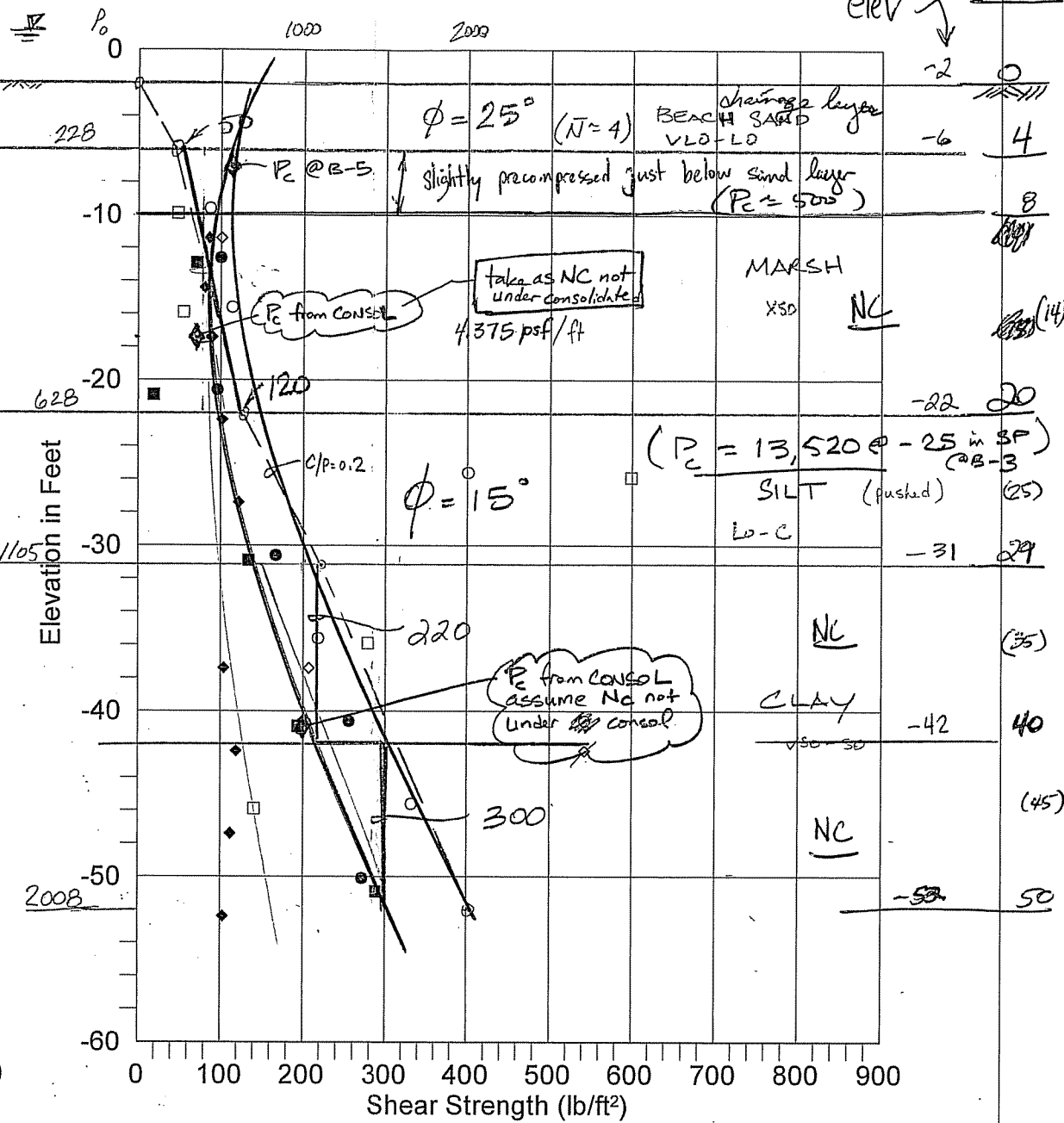
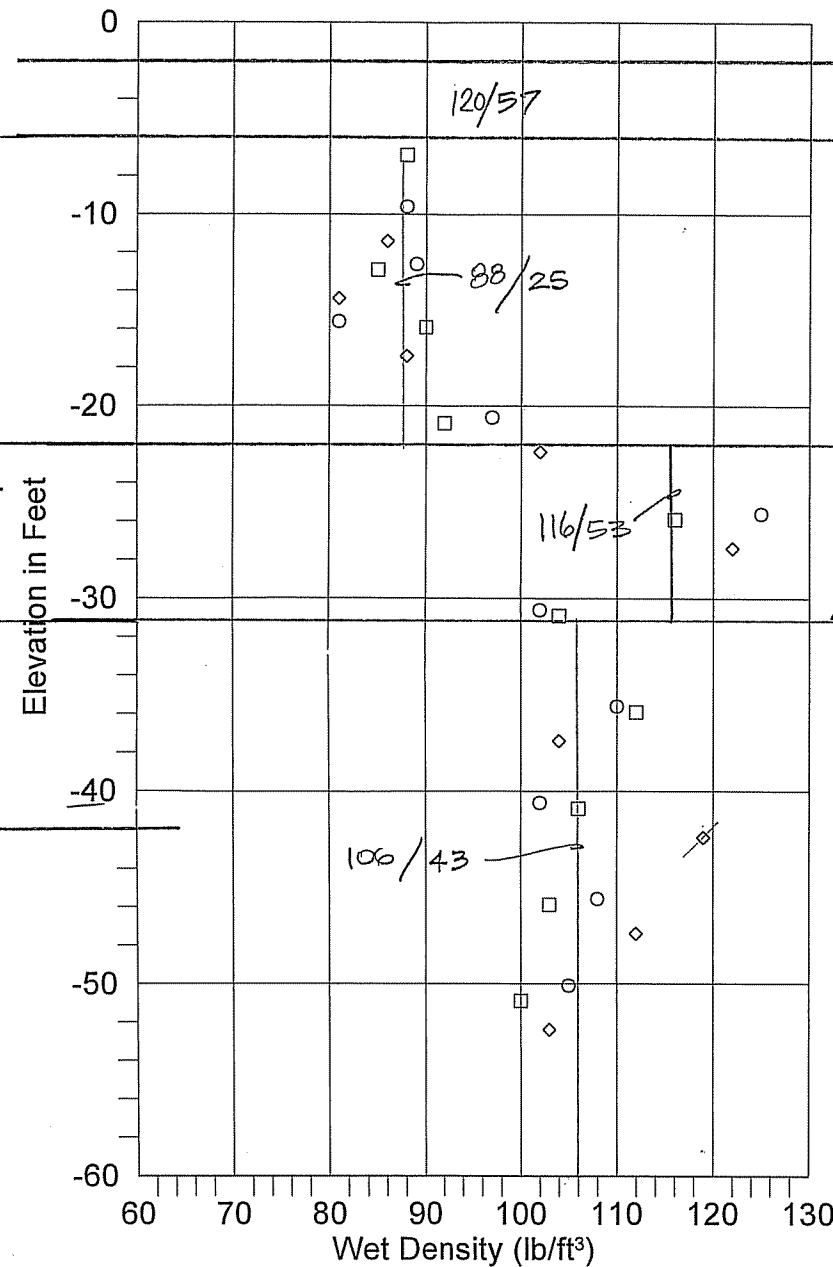
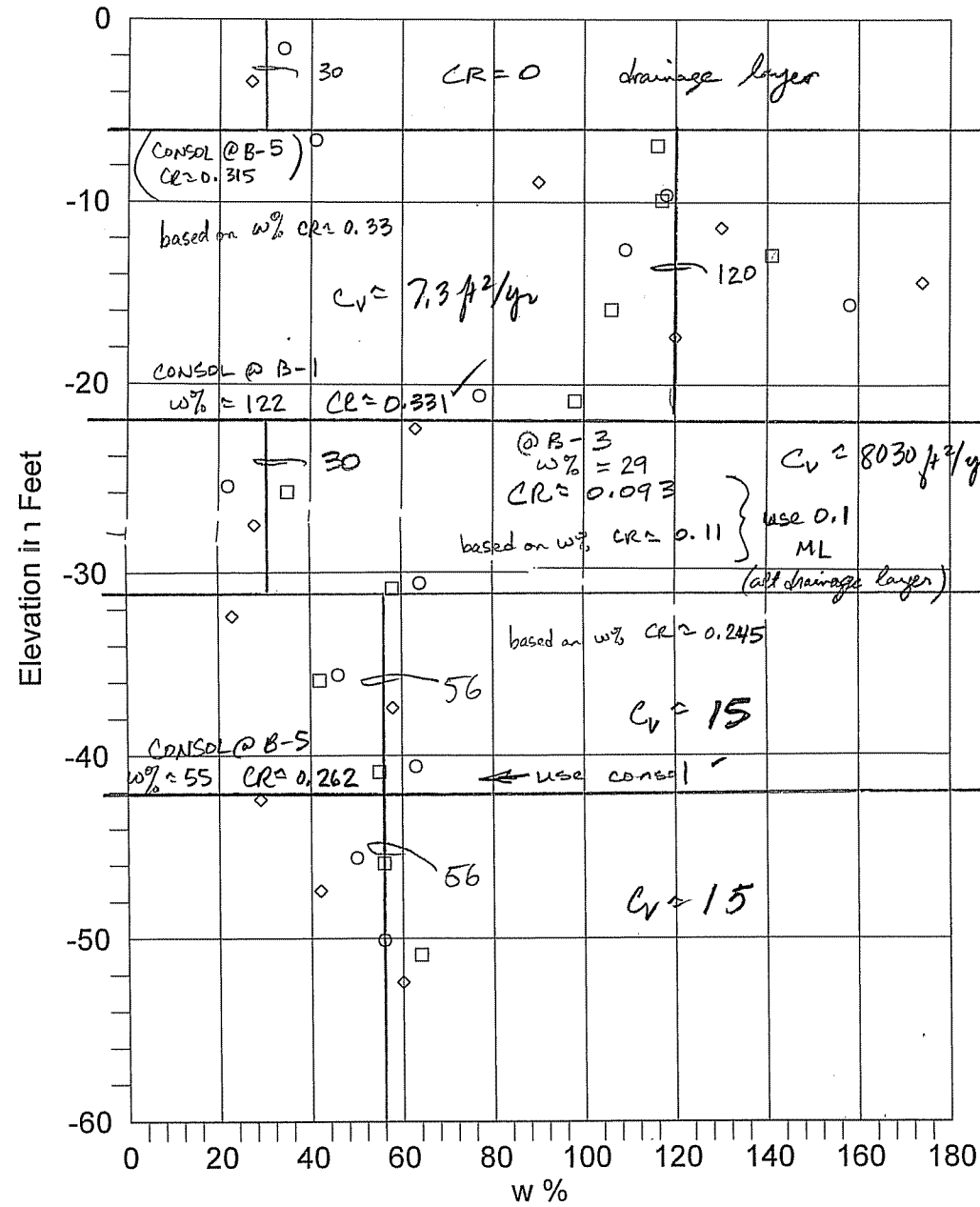
Project: 19292  
 Title: Scofield Island  
 Reach 1 - Thickest Barrier Beach @ Surface  
 Engineer: GPS  
 Borings: B-4  
 Date: 20-Feb-09  
 Time: 10:06 AM

Effective Vertical Stress at Mid-Stratum

Depth (ft)	H (ft)	z (ft)	t' (pcf)	CR	RR	Shear Strength (psf)	Influence Values					Past Maximum Burden (psf)	Present Burden (psf)	Induced Change (psf)	Applied as % Present	Settlement			10% (ft)	su/ps	OCR	Total Profile (in)	
							I-1	I-2	I-3	I-4	Iz					Recomp (ft)	Virgin (ft)	Total (ft)					
1	8.0 16.0	16.0	8.0	57.0	0.000	0.000	0.00	0.246	0.246	0.246	0.246	0.985	456.00	456.00 912.00	0.00	0.00	0.000	0.000	0.000	0.000	0.00	1.00	0.00
2	18.0 20.0	4.0	18.0	53.0	0.225	0.034	200.00	0.242	0.242	0.242	0.242	0.967	1018.00	1018.00 1124.00	0.00	0.00	0.000	0.000	0.000	0.000	0.20	1.00	0.00
3	22.5 25.0	5.0	22.5	38.0	0.260	0.039	245.00	0.240	0.240	0.240	0.240	0.959	1219.00	1219.00 1314.00	0.00	0.00	0.000	0.000	0.000	0.000	0.20	1.00	0.00
4	27.5 30.0	5.0	27.5	38.0	0.260	0.039	284.00	0.237	0.237	0.237	0.237	0.950	1409.00	1409.00 1604.00	0.00	0.00	0.000	0.000	0.000	0.000	0.20	1.00	0.00
5	33.0 36.0	6.0	33.0	38.0	0.260	0.039	323.00	0.235	0.235	0.235	0.235	0.940	1618.00	1618.00 1732.00	0.00	0.00	0.000	0.000	0.000	0.000	0.20	1.00	0.00
6	38.5 41.0	5.0	38.5	57.0	0.000	0.000	0.00	0.232	0.232	0.232	0.232	0.930	1874.50	1874.50 2017.00	0.00	0.00	0.000	0.000	0.000	0.000	0.00	1.00	0.00
7	43.5 46.0	5.0	43.5	57.0	0.000	0.000	0.00	0.230	0.230	0.230	0.230	0.921	2159.50	2159.50 2302.00	0.00	0.00	0.000	0.000	0.000	0.000	0.00	1.00	0.00
8	48.5 51.0	5.0	48.5	49.0	0.210	0.032	440.00	0.228	0.228	0.228	0.228	0.912	2424.50	2424.50 2547.00	0.00	0.00	0.000	0.000	0.000	0.000	0.18	1.00	0.00
9	53.0 55.0	4.0	53.0	43.0	0.000	0.000	0.00	0.226	0.226	0.226	0.226	0.904	2633.00	2633.00 2719.00	0.00	0.00	0.000	0.000	0.000	0.000	0.00	1.00	0.00
10	57.5 60.0	5.0	57.5	43.0	0.000	0.000	0.00	0.224	0.224	0.224	0.224	0.896	2826.50	2826.50 2934.00	0.00	0.00	0.000	0.000	0.000	0.000	0.00	1.00	0.00
11	62.5 65.0	5.0	62.5	47.0	0.000	0.000	0.00	0.222	0.222	0.222	0.222	0.887	3051.50	3051.50 3169.00	0.00	0.00	0.000	0.000	0.000	0.000	0.00	1.00	0.00
12	67.5 70.0	5.0	67.5	47.0	0.000	0.000	0.00	0.220	0.220	0.220	0.220	0.878	3286.50	3286.50 3404.00	0.00	0.00	0.000	0.000	0.000	0.000	0.00	1.00	0.00
13	72.5 75.0	5.0	72.5	47.0	0.000	0.000	0.00	0.217	0.217	0.217	0.217	0.869	3521.50	3521.50 3639.00	0.00	0.00	0.000	0.000	0.000	0.000	0.00	1.00	0.00
14	77.5 80.0	5.0	77.5	47.0	0.000	0.000	0.00	0.215	0.215	0.215	0.215	0.861	3756.50	3756.50 3874.00	0.00	0.00	0.000	0.000	0.000	0.000	0.00	1.00	0.00

TOTAL S (ft) = 0.000 0.000 0.000 0.000  
 (in) = 0.000 0.000 0.000 0.000  
 REDUCED S (ft) = 0.000 0.000 0.000 0.000  
 (in) = 0.000 0.000 0.000 0.000

# SETTLEMENT PARAMETERS



◇	◇	◇	B1-OB
○	○	○	B3-OB
□	□	□	B5-OB
◆	◆	◆	B1
●	●	●	B3
■	■	■	B5

REACH 2 (THIN BARRIER BEACH @ SURFACE)

**EUSTIS ENGINEERING COMPANY, INC.**  
 GEOTECHNICAL ENGINEERS  
 3011 28TH STREET METAIRIE, LOUISIANA

**SOIL DESIGN PARAMETERS**  
 B-1, B-3 AND B-5

**RIVERINE SAND MINING**  
 SCOFIELD ISLAND RESTORATION (BA-40)  
 PLAQUEMINES PARISH, LOUISIANA

DRAWN BY: J.T.H.	5 JAN 2009	FILE: COMBINEBORING DATA.GRF
CHECKED BY: G.P.S.	JOB NO.: 19292	

Recompression Virgin total 10% total  
 TOTAL S (ft) = 0.000 0.000 0.000 0.000  
 (in) = 0.000 0.000 0.000 0.000  
 REDUCED S (ft) = 0.000 0.000 0.000 0.000  
 (in) = 0.000 0.000 0.000 0.000

If you would like to compute the capacity by imposing the load 2/3 down the length of the pile place a 1 here

Initial Ld  
 s = 0.00  
 s' = 0.00  
 0.00

SETTLEMENT ANALYSIS - WESTERGAARD THEORY

Water Table Depth = 0.00  
 Bearing Area Width, Xb (ft) = 500.00 500.00  
 Bearing Area Length, Yb (ft) = 3000.00 3000.00  
 Abscissa for Analysis, Xa (ft) = 250.00 250.00  
 Ordinate for Analysis, Ya (ft) = 1500.00 1500.00  
 Net Applied Pressure (psf) = 0.00 0.00  
 Depth of Application, Da (ft) = 0.00 0.00  
 Depth of Foundation, Df (ft) = 0.00 0.00  
 Poisson's Ratio, u = 0.00 0.00  
 Recompression Factor = 0.15 0.15  
 Contact Pressure Multiplier = 1.00 1.00  
 Total Settlement Multiplier = 1.00 1.00  
 Applied Pressure Criteria = 0.00 0.00%

Region dX (ft) dY (ft)  
 1 250.0 1500.0  
 2 250.0 -1500.0  
 3 -250.0 1500.0  
 4 -250.0 -1500.0  
 Strain Factor = 0.50

Project: 19292  
 Title: Scofield Island  
 Reach 2 - Thin Barrier Beach @ Surface  
 Engineer: GPS  
 Borings: B-1, B-3 and B-5  
 Date: 20-Feb-09  
 Time: 9:10 AM

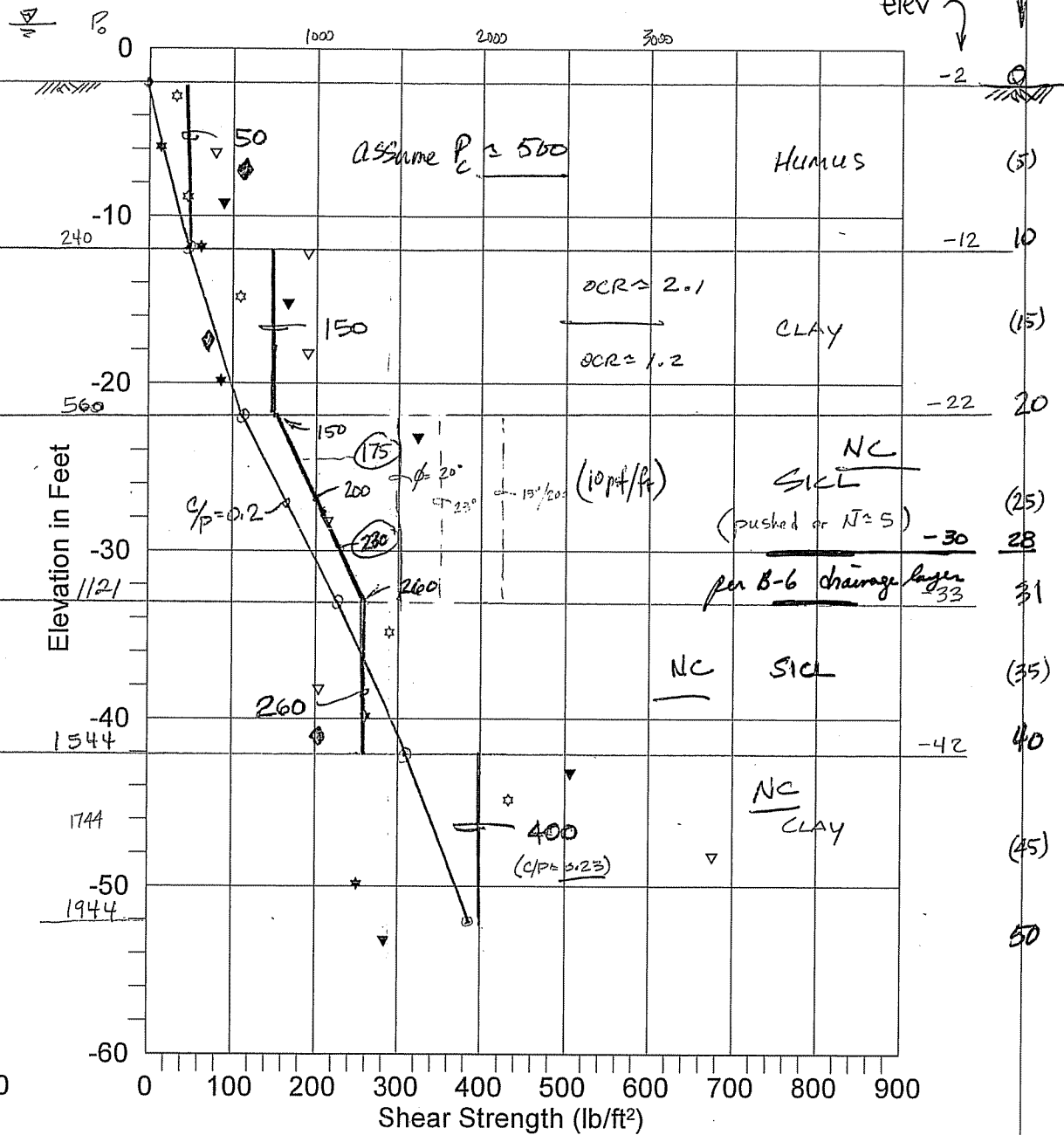
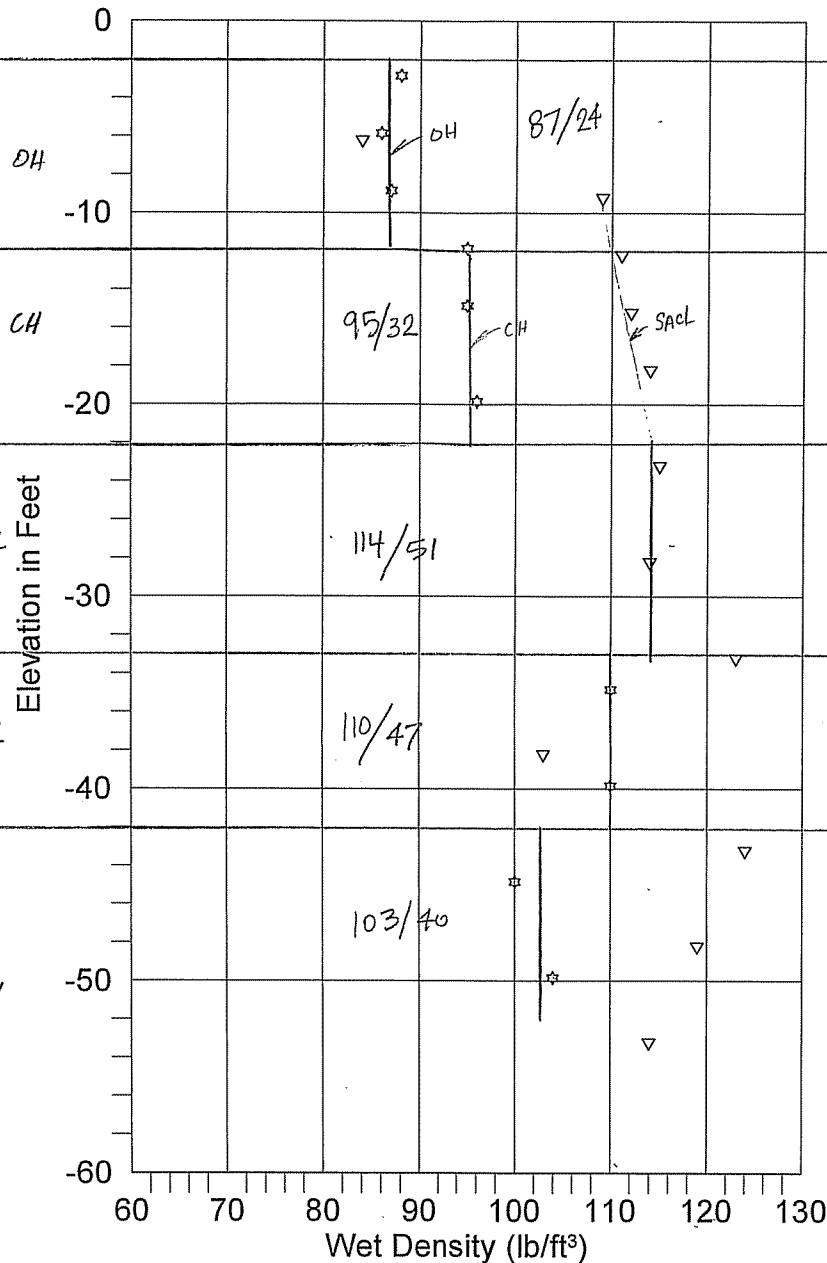
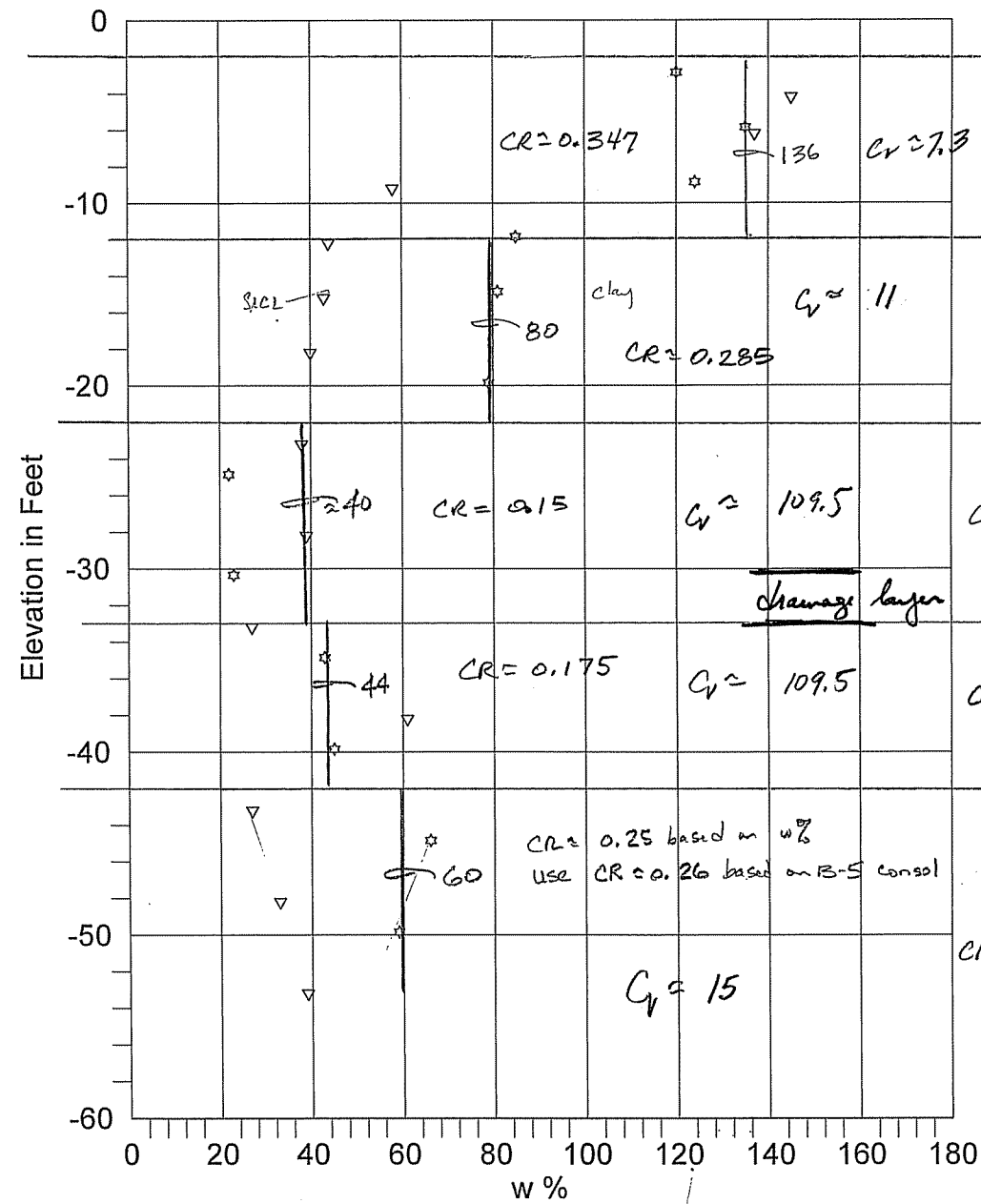
Effective Vertical Stress at Mid-Stratum

Depth (ft)	H (ft)	z (ft)	t' (pcf)	CR	RR	Shear Strength (psf)	Influence Values					Past Maximum (psf)	Present Burden (psf)	Induced Change (psf)	Applied as % Present	Settlement			10% (ft)	su/po	OCR	Total Profile (in)				
							I-1	I-2	I-3	I-4	Iz					Recomp (ft)	Virgin (ft)	Total (ft)								
0.0																										
1	2.0	4.0	2.0	57.0	0.000	0.000	0.00	0.249	0.249	0.249	0.249	0.996	114.00	114.00	0.00	0.00	0.000	0.000	0.000	0.000	0.00	0.00	1.00	0.00	0.00	
2	6.0	8.0	6.0	25.0	0.331	0.050	(115.00)	0.247	0.247	0.247	0.247	0.989	500.74	278.00	0.00	0.00	0.000	0.000	0.000	0.000	0.000	0.000	0.41	1.80	0.00	0.00
3	11.0	14.0	11.0	25.0	0.331	0.050	80.00	0.245	0.245	0.245	0.245	0.980	403.00	403.00	0.00	0.00	0.000	0.000	0.000	0.000	0.000	0.000	0.20	1.00	0.00	0.00
4	17.0	20.0	17.0	25.0	0.331	0.050	107.00	0.242	0.242	0.242	0.242	0.969	553.00	553.00	0.00	0.00	0.000	0.000	0.000	0.000	0.000	0.000	0.19	1.00	0.00	0.00
5	22.5	25.0	22.5	53.0	0.100	0.015	203.00	0.240	0.240	0.240	0.240	0.959	760.50	760.50	0.00	0.00	0.000	0.000	0.000	0.000	0.000	0.000	0.27	1.00	0.00	0.00
6	27.0	29.0	27.0	53.0	0.100	0.015	268.00	0.238	0.238	0.238	0.238	0.951	999.00	999.00	0.00	0.00	0.000	0.000	0.000	0.000	0.000	0.000	0.27	1.00	0.00	0.00
7	32.0	35.0	32.0	43.0	0.262	0.039	220.00	0.235	0.235	0.235	0.235	0.942	1234.00	1234.00	0.00	0.00	0.000	0.000	0.000	0.000	0.000	0.000	0.18	1.00	0.00	0.00
8	37.5	40.0	37.5	43.0	0.262	0.039	220.00	0.233	0.233	0.233	0.233	0.932	1470.50	1470.50	0.00	0.00	0.000	0.000	0.000	0.000	0.000	0.000	0.15	1.00	0.00	0.00
9	42.5	45.0	42.5	43.0	0.262	0.039	300.00	0.231	0.231	0.231	0.231	0.923	1685.50	1685.50	0.00	0.00	0.000	0.000	0.000	0.000	0.000	0.000	0.18	1.00	0.00	0.00
10	47.5	50.0	47.5	43.0	0.262	0.039	300.00	0.228	0.228	0.228	0.228	0.914	1900.50	1900.50	0.00	0.00	0.000	0.000	0.000	0.000	0.000	0.000	0.16	1.00	0.00	0.00
11	52.5	55.0	52.5	47.0	0.000	0.000	0.00	0.226	0.226	0.226	0.226	0.905	2125.50	2125.50	0.00	0.00	0.000	0.000	0.000	0.000	0.000	0.000	0.00	1.00	0.00	0.00
12	57.5	60.0	57.5	47.0	0.000	0.000	0.00	0.224	0.224	0.224	0.224	0.896	2360.50	2360.50	0.00	0.00	0.000	0.000	0.000	0.000	0.000	0.000	0.00	1.00	0.00	0.00
13	62.5	65.0	62.5	47.0	0.000	0.000	0.00	0.222	0.222	0.222	0.222	0.887	2595.50	2595.50	0.00	0.00	0.000	0.000	0.000	0.000	0.000	0.000	0.00	1.00	0.00	0.00
14	67.5	70.0	67.5	47.0	0.000	0.000	0.00	0.220	0.220	0.220	0.220	0.878	2830.50	2830.50	0.00	0.00	0.000	0.000	0.000	0.000	0.000	0.000	0.00	1.00	0.00	0.00

*drainage layer*

*limits of data*

TOTAL S (ft) = 0.000 0.000 0.000 0.000  
 (in) = 0.000 0.000 0.000 0.000  
 REDUCED S (ft) = 0.000 0.000 0.000 0.000  
 (in) = 0.000 0.000 0.000 0.000



- ☆ B2-OB
- ▽ B6-OB
- ★ B2
- ▼ B6

REACH 3 (MARSH SURFACE)

**EUSTIS ENGINEERING COMPANY, INC.**  
 GEOTECHNICAL ENGINEERS  
 3011 28TH STREET METAIRIE, LOUISIANA

**SOIL DESIGN PARAMETERS**  
 B-2 AND B-6

**RIVERINE SAND MINING**  
 SCOFIELD ISLAND RESTORATION (BA-40)  
 PLAQUEMINES PARISH, LOUISIANA

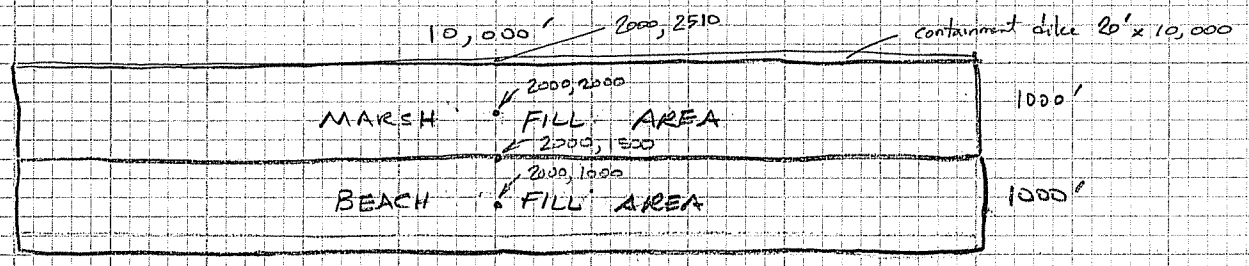
DRAWN BY: J.T.H.	5 JAN 2009	FILE: COMBINEBORING DATA.GRF
CHECKED BY: G.P.S.	JOB NO.: 19292	



Project Title: Seaford Island  
 Subject: Settlement models + time rate Job No: 19292  
 By: RPY Date: \_\_\_\_\_ Checked By: \_\_\_\_\_ Date: \_\_\_\_\_



Page: \_\_\_\_\_



ULTIMATE BULB

assumed NC to 80' w/ no drainage layers beneath boring

Reach	Mudline	Initial (psf)	depth 10% Bulb (ft)	value (in)	adj. pressure
1	0	268.5	51	5.32	~245 ~4.4"
	-1	305.5	60	6.92	~277 ~5.5"
	-2	342.5	65	8.19	~306 ~7"
2	0		60	16.32	~207 ~12"
	-1		65	19.65	~234 ~14"
	-2		75	22.27	~260 ~16"
3	0		60	12.98	1.5(37) + 1.5(100) ≈ 205.5 ⇒ 9.54"
	-1		70	15.54	2.75(37) + 1.25(100) ≈ 246.8 ⇒ 10.75"
	-2		80	17.92	4(37) + 1(100) ≈ 248 ⇒ 12"

↑ for fill to +3 (3 to 5' fill)

EST TIME RATE

IND % ULTIMATE (using mudline @ el-d)

r avg assumed for reaches 3+4

MONTHS	YRS.	R-1	R-2	R-3	R-2(d)	R-3(d)
1	0.08	14/18	~4	13/14	7	
3	0.25	22/28	~4	"	10.5/12	
6	0.5	29.5/38	~4	"	17.6	
	1	37/50	6/7	13/15	27	
	3	53/76	14.5/16	16/18	44/51	
	5	66/86	21/23	19/21	65	
	10	72/92	31/35	25/28	71/80	
	20	74/95	42/47	33/36	87	

↳ 6.5/7"

↳ w/ML not drainage layer

↳ w/ drainage layer

weight induced settlements only  
 does not include geologic subsidence  
 does not include self-weight consolidation

need to run alt case w/ thin drainage layer

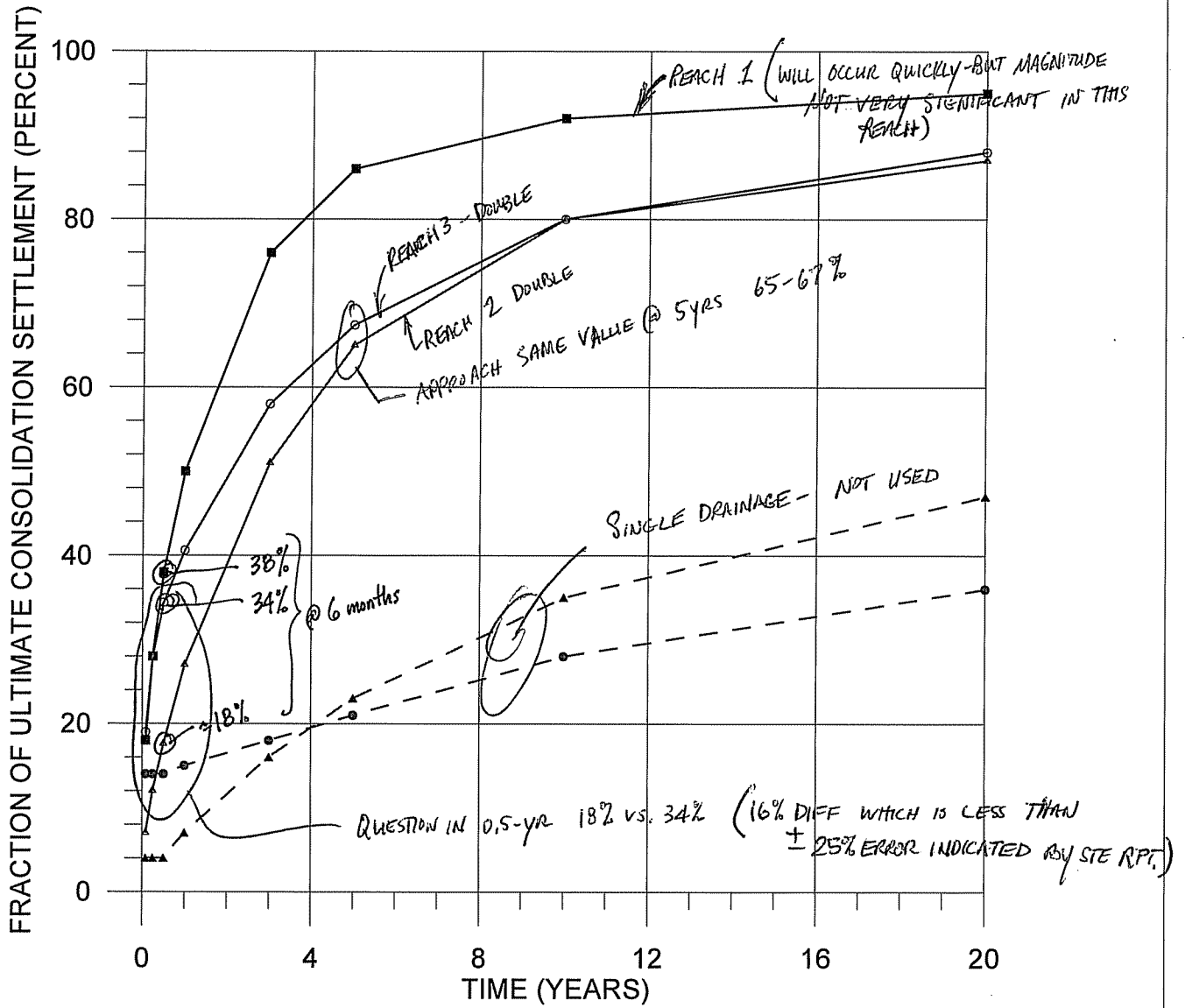
Reach 3 w/ added drainage layer: % bulb int / % bulb ult

time	Mud 0	Mud = 2
0.08	20.4	19
.25	30.4	28
.5	37.3	34.4
1	44.4	40.6
3	61.4	58
5	72	67.4
10	85	80
20	93	88

vs. 18% w/out drainage layer

↑ use these values

# ESTIMATES OF SETTLEMENT TIME RATE



■ — ■ — ■	Reach 1
▲ — ▲ — ▲	Reach 2 - Single Drainage
● — ● — ●	Reach 3 - Single Drainage
▲ — ▲ — ▲	Reach 2 - Double Drainage
○ — ○ — ○	Reach 3 - Double Drainage

~~NOTED IN~~

- WEIGHT INDUCED SETTLEMENT ONLY ASSUMING ~~MOISTURE~~ SOIL PROPERTIES BETWEEN 50 AND 80 FEET SETTLEMENT INDUCED BELOW 80 FEET NOT CONSIDERED
- GEOLOGIC SUBSIDENCE NOT INCLUDED
- SELF-WEIGHT CONSOLIDATION NOT INCLUDED
- VARIATION IN SUBMERGENCE NOT INCLUDED (ASSUMES WATER SURFACE AT MLW = 0.5)



**EUSTIS ENGINEERING SERVICES, L.L.C.**  
 GEOTECHNICAL ENGINEERS  
 3011 28TH STREET METAIRIE, LOUISIANA

## ESTIMATED TIME-RATE OF SETTLEMENT

RIVERINE SAND MINING  
 SCOFIELD ISLAND RETORATION (BA-40)  
 PLAQUEMINES PARISH, LOUISIANA

DRAWN BY: G.P.S.

MARCH 2009

FILE: SETL\_TIME\_RATE.GRF

CHECKED BY: W.W.G.

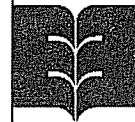
JOB NO.: 19292

FIGURE 14

Project Title: Scofield Island

Subject: time-rate of settlement (subsoils) Job No: 19292

By: GPS Date: 3/21/09 Checked By: \_\_\_\_\_ Date: \_\_\_\_\_



**EUSTIS**  
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Lafayette, Louisiana  
Gulfport, Mississippi

Page: \_\_\_\_\_

based on Reach 2 (double drainage)

% INDUCED

(Single)

TIME (yrs)	WATER MUDLINE MARSH	% INDUCED				
		0.5 -2 +3(only)	0.5 -2 +3 @ BEACH	0.5 0 +3(all)	+1 0 +3(all)	0.5 0 +2
0.5		18	18	20	20	23
1		27	27	31	31	34
3		51		59	59	64
5		65		76	74	79
10		80		92		94
20		87		97		98

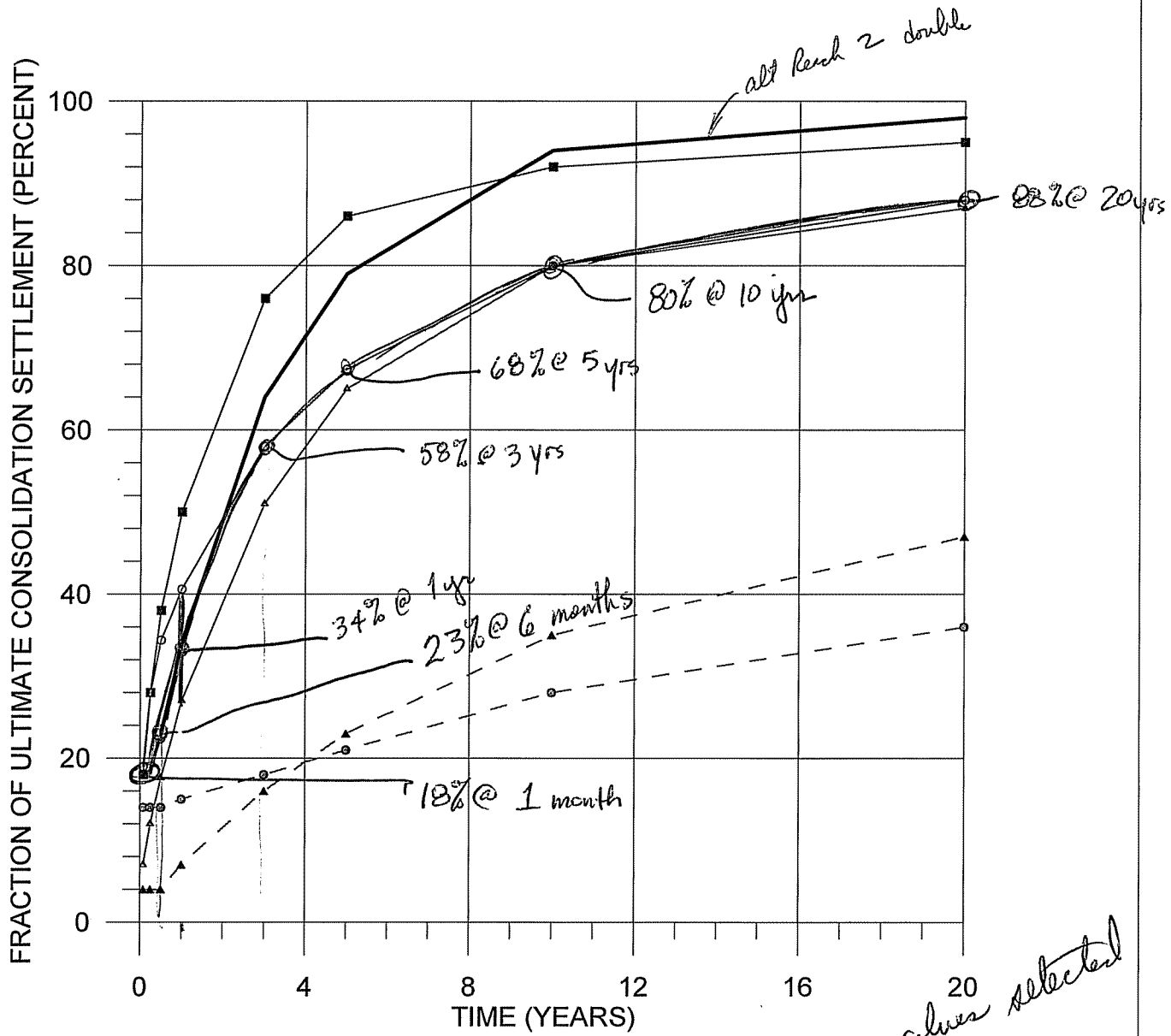
31

59

REACH 3  
34  
41  
58  
67  
80  
88



# ESTIMATES OF SETTLEMENT TIME RATE



■ — ■	Reach 1
▲ — ▲	Reach 2 - Single Drainage
● — ●	Reach 3 - Single Drainage
▲ — ▲	Reach 2 - Double Drainage
○ — ○	Reach 3 - Double Drainage
—	Line/Scatter Plot 8

*Composite values selected for report*



**EUSTIS ENGINEERING SERVICES, L.L.C.**  
 GEOTECHNICAL ENGINEERS

3011 28TH STREET METAIRIE, LOUISIANA

**ESTIMATED TIME-RATE OF SETTLEMENT**

RIVERINE SAND MINING  
 SCOFIELD ISLAND RETORATION (BA-40)  
 PLAQUEMINES PARISH, LOUISIANA

DRAWN BY: G.P.S.

16 MARCH 2009

FILE: SETL\_TIME\_RATE.GRF

CHECKED BY: W.W.G.

JOB NO.: 19292

FIGURE 5

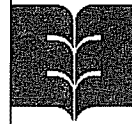
FIG 5

Sheet 1

Project Title: Scofield Island

Subject: Geologic Subsidence Job No: 19292

By: GPS Date: 3/20/09 Checked By: \_\_\_\_\_ Date: \_\_\_\_\_



**EUSTIS**  
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Lafayette, Louisiana  
Gulfport, Mississippi

Page: \_\_\_\_\_

*0.025 ft/yr*

TIME (yr)	GEOLOGIC SUBSIDENCE (Ft)	(in)
0.5	0.0125	0.15
1	0.025	0.3
3	0.075	0.9
5	0.125	1.5
10	0.25	3
20	0.5	6

Project Title: Scofield Island  
 Subject: "self weight settlement" Job No: 19292  
 By: GPS Date: 3/21/09 Checked By: \_\_\_\_\_ Date: \_\_\_\_\_



Page: \_\_\_\_\_

MARSH AREA FILL (DREDGED FILL)

PLATFORM ELEV	FILL HEIGHT			30% HT			50%		
	Mud line								
	0	-1	-2						
3	3	4	5	0.9	1.25	1.56	0.45 (5.6)	0.63 (7.5)	0.78 (9)
2.5	2.5	3.5	4.5	0.78	1.1	1.41	0.39 (4.6)	0.55 (6.5)	0.7 (8.5)
2	2	3	4	0.6	0.9	1.25	0.3 (3.6)	0.45 (5.6)	0.63 (7.5)

CONTAINMENT DIKE

→ ASSUME PLACED BY MECHANICAL METHODS  
 MUCH LESS SELF-WEIGHT CONSOLIDATION  
 POTENTIAL THAN PUMPED SEDIMENTS

assume half as much consol.  
 50% ← still occurs rapidly during const.

DIKE ELEV	FILL HT			.15 HT			50%		
	0	-1	-2	0	-1	-2	0	-1	-2
4	4	5	6	0.6	0.75	0.9	0.3	0.375	0.45
6	6	7	8	0.9	1.05	1.2	0.45	0.53	0.6

Project Title: Scotfield Island  
 Subject: "self weight" consolidation Job No: 19292  
 By: GPS Date: 3/21/09 Checked By: \_\_\_\_\_ Date: \_\_\_\_\_



Page: \_\_\_\_\_

based on STE's report text - appears they are combining "settling" characteristics for dredge & fill (results from settling column type test) w/ the consolidation of fill at low pressure (low pressure consolidation / self-weight consolidation)

check our evaluation in "Bayou Dupont" project where both tests were run  
 - we reported 50% settl as self weight & 50% settl as consolidation of subsoils in July 2006 rpt.  
 in Sept/Oct 2006 rpt. we est 60-70% of total is self-weight & the remainder is consol.

[from STE's table self wt. is 33 to 63% of total depending on water depth & freeboard]

for marsh fill to +3 & grade @ -2 = 5' fill

note that for Bayou Dupont; single drainage was considered for subsoils which is why small contribution to overall settl.

relationship developed as  $\Delta H = \frac{\Delta e}{1+e_0} H_{fill} = 0.313 H_{fill}$

[alt eq.  $\Delta H = H \cdot (C_c) \log \left( \frac{\sigma_{vo} + \Delta \sigma}{\sigma_{vo}'} \right)$  } selection of  $\sigma_{vo}$  &  $\sigma_{vo}'$  not practical for dredged fill]

$C_v \approx 0.015 \text{ ft}^2/\text{day} \approx 5.5 \text{ ft}^2/\text{yr}$

est. consolidation will be essentially complete w/in 5 years for 5' fill & much less for lesser fill heights.

(Paul used 0.313H for Whiskey Island based on Bayou Dupont test results)  
 received comments

H	$\Delta H$
3'	0.9'
4'	1.25'
5'	1.56'

Approximately 50% to 70% of this ultimate settlement would be anticipated during construction (thus - fill quantities may be increased to achieve "critical" platform heights & these movements may not be directly observed.)

reduce to 50% for settlement observed "after" construction  
 ie, > 5 months after fill placement

$0.45' = 5"$   
 $0.63' = 7\frac{1}{2}"$   
 $0.78' = 9"$

\* "SELF WEIGHT" - BASED ON WATER DEPTH & SETTLING CHARACTERISTICS  
 REACH 2 VALUES FOR MARSH

FOR SCOPEFIELD - ONLY EVALUATING AVE MIDDLELINE  
 @ EL 0 TO EL -2

BAY JOE WISE AREA: PROJECT BA-35  
 TABLE 3.2-6  
 POST-CONSTRUCTION AREA FILL SETTLEMENTS

HAVE SOME NATURAL/EXISTING "CHANNELS"



SO FOR:

WATER AT	GRADE AT	WATER DEPTH
0.5	-2	2.5
1.0		3
1.5		3.5

Water Depth (feet)	Time After Construction (years)	Post Construction Settlement (feet)															
		1 Foot Freeboard				2 Feet Freeboard				3 Feet Freeboard				4 Feet Freeboard			
		Subgrade	Fill	Geol.	Total	Subgrade	Fill	Geol.	Total	Subgrade	Fill	Geol.	Total	Subgrade	Fill	Geol.	Total
0	0.5	0.11	0.08	0.01	0.20	0.20	0.14	0.01	0.35	0.26	0.21	0.01	0.48	0.31	0.27	0.01	0.59
	1.0	0.14	0.08	0.02	0.24	0.23	0.15	0.02	0.40	0.31	0.22	0.02	0.55	0.37	0.28	0.02	0.67
	2.0	0.15	0.08	0.05	0.28	0.27	0.16	0.05	0.48	0.37	0.23	0.05	0.65	0.46	0.30	0.05	0.81
	33% 5.0	0.16	0.08	0.12	0.36	0.29	0.16	0.12	0.57	0.41	0.24	0.12	0.77	0.52	0.31	0.12	0.95
	10.0	0.17	0.08	0.25	0.50	0.31	0.16	0.25	0.72	0.46	0.24	0.25	0.95	0.57	0.32	0.25	1.14
	20.0	0.18	0.08	0.50	0.76	0.33	0.16	0.50	0.99	0.47	0.24	0.50	1.21	0.69	0.32	0.50	1.51
2	0.5	0.11	0.21	0.01	0.33	0.19	0.26	0.01	0.46	0.25	0.32	0.01	0.58	0.30	0.35	0.01	0.66
	1.0	0.14	0.21	0.02	0.37	0.23	0.27	0.02	0.52	0.31	0.33	0.02	0.66	0.37	0.38	0.02	0.77
	2.0	0.15	0.21	0.05	0.41	0.26	0.28	0.05	0.59	0.37	0.34	0.05	0.76	0.47	0.40	0.05	0.92
	55% 5.0	0.17	0.21	0.12	0.50	0.30	0.28	0.12	0.70	0.43	0.35	0.12	0.90	0.54	0.41	0.12	1.07
	10.0	0.19	0.21	0.25	0.65	0.33	0.28	0.25	0.86	0.48	0.35	0.25	1.08	0.61	0.42	0.25	1.28
	20.0	0.20	0.21	0.50	0.91	0.35	0.28	0.50	1.13	0.51	0.35	0.50	1.36	0.64	0.42	0.50	1.56
4	0.5	0.13	0.32	0.01	0.46	0.17	0.37	0.01	0.55	0.25	0.41	0.01	0.67	0.33	0.44	0.01	0.78
	1.0	0.15	0.32	0.02	0.49	0.23	0.38	0.02	0.63	0.30	0.42	0.02	0.74	0.36	0.47	0.02	0.85
	2.0	0.16	0.32	0.05	0.53	0.25	0.39	0.05	0.69	0.35	0.44	0.05	0.84	0.45	0.49	0.05	0.99
	63% 5.0	0.19	0.32	0.12	0.63	0.31	0.39	0.12	0.82	0.44	0.45	0.12	1.01	0.58	0.51	0.12	1.21
	10.0	0.20	0.32	0.25	0.77	0.33	0.39	0.25	0.97	0.47	0.45	0.25	1.17	0.62	0.52	0.25	1.39
	20.0	0.22	0.32	0.50	1.04	0.36	0.39	0.50	1.25	0.51	0.45	0.50	1.46	0.66	0.52	0.50	1.68
8	0.5	0.12	0.42	0.01	0.55	0.18	0.45	0.01	0.64	0.24	0.49	0.01	0.74	0.27	0.51	0.01	0.79
	1.0	0.15	0.42	0.02	0.59	0.22	0.47	0.02	0.71	0.31	0.51	0.02	0.84	0.42	0.55	0.02	0.99
	2.0	0.20	0.42	0.05	0.67	0.31	0.48	0.05	0.84	0.41	0.53	0.05	0.99	0.51	0.57	0.05	1.13
	5.0	0.22	0.42	0.12	0.76	0.35	0.48	0.12	0.95	0.48	0.54	0.12	1.14	0.60	0.59	0.12	1.31
	10.0	0.24	0.42	0.25	0.91	0.38	0.48	0.25	1.11	0.51	0.54	0.25	1.30	0.66	0.60	0.25	1.51
	20.0	0.26	0.42	0.50	1.18	0.41	0.48	0.50	1.39	0.55	0.54	0.50	1.59	0.69	0.60	0.50	1.73
8	0.5	0.13	0.50	0.01	0.64	0.16	0.51	0.01	0.68	0.25	0.55	0.01	0.81	0.35	0.56	0.01	0.92
	1.0	0.18	0.50	0.02	0.70	0.22	0.53	0.02	0.77	0.30	0.57	0.02	0.89	0.41	0.60	0.02	1.03
	2.0	0.20	0.50	0.05	0.75	0.29	0.55	0.05	0.89	0.39	0.59	0.05	1.03	0.52	0.63	0.05	1.20
	5.0	0.25	0.50	0.12	0.87	0.35	0.55	0.12	1.02	0.47	0.60	0.12	1.19	0.62	0.65	0.12	1.39
	10.0	0.29	0.50	0.25	1.04	0.42	0.55	0.25	1.22	0.54	0.60	0.25	1.39	0.67	0.66	0.25	1.58
	20.0	0.31	0.50	0.50	1.31	0.45	0.55	0.50	1.50	0.58	0.60	0.50	1.68	0.72	0.66	0.50	1.88

~1" per foot of freeboard

Not typical

@ 5 yrs  

$$\left[ \frac{\text{fill}}{\text{Subgrade} + \text{fill}} \right] = \%$$

rate of consolidation

$H \text{ (ft)} = 2.5$   
 $c_v \text{ (ft}^2\text{/day)} = 0.015$

U%	constant pore pressure Tv	t (days)	t (years)	t (months)
5	0.0017	0.71	0.00	0.0
10	0.0077	3.21	0.01	0.1
15	0.0177	7.38	0.02	0.2
20	0.0314	13.08	0.04	0.4
25	0.0491	20.46	0.06	0.7
30	0.0707	29.46	0.08	1.0
35	0.0962	40.08	0.11	1.3
40	0.126	52.50	0.14	1.7
45	0.159	66.25	0.18	2.2
50	0.196	81.67	0.22	2.7
55	0.238	99.17	0.27	3.3
60	0.286	119.17	0.33	3.9
65	0.342	142.50	0.39	4.7
70	0.403	167.92	0.46	5.5
75	0.477	198.75	0.54	6.5
80	0.567	236.25	0.65	7.8
85	0.684	285.00	0.78	9.4
90	0.848	353.33	0.97	11.6
95	1.129	470.42	1.29	15.5

rate of consolidation

H (ft) =  
 cv (ft<sup>2</sup>/day) =

5  
 0.015

U%	constant pore pressure Tv	t (days)	t (years)	t (months)
5	0.0017	2.83	0.01	0.1
10	0.0077	12.83	0.04	0.4
15	0.0177	29.50	0.08	1.0
20	0.0314	52.33	0.14	1.7
25	0.0491	81.83	0.22	2.7
30	0.0707	117.83	0.32	3.9
35	0.0962	160.33	0.44	5.3
40	0.126	210.00	0.58	6.9
45	0.159	265.00	0.73	8.7
50	0.196	326.67	0.89	10.7
55	0.238	396.67	1.09	13.0
60	0.286	476.67	1.31	15.7
65	0.342	570.00	1.56	18.7
70	0.403	671.67	1.84	22.1
75	0.477	795.00	2.18	26.1
80	0.567	945.00	2.59	31.1
85	0.684	1140.00	3.12	37.5
90	0.848	1413.33	3.87	46.5
95	1.129	1881.67	5.16	61.9



Project Title: SCOFIELD ISLAND  
 Subject: SOIL REACH 2 Job No: 19292  
 By: GPS Date: 3/20/09 Checked By: \_\_\_\_\_ Date: \_\_\_\_\_



Page: \_\_\_\_\_

PLATFORM ELEV.	WATER ELEV.	MUDLINE ELEV.	INDUCED LOADS			
			beach	marsh	dike +4	+6
+ 3	0.5	0	515	<del>207</del> 234	329	503
		-1	560	234	362	536
		-2	604	260	395	569
+ 3	1.0	0	491	184.3	302	475
		-1	535	211	334	508
		-2	579	238	367	540.5
+ 3	1.5	0	467	162	273.5	450.5
		-1	510.5	189	307	480
		-2	555	215.5	339	513
+ 2.5	0.5	0		172		
		-1		197		
		-2		225		
	1.0	0		150		
		-1		174		
		-2		201		
	1.5	0		124		
		-1		151		
		-2		178		
+ 2	0.5	0		134.4		
		-1		162		
		-2		189		
	1.0	0		112		
		-1		139		
		-2		166		
	1.5	0		86		
		-1		110		
		-2		143		

12  
0.5



CONSOLIDATION SETTLEMENT OF MARSH AREA FILL

	PLATFORM ELEVATION	3	3	3	3	3	3	3	3	3	2.5	2.5	2.5	2.5	2.5	2.5	2.5	2	2	2	2	2	2	2	2	2		
	MUDLINE ELEVATION	0	-1	-2	0	-1	-2	0	-1	-2	0	-1	-2	0	-1	-2	0	-1	-2	0	-1	-2	0	-1	-2	0	-1	-2
	WATER SURFACE ELEVATION	0.5	0.5	0.5	1	1	1	1.5	1.5	1.5	0.5	0.5	0.5	1	1	1	1.5	1.5	1.5	0.5	0.5	0.5	1	1	1	1.5	1.5	1.5
	ULTIMATE SETTLEMENT (FT)	1	1.16	1.34	0.85	1.03	1.18	0.71	0.87	1.05	0.8	0.96	1.12	0.66	0.81	0.97	0.51	0.67	0.82	0.55	0.71	0.87	0.41	0.57	0.72	0.32	0.42	0.58
		Magnitude of settlement in feet over time																										
TIME (YRS)	% ULT																											
0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
0.5	0.23	0.23	0.27	0.31	0.20	0.24	0.27	0.16	0.20	0.24	0.18	0.22	0.26	0.15	0.19	0.22	0.12	0.15	0.19	0.13	0.16	0.20	0.09	0.13	0.17	0.07	0.10	0.13
1	0.34	0.34	0.39	0.46	0.29	0.35	0.40	0.24	0.30	0.36	0.27	0.33	0.38	0.22	0.28	0.33	0.17	0.23	0.28	0.19	0.24	0.30	0.14	0.19	0.24	0.11	0.14	0.20
3	0.58	0.58	0.67	0.78	0.49	0.60	0.68	0.41	0.50	0.61	0.46	0.56	0.65	0.38	0.47	0.56	0.30	0.39	0.48	0.32	0.41	0.50	0.24	0.33	0.42	0.19	0.24	0.34
5	0.68	0.68	0.79	0.91	0.58	0.70	0.80	0.48	0.59	0.71	0.54	0.65	0.76	0.45	0.55	0.66	0.35	0.46	0.56	0.37	0.48	0.59	0.28	0.39	0.49	0.22	0.29	0.39
10	0.8	0.80	0.93	1.07	0.68	0.82	0.94	0.57	0.70	0.84	0.64	0.77	0.90	0.53	0.65	0.78	0.41	0.54	0.66	0.44	0.57	0.70	0.33	0.46	0.58	0.26	0.34	0.46
20	0.88	0.88	1.02	1.18	0.75	0.91	1.04	0.62	0.77	0.92	0.70	0.84	0.99	0.58	0.71	0.85	0.45	0.59	0.72	0.48	0.62	0.77	0.36	0.50	0.63	0.28	0.37	0.51
ULT	1	1.00	1.16	1.34	0.85	1.03	1.18	0.71	0.87	1.05	0.80	0.96	1.12	0.66	0.81	0.97	0.51	0.67	0.82	0.55	0.71	0.87	0.41	0.57	0.72	0.32	0.42	0.58

Elevation of fill over time

		3	3	3	3	3	3	3	3	3	2.5	2.5	2.5	2.5	2.5	2.5	2.5	2	2	2	2	2	2	2	2	2	2	
0																												
0.5		2.77	2.73	2.69	2.80	2.76	2.73	2.84	2.80	2.76	2.32	2.28	2.24	2.35	2.31	2.28	2.38	2.35	2.31	1.87	1.84	1.80	1.91	1.87	1.83	1.93	1.90	1.87
1		2.66	2.61	2.54	2.71	2.65	2.60	2.76	2.70	2.64	2.23	2.17	2.12	2.28	2.22	2.17	2.33	2.27	2.22	1.81	1.76	1.70	1.86	1.81	1.76	1.89	1.86	1.80
3		2.42	2.33	2.22	2.51	2.40	2.32	2.59	2.50	2.39	2.04	1.94	1.85	2.12	2.03	1.94	2.20	2.11	2.02	1.68	1.59	1.50	1.76	1.67	1.58	1.81	1.76	1.66
5		2.32	2.21	2.09	2.42	2.30	2.20	2.52	2.41	2.29	1.96	1.85	1.74	2.05	1.95	1.84	2.15	2.04	1.94	1.63	1.52	1.41	1.72	1.61	1.51	1.78	1.71	1.61
10		2.20	2.07	1.93	2.32	2.18	2.06	2.43	2.30	2.16	1.86	1.73	1.60	1.97	1.85	1.72	2.09	1.96	1.84	1.56	1.43	1.30	1.67	1.54	1.42	1.74	1.66	1.54
20		2.12	1.98	1.82	2.25	2.09	1.96	2.38	2.23	2.08	1.80	1.66	1.51	1.92	1.79	1.65	2.05	1.91	1.78	1.52	1.38	1.23	1.64	1.50	1.37	1.72	1.63	1.49
ULT		2.00	1.84	1.66	2.15	1.97	1.82	2.29	2.13	1.95	1.70	1.54	1.38	1.84	1.69	1.53	1.99	1.83	1.68	1.45	1.29	1.13	1.59	1.43	1.28	1.68	1.58	1.42

CONSOLIDATION SETTLEMENT OF MARSH AREA FILL ADJUSTED BY ESTIMATED SELF WEIGHT CONSOLIDATION

	INITIAL	PLATFORM ELEVATION	3	3	3	3	3	3	3	3	3	3	3	2.5	2.5	2.5	2.5	2.5	2.5	2.5	2	2	2	2	2	2	2					
		PLATFORM ELEVATION AFTER SELF WT	2.55	2.37	2.22	2.55	2.37	2.22	2.55	2.37	2.22	2.55	2.37	2.22	2.11	1.95	1.8	2.11	1.95	1.8	2.11	1.95	1.8	1.7	1.55	1.37	1.7	1.55	1.37	1.7	1.55	1.37
		MUDLINE ELEVATION	0	-1	-2	0	-1	-2	0	-1	-2	0	-1	-2	0	-1	-2	0	-1	-2	0	-1	-2	0	-1	-2	0	-1	-2	0	-1	-2
		WATER SURFACE ELEVATION	0.5	0.5	0.5	1	1	1	1.5	1.5	1.5	0.5	0.5	0.5	1	1	1	1.5	1.5	1.5	0.5	0.5	0.5	1	1	1	1.5	1.5	1.5	0.5	0.5	0.5
		ULTIMATE SETTLEMENT (FT)	1	1.16	1.34	0.85	1.03	1.18	0.71	0.87	1.05	0.8	0.96	1.12	0.66	0.81	0.97	0.51	0.67	0.82	0.55	0.71	0.87	0.41	0.57	0.72	0.32	0.42	0.58	0.32	0.42	0.58
		Magnitude of settlement in feet over time (consolidation settlement)																														
TIME (YRS)	% ULT																															
0	0		0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
0.5	0.23		0.23	0.27	0.31	0.20	0.24	0.27	0.16	0.20	0.24	0.18	0.22	0.26	0.15	0.19	0.22	0.12	0.15	0.19	0.13	0.16	0.20	0.09	0.13	0.17	0.07	0.10	0.13	0.07	0.10	0.13
1	0.34		0.34	0.39	0.46	0.29	0.35	0.40	0.24	0.30	0.36	0.27	0.33	0.38	0.22	0.28	0.33	0.17	0.23	0.28	0.19	0.24	0.30	0.14	0.19	0.24	0.11	0.14	0.20	0.11	0.14	0.20
3	0.58		0.58	0.67	0.78	0.49	0.60	0.68	0.41	0.50	0.61	0.46	0.56	0.65	0.38	0.47	0.56	0.30	0.39	0.48	0.32	0.41	0.50	0.24	0.33	0.42	0.19	0.24	0.34	0.19	0.24	0.34
5	0.68		0.68	0.79	0.91	0.58	0.70	0.80	0.48	0.59	0.71	0.54	0.65	0.76	0.45	0.55	0.66	0.35	0.46	0.56	0.37	0.48	0.59	0.28	0.39	0.49	0.22	0.29	0.39	0.22	0.29	0.39
10	0.8		0.80	0.93	1.07	0.68	0.82	0.94	0.57	0.70	0.84	0.64	0.77	0.90	0.53	0.65	0.78	0.41	0.54	0.66	0.44	0.57	0.70	0.33	0.46	0.58	0.26	0.34	0.46	0.26	0.34	0.46
20	0.88		0.88	1.02	1.18	0.75	0.91	1.04	0.62	0.77	0.92	0.70	0.84	0.99	0.58	0.71	0.85	0.45	0.59	0.72	0.48	0.62	0.77	0.36	0.50	0.63	0.28	0.37	0.51	0.28	0.37	0.51
ULT	1		1.00	1.16	1.34	0.85	1.03	1.18	0.71	0.87	1.05	0.80	0.96	1.12	0.66	0.81	0.97	0.51	0.67	0.82	0.55	0.71	0.87	0.41	0.57	0.72	0.32	0.42	0.58	0.32	0.42	0.58

Elevation of fill over time considering self weight consolidation

TIME (YRS)																																		
0		2.55	2.37	2.22	2.55	2.37	2.22	2.55	2.37	2.22	2.11	1.95	1.80	2.11	1.95	1.80	2.11	1.95	1.80	1.70	1.55	1.37	1.70	1.55	1.37	1.70	1.55	1.37	1.70	1.55	1.37	1.70	1.55	1.37
0.5		2.32	2.10	1.91	2.35	2.13	1.95	2.39	2.17	1.98	1.93	1.73	1.54	1.96	1.76	1.58	1.99	1.80	1.61	1.57	1.39	1.17	1.61	1.42	1.20	1.63	1.45	1.24	1.63	1.45	1.24	1.63	1.45	1.24
1		2.21	1.98	1.76	2.26	2.02	1.82	2.31	2.07	1.86	1.84	1.62	1.42	1.89	1.67	1.47	1.94	1.72	1.52	1.51	1.31	1.07	1.56	1.36	1.13	1.59	1.41	1.17	1.59	1.41	1.17	1.59	1.41	1.17
3		1.97	1.70	1.44	2.06	1.77	1.54	2.14	1.87	1.61	1.65	1.39	1.15	1.73	1.48	1.24	1.81	1.56	1.32	1.38	1.14	0.87	1.46	1.22	0.95	1.51	1.31	1.03	1.51	1.31	1.03	1.51	1.31	1.03
5		1.87	1.58	1.31	1.97	1.67	1.42	2.07	1.78	1.51	1.57	1.30	1.04	1.66	1.40	1.14	1.76	1.49	1.24	1.33	1.07	0.78	1.42	1.16	0.88	1.48	1.26	0.98	1.48	1.26	0.98	1.48	1.26	0.98
10		1.75	1.44	1.15	1.87	1.55	1.28	1.98	1.67	1.38	1.47	1.18	0.90	1.58	1.30	1.02	1.70	1.41	1.14	1.26	0.98	0.67	1.37	1.09	0.79	1.44	1.21	0.91	1.44	1.21	0.91	1.44	1.21	0.91
20		1.67	1.35	1.04	1.80	1.46	1.18	1.93	1.60	1.30	1.41	1.11	0.81	1.53	1.24	0.95	1.66	1.36	1.08	1.22	0.93	0.60	1.34	1.05	0.74	1.42	1.18	0.86	1.42	1.18	0.86	1.42	1.18	0.86
ULT		1.55	1.21	0.88	1.70	1.34	1.04	1.84	1.50	1.17	1.31	0.99	0.68	1.45	1.14	0.83	1.60	1.28	0.98	1.15	0.84	0.50	1.29	0.98	0.65	1.38	1.13	0.79	1.38	1.13	0.79	1.38	1.13	0.79

CONSOLIDATION SETTLEMENT OF MARSH AREA FILL ADJUSTED BY ESTIMATED SELF WEIGHT CONSOLIDATION AND GEOLOGIC SUBSIDENCE

	<b>INITIAL PLATFORM ELEVATION</b>	3	3	3	3	3	3	3	3	3	2.5	2.5	2.5	2.5	2.5	2.5	2.5	2.5	2.5	2	2	2	2	2	2	2	2	2
	<b>PLATFORM ELEVATION AFTER SELF WT</b>	2.55	2.37	2.22	2.55	2.37	2.22	2.55	2.37	2.22	2.11	1.95	1.8	2.11	1.95	1.8	2.11	1.95	1.8	1.7	1.55	1.37	1.7	1.55	1.37	1.7	1.55	1.37
	<b>MUDLINE ELEVATION</b>	0	-1	-2	0	-1	-2	0	-1	-2	0	-1	-2	0	-1	-2	0	-1	-2	0	-1	-2	0	-1	-2	0	-1	-2
	<b>WATER SURFACE ELEVATION</b>	0.5	0.5	0.5	1	1	1	1.5	1.5	1.5	0.5	0.5	0.5	1	1	1	1.5	1.5	1.5	0.5	0.5	0.5	1	1	1	1.5	1.5	1.5
	<b>ULTIMATE SETTLEMENT (FT)</b>	1	1.16	1.34	0.85	1.03	1.18	0.71	0.87	1.05	0.8	0.96	1.12	0.66	0.81	0.97	0.51	0.67	0.82	0.55	0.71	0.87	0.41	0.57	0.72	0.32	0.42	0.58
	Magnitude of settlement in feet over time (consolidation settlement)																											
TIME (YRS)	% ULT																											
0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
0.5	0.23	0.23	0.27	0.31	0.20	0.24	0.27	0.16	0.20	0.24	0.18	0.22	0.26	0.15	0.19	0.22	0.12	0.15	0.19	0.13	0.16	0.20	0.09	0.13	0.17	0.07	0.10	0.13
1	0.34	0.34	0.39	0.46	0.29	0.35	0.40	0.24	0.30	0.36	0.27	0.33	0.38	0.22	0.28	0.33	0.17	0.23	0.28	0.19	0.24	0.30	0.14	0.19	0.24	0.11	0.14	0.20
3	0.58	0.58	0.67	0.78	0.49	0.60	0.68	0.41	0.50	0.61	0.46	0.56	0.65	0.38	0.47	0.56	0.30	0.39	0.48	0.32	0.41	0.50	0.24	0.33	0.42	0.19	0.24	0.34
5	0.68	0.68	0.79	0.91	0.58	0.70	0.80	0.48	0.59	0.71	0.54	0.65	0.76	0.45	0.55	0.66	0.35	0.46	0.56	0.37	0.48	0.59	0.28	0.39	0.49	0.22	0.29	0.39
10	0.8	0.80	0.93	1.07	0.68	0.82	0.94	0.57	0.70	0.84	0.64	0.77	0.90	0.53	0.65	0.78	0.41	0.54	0.66	0.44	0.57	0.70	0.33	0.46	0.58	0.26	0.34	0.46
20	0.88	0.88	1.02	1.18	0.75	0.91	1.04	0.62	0.77	0.92	0.70	0.84	0.99	0.58	0.71	0.85	0.45	0.59	0.72	0.48	0.62	0.77	0.36	0.50	0.63	0.28	0.37	0.51
ULT	1	1.00	1.16	1.34	0.85	1.03	1.18	0.71	0.87	1.05	0.80	0.96	1.12	0.66	0.81	0.97	0.51	0.67	0.82	0.55	0.71	0.87	0.41	0.57	0.72	0.32	0.42	0.58

Elevation of fill over time considering self weight consolidation and geologic subsidence

TIME (YRS)	sub (ft)																											
0	0	2.55	2.37	2.22	2.55	2.37	2.22	2.55	2.37	2.22	2.11	1.95	1.80	2.11	1.95	1.80	2.11	1.95	1.80	1.70	1.55	1.37	1.70	1.55	1.37	1.70	1.55	1.37
0.5	0.0125	2.31	2.09	1.90	2.34	2.12	1.94	2.37	2.16	1.97	1.91	1.72	1.53	1.95	1.75	1.56	1.98	1.78	1.60	1.56	1.37	1.16	1.59	1.41	1.19	1.61	1.44	1.22
1	0.025	2.19	1.95	1.74	2.24	1.99	1.79	2.28	2.05	1.84	1.81	1.60	1.39	1.86	1.65	1.45	1.91	1.70	1.50	1.49	1.28	1.05	1.54	1.33	1.10	1.57	1.38	1.15
3	0.075	1.90	1.62	1.37	1.98	1.70	1.46	2.06	1.79	1.54	1.57	1.32	1.08	1.65	1.41	1.16	1.74	1.49	1.25	1.31	1.06	0.79	1.39	1.14	0.88	1.44	1.23	0.96
5	0.125	1.75	1.46	1.18	1.85	1.54	1.29	1.94	1.65	1.38	1.44	1.17	0.91	1.54	1.27	1.02	1.64	1.37	1.12	1.20	0.94	0.65	1.30	1.04	0.76	1.36	1.14	0.85
10	0.25	1.50	1.19	0.90	1.62	1.30	1.03	1.73	1.42	1.13	1.22	0.93	0.65	1.33	1.05	0.77	1.45	1.16	0.89	1.01	0.73	0.42	1.12	0.84	0.54	1.19	0.96	0.66
20	0.5	1.17	0.85	0.54	1.30	0.96	0.68	1.43	1.10	0.80	0.91	0.61	0.31	1.03	0.74	0.45	1.16	0.86	0.58	0.72	0.43	0.10	0.84	0.55	0.24	0.92	0.68	0.36
ULT																												



SETTLEMENT ANALYSIS - STRESS DISTRIBUTION BY WESTERGAARD THEORY, MAGNITUDE AND RATE OF CONSOLIDATION BY TERZAGHI THEORY

EE 19292 marsh to beach to el 6 dike to el 4  
Reach 2 - Thin Barrier Beach at surface el 3 average

Table with 11 columns (Region 1-10) and 13 rows (Bearing Area Center, Bearing Area Length, Net Applied Pressure, Depth of Application, Depth of Foundation, Contact Pressure Multiplier, Time of Application).

Water at le 0.5  
1.16 2.34 100.00 233.86  
2.66 37.00 98.47 332.33

Table with 11 columns (Region 1-10) and 4 rows (Sub 1-4) for dx and dy values.

Strain Factor = 0.50

Table with 2 columns and 10 rows (Abscissa for Analysis, Ordinate for Analysis, Poisson's Ratio, Recompression Factor, Total Settlement Multiplier, Stress Bulb Criterion, Current Time, Depth of Wick Drains, Wick Drain Triangular Grid Spacing, Equivalent Drain Diameter, Equivalent Mandrel Diameter, Diameter of Smear Zone, Drain Capacity Ratio).

Table with 4 columns (Recomp Sett, Virgin Sett, Total Sett, Bulb Sett) and 4 rows (TOTAL, REDUCD, %Ultimate).

Table with 11 columns (Region 1-10) and 3 rows (I, II, III) for Ty and Tv values.

Main table with columns: Str No, Depth (ft), q', CR (N/D), RR (N/D), cv (s/ft/yr), ch (s/ft/yr), Total H (ft), Past Maximum (psf), Zero Burden (psf), Ultimate (Change, Value), Induced (Change, Value), Recomp Sett, Virgin Sett, Total Sett, Bulb Sett, Influence Factors (Region 1-10, lz), Rate Parameters (Effective H\*, Position z/L), Percent Consolidation (Region 1-10).

Summary table with 4 columns (Recomp Sett, Virgin Sett, Total Sett, Bulb Sett) and 4 rows (TOTAL, REDUCD, %Ultimate).





SETTLEMENT ANALYSIS - STRESS DISTRIBUTION BY WESTERGAARD THEORY, MAGNITUDE AND RATE OF CONSOLIDATION BY TERZAGHI THEORY

Table with columns: Load Region, Region 1-10, and rows for parameters like Bearing Area Center, Depth of Application, etc.

water at 1  
0.85 4.15 120.00 488.22  
1.85 57.00 105.35 603.58

Table with columns: Region 1-10 and rows for Sub 1-4, dX, dY.

Strain Factor = 0.50

Table with columns: Parameter and Value, including Abscissa for Analysis, Ordinate for Analysis, etc.

Table with columns: Ultimate Settlement, Induced Settlement, and rows for TOTAL, REDUCD.

Table with columns: Group, L (ft), and rows for I, II, III.

Main settlement analysis table with columns: Str No., Depth, q', CR, RR, cv, ch, Total H, Past Maximum, Zero Burden, Change, Value, Ultimate Settlement, Induced Settlement, Influence Factors, Rate Parameters, Percent Consolidation.

Summary table with columns: Parameter, Value, and rows for TOTAL, REDUCD.

SETTLEMENT ANALYSIS - STRESS DISTRIBUTION BY WESTERGAARD THEORY, MAGNITUDE AND RATE OF CONSOLIDATION BY TERZAGHI THEORY

EE 19292 marsh to beach to el 6 dike to el 4  
Reach 2 - Thin Barrier Beach at surface

Table with 11 columns for Region 1-10 and rows for Bearing Area Center (Xc, Yc), Bearing Area Width (B), Bearing Area Length (L), Net Applied Pressure (psf), Depth of Application (Da), Depth of Foundation (Df), Contact Pressure Multiplier (ND), and Time of Application (yrs).

water at 1  
1.03 2.97 120.00 356.51  
3.03 57.00 172.66 529.17

Table with 11 columns for Region 1-10 and 4 rows for Sub 1-4, each with dX and dY values.

Strain Factor = 0.50

Abscissa for Analysis, Xa (ft) = 2000.00  
Ordinate for Analysis, Ya (ft) = 2000.00  
Poisson's Ratio, u (N/D) = 0.00  
Recompression Factor (ND) = 0.15  
Total Settlement Multiplier (ND) = 0.85  
Stress Bulb Criterion (%) = 10.00%  
Current Time (yrs) = 0.00  
Depth of Wick Drains (ft) = 0.00  
Wick Drain Triangular Grid Spacing (ft) = 4.00  
Equivalent Drain Diameter, dw (in) = 2.060  
Equivalent Mandrel Diameter, dm (in) = 3.670  
Diameter of Smear Zone, ds (in) = 9.850  
Drain Capacity Ratio (scg) = 4309.0

Table with 4 columns for Ultimate Settlement (Recomp Sett, Virgin Sett, Total Sett, Bulb Sett) and 4 columns for Induced Settlement (Recomp Sett, Virgin Sett, Total Sett, Bulb Sett), plus a TOTAL row.

Table with 11 columns for Region 1-10 and 3 rows for Group I, II, III with L (ft) and Tv values.

Main settlement analysis table with columns for Str No., Depth (ft), q' (pcf), CR (N/D), RR (N/D), cv (sq/yr), ch (sq/yr), Total H (ft), Past Maximum (psf), Zero Burden (psf), Ultimate Settlement, Induced Settlement, Influence Factors, Rate Parameters, and Percent Consolidation.

Summary table with 4 columns for Ultimate Settlement and 4 columns for Induced Settlement, plus a REDUCD row.





SETTLEMENT ANALYSIS - STRESS DISTRIBUTION BY WESTERGAARD THEORY, MAGNITUDE AND RATE OF CONSOLIDATION BY TERZAGHI THEORY

EE 19292 marsh to beach to el 6 dike to el 4

Table with 10 columns (Region 1-10) and 13 rows (Load Region Parameter, Bearing Area Center, etc.)

water at 1.5

Table with 10 columns (Region 1-10) and 4 rows (Sub 1-4) for stress distribution.

Strain Factor = 0.50

Table with 2 columns and 13 rows for analysis parameters like Abscissa for Analysis, Ordinate for Analysis, etc.

Summary table with 4 columns (Ultimate Settlement, Induced Settlement) and 3 rows (TOTAL, REDUCD, %Ultimate).

Table with 10 columns (Region 1-10) and 3 rows (I, II, III) for rate parameters.

Main settlement analysis table with columns for Str No., Depth, q', CR, RR, cv, ch, Total H, Past Maximum, Zero Burden, Induced, Ultimate Settlement, Induced Settlement, Influence Factors, Rate Parameters, and Percent Consolidation.

Summary table with 4 columns and 3 rows (TOTAL, REDUCD, %Ultimate).







SETTLEMENT ANALYSIS - STRESS DISTRIBUTION BY WESTERGAARD THEORY, MAGNITUDE AND RATE OF CONSOLIDATION BY TERZAGHI THEORY

Table with 12 columns for Regions 1-10 and rows for various parameters like Bearing Area Center, Depth of Application, etc.

water at 0.5

Table with 12 columns for Regions 1-10 and rows for Sub 1-4 with dx and dy values.

Strain Factor = 0.50

Table of analysis parameters including Abscissa for Analysis, Ordinate for Analysis, etc.

Table showing Ultimate Settlement and Induced Settlement data for Total and REDUCD cases.

Table with 12 columns for Regions 1-10 and rows for Group I, II, III with L and Tv values.

Main table with columns for Str No., Depth, g', CR, RR, cv, ch, Total H, Past Maximum, Zero Burden, Change, Value, etc.

Summary table for TOTAL and REDUCD cases showing settlement values and percentages.

SETTLEMENT ANALYSIS - STRESS DISTRIBUTION BY WESTERGAARD THEORY, MAGNITUDE AND RATE OF CONSOLIDATION BY TERZAGHI THEORY

EE 19282

marsh to beach to el 6 dike to el 4

Reach 2 - Thin Barrier Beach at surface

Table with columns: Load Region, Region 1-10, and rows: Bearing Area Center, Xc (ft), Bearing Area Center, Yc (ft), Bearing Area Width in X-direction, B (ft), Bearing Area Length in Y-direction, L (ft), Net Applied Pressure (psf), Depth of Application, Da (ft), Depth of Foundation, Df (ft), Contact Pressure Multiplier (N/D), Time of Application (yrs).

water at 0.5

Table with columns: Region 1-10 and rows: Sub 1-4, dX, dY.

Strain Factor = 0.50

Table with columns: Parameter and Value, rows: Abscissa for Analysis, Xa (ft), Ordinate for Analysis, Ya (ft), Poisson's Ratio, u (N/D), Recompression Factor (N/D), Total Settlement Multiplier (N/D), Stress Bulb Criterion (%), Current Time (yrs), Depth of Wick Drains (ft), Wick Drain Triangular Grid Spacing (ft), Equivalent Drain Diameter, dw (in), Equivalent Mandrel Diameter, dm (in), Diameter of Smear Zone, ds (in), Drain Capacity Ratio (sqft).

Table with columns: Ultimate Settlement, Induced Settlement, and rows: TOTAL, REDUCD.

Table with columns: Group, L (ft), Tv, and rows: I, II, III.

Main data table with columns: Str No., Depth (ft), g' (pcf), CR (N/D), RR (N/D), cv (s/yr), ch (s/ft/yr), Total H (ft), Past Maximum (psf), Zero Burden (psf), Ultimate Settlement (Change, Value), Induced Settlement (Change, Value), Influence Factors (Region 1-10), Rate Parameters (Effective H\*, Position z'/L), Percent Consolidation for Combined Drainage (Region 1-10).

Summary table with columns: Parameter and Value, rows: TOTAL, REDUCD.













SETTLEMENT ANALYSIS - STRESS DISTRIBUTION BY WESTERGAARD THEORY, MAGNITUDE AND RATE OF CONSOLIDATION BY TERZAGHI THEORY

Table with 11 columns (Region 1-10) and 10 rows (Load Region Parameter, Bearing Area Center, etc.).

Table with 2 columns (Parameter, Value) listing analysis parameters like Abscissa for Analysis, Ordinate for Analysis, etc.

Table with 8 columns (Ultimate/Induced Settlement, Recom/Virgin, Total/Bulb) and 3 rows (TOTAL, REDUCD, %Ultimate).

Table with 10 columns (Region 1-10) and 4 rows (Sub 1-4) showing stress distribution values.

Strain Factor = 0.50

Table with 10 columns (Region 1-10) and 3 rows (I, II, III) showing consolidation parameters.

Main settlement analysis table with columns for Str No., Depth, Soil Properties, Settlements, and Influence Factors.

Summary table with 8 columns (Settlement types) and 3 rows (TOTAL, REDUCD, %Ultimate).





SETTLEMENT ANALYSIS - STRESS DISTRIBUTION BY WESTERGAARD THEORY, MAGNITUDE AND RATE OF CONSOLIDATION BY TERZAGHI THEORY  
EE 19292

Table with columns for Region 1 through 10 and rows for various parameters: Load Region, Bearing Area Center, Bearing Area Width, Bearing Area Length, Net Applied Pressure, Depth of Application, Depth of Foundation, Contact Pressure Multiplier, Time of Application.

Table with parameters and values: Abscissa for Analysis, Ordinate for Analysis, Poisson's Ratio, Recompression Factor, Total Settlement Multiplier, Stress Bulb Criterion, Current Time, Depth of Wick Drains, Wick Drain Triangular Grid Spacing, Equivalent Drain Diameter, Equivalent Mandrel Diameter, Diameter of Smear Zone, Drain Capacity Ratio.

Table with columns: Ultimate Settlement (Recomp Sett, Virgin Sett, Total Sett, Bulb Sett) and Induced Settlement (Recomp Sett, Virgin Sett, Total Sett, Bulb Sett). Rows include TOTAL and REDUCD with values in inches and percentages.

Table with columns for Region 1 through 10 and rows for Sub 1, Sub 2, Sub 3, Sub 4 with parameters dX, dY and values for each region.

Strain Factor = 0.50

Table with columns: Group, L (ft), Region 1 through 10 and rows for I, II, III with values for Tv and Uv.

Large table with columns: Str No., Depth (ft), g' (pcf), CR (N/D), RR (N/D), cv (s/tyr), ch (s/tyr), Total H (ft), Past Maximum (psf), Zero Burden (psf), Change (psf), Value (psf), Induced (psf), Ultimate Settlement (Recomp, Virgin, Total, Bulb), Induced Settlement (Recomp, Virgin, Total, Bulb), Influence Factors (Region 1-10), Rate Parameters (Effective H\*, Position z/L), Percent Consolidation for Combined Drainage (Region 1-10).

Summary table with columns: (in), (ft), %Ultimate and rows for TOTAL and REDUCD with values for settlement and percentage.



SETTLEMENT ANALYSIS - STRESS DISTRIBUTION BY WESTERGAARD THEORY, MAGNITUDE AND RATE OF CONSOLIDATION BY TERZAGHI THEORY

Table with columns for Region 1-10 and rows for parameters like Bearing Area Center, Net Applied Pressure, etc.

water at 0.5

Table with columns for Region 1-10 and rows for Sub 1-4 with dx/dy values.

Strain Factor = 0.50

Table with parameters like Abscissa for Analysis, Ordinate for Analysis, Poisson's Ratio, etc.

Table with columns for Ultimate Settlement and Induced Settlement, and rows for TOTAL and REDUCD.

4.73

Table with columns for Region 1-10 and rows for Group I, II, III with L (ft) and settlement values.

Main data table with columns for Str No., Depth, q', CR, RR, cv, ch, Total H, Past Maximum, Zero Burden, Ultimate Settlement, Induced Settlement, Influence Factors, Rate Parameters, and Percent Consolidation.

Summary table with columns for parameters and rows for TOTAL and REDUCD.



SETTLEMENT ANALYSIS - STRESS DISTRIBUTION BY WESTERGAARD THEORY, MAGNITUDE AND RATE OF CONSOLIDATION BY TERZAGHI THEORY

EE 19292 marsh to beach to el 6 dike to el 4  
Reach 2 - Thin Barrier Beach at surface el 2 average

Table with 11 columns (Region 1-10) and 11 rows (Load Region Parameter, Bearing Area Center, etc.)

water at 1

Table with 10 columns (Region 1-10) and 4 rows (Sub 1-4) for stress distribution parameters.

Strain Factor = 0.50

Table with 2 columns (Parameter, Value) for analysis parameters like Abscissa for Analysis, Ordinate for Analysis, etc.

Table with 8 columns (Ultimate Settlement, Induced Settlement) and 3 rows (TOTAL, REDUCD, %Ultimate).

Table with 11 columns (Region 1-10) and 3 rows (I, II, III) for consolidation parameters.

Main data table with columns for Str No, Depth, q', CR, RR, cv, ch, Total H, Past Maximum, Zero Burden, Change, Value, Induced, Ultimate Settlement, Induced Settlement, Influence Factors, Rate Parameters, and Percent Consolidation.

Summary table with 8 columns (Settlement, Change, Value, Induced) and 3 rows (TOTAL, REDUCD, %Ultimate).



SETTLEMENT ANALYSIS - STRESS DISTRIBUTION BY WESTERGAARD THEORY, MAGNITUDE AND RATE OF CONSOLIDATION BY TERZAGHI THEORY

EE 19282

marsh to beach to el 6 dike to el 4  
el 2 average

Table with columns: Load Region Parameter, Region 1-10. Rows include Bearing Area Center, Bearing Area Width, Bearing Area Length, Net Applied Pressure, Depth of Application, etc.

water at 1

Table with columns: Region 1-10, dx, dy. Rows include Sub 1, Sub 2, Sub 3, Sub 4.

Strain Factor = 0.50

Table with columns: Parameter, Value. Rows include Abscissa for Analysis, Ordinate for Analysis, Poisson's Ratio, etc.

Table with columns: Ultimate Settlement, Induced Settlement. Rows include TOTAL, REDUCED.

Table with columns: Group, L (ft), Region 1-10. Rows include I, II, III.

Main data table with columns: Str No., Depth (ft), g', CR (N/D), RR (N/D), cv (ft/yr), ch (ft/yr), Total H (ft), Past Maximum (psf), Zero Burden (psf), Ultimate Settlement, Induced Settlement, Influence Factors, Rate Parameters, Percent Consolidation.

Summary table with columns: Parameter, Value. Rows include TOTAL, REDUCED.

SETTLEMENT ANALYSIS - STRESS DISTRIBUTION BY WESTERGAARD THEORY, MAGNITUDE AND RATE OF CONSOLIDATION BY TERZAGHI THEORY

Table with columns: Load Region, Region 1-10, and rows: Bearing Area Center, Net Applied Pressure, Depth of Application, etc.

Table with columns: Parameter, Value and rows: Abscissa for Analysis, Ordinate for Analysis, Poisson's Ratio, etc.

Table with columns: Ultimate Settlement (Recomp Sett, Virgin Sett, Total Sett, Bulb Sett), Induced Settlement (Recomp Sett, Virgin Sett, Total Sett, Bulb Sett) and rows: TOTAL, REDUCD.

water at 1.5

Table with columns: Region 1-10, dX, dY and rows: Sub 1, Sub 2, Sub 3, Sub 4.

Strain Factor = 0.50

Table with columns: Group, L (ft), Tv, Uv, and rows: I, II, III.

Main settlement analysis table with columns: Str No, Depth, g', CR, RR, cv, ch, Total H, Past Maximum, Zero Burden, Ultimate Settlement, Induced Settlement, Influence Factors, Rate Parameters, Percent Consolidation.

Summary table with columns: Parameter, Value and rows: TOTAL, REDUCD.



SETTLEMENT ANALYSIS - STRESS DISTRIBUTION BY WESTERGAARD THEORY, MAGNITUDE AND RATE OF CONSOLIDATION BY TERZAGHI THEORY

Table with 11 columns: Region 1-10. Rows include: Load Region Parameter, Bearing Area Center, Xc (ft), Bearing Area Center, Yc (ft), Bearing Area Width in X-direction, B (ft), Bearing Area Length in Y-direction, L (ft), Net Applied Pressure (psf), Depth of Application, Da (ft), Depth of Foundation, Df (ft), Contact Pressure Multiplier (N/D), Time of Application (yrs).

Table with 2 columns: Parameter, Value. Rows include: Abscissa for Analysis, Xa (ft), Ordinate for Analysis, Ya (ft), Poisson's Ratio, u (N/D), Recompression Factor (N/D), Total Settlement Multiplier (N/D), Stress Bulb Criterion (%), Current Time (yrs), Depth of Wick Drains (ft), Wick Drain Triangular Grid Spacing (ft), Equivalent Drain Diameter, dw (in), Equivalent Mandrel Diameter, dm (in), Diameter of Smear Zone, ds (in), Drain Capacity Ratio (eqt).

Table with 8 columns: Recom Sett, Virgin Sett, Total Sett, Bulb Sett, Recom Sett, Virgin Sett, Total Sett, Bulb Sett. Rows include: Ultimate Settlement, Induced Settlement, TOTAL (in), TOTAL (ft), REDUCD (in), REDUCD (ft).

water at 1.5

Table with 10 columns: Region 1-10. Rows include: Sub 1, Sub 2, Sub 3, Sub 4. Columns include: dx, dy, values for each region.

Strain Factor = 0.50

Table with 16 columns: Region 1-10. Rows include: Group, L (ft), Tv, Uv, values for each region.

Main data table with columns: Str No., Depth (ft), g' (pcf), CR (N/D), RR (N/D), cv (in/yr), ch (in/yr), Total H (ft), Past Maximum (psf), Zero Burden (psf), Ultimate Settlement (psf), Induced Settlement (psf), Influence Factors (Ultimate Induced Stresses in Parentheses), Rate Parameters, Percent Consolidation for Combined Drainage (Current Induced Stresses in Parentheses).

Summary table with 8 columns: Recom Sett, Virgin Sett, Total Sett, Bulb Sett, Recom Sett, Virgin Sett, Total Sett, Bulb Sett. Rows include: TOTAL (in), TOTAL (ft), REDUCD (in), REDUCD (ft).

SETTLEMENT ANALYSIS - STRESS DISTRIBUTION BY WESTERGAARD THEORY, MAGNITUDE AND RATE OF CONSOLIDATION BY TERZAGHI THEORY

Table with columns: Load Region Parameter, Region 1, Region 2, Region 3, Region 4, Region 5, Region 6, Region 7, Region 8, Region 9, Region 10. Rows include parameters like Bearing Area Center, Bearing Area Width, etc.

Table with 2 columns: Parameter, Value. Includes parameters like Abscissa for Analysis, Ordinate for Analysis, Poisson's Ratio, etc.

Table with 4 columns: Ultimate Settlement, Induced Settlement, and two summary columns. Rows: TOTAL, REDUCD.

Table with columns: Region 1-10 and rows for Sub 1, Sub 2, Sub 3, Sub 4. Parameters include dX, dY, dK, dY.

Strain Factor = 0.50

Table with columns: Group, L (ft), Region 1-10. Rows: I, II, III.

Main data table with columns: Str No., Depth, q', CR, RR, cv, ch, Total H, Past Maximum, Zero Burden, Ultimate (Change, Value), Induced (Change, Value), Recompt Sett, Virgin Sett, Total Sett, Bulb Sett, Influence Factors (Region 1-10), Rate Parameters, Percent Consolidation for Combined Drainage (Region 1-10).

Summary table with columns: Parameter, Value. Includes TOTAL, REDUCD rows.

CONSOLIDATION SETTLEMENT OF DIKE CONTAINMENT - ASSUMING NO INFLUENCE FROM AREA FILL

	<b>DIKE CREST ELEVATION</b>	4	4	4	4	4	4	4	4	4	6	6	6	6	6	6	6	6	6
	<b>MUDLINE ELEVATION</b>	0	-1	-2	0	-1	-2	0	-1	-2	0	-1	-2	0	-1	-2	0	-1	-2
	<b>WATER SURFACE ELEVATION</b>	0.5	0.5	0.5	1	1	1	1.5	1.5	1.5	0.5	0.5	0.5	1	1	1	1.5	1.5	1.5
	<b>ULTIMATE SETTLEMENT (FT)</b>	0.62	0.69	0.76	0.56	0.63	0.7	0.51	0.57	0.64	1.04	1.11	1.17	0.84	1.05	1.12	0.87	1	1.06
			✓																
		Magnitude of settlement in feet over time																	
TIME (YRS)	% ULT																		
0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
0.5	0.23	0.14	0.16	0.17	0.13	0.14	0.16	0.12	0.13	0.15	0.24	0.26	0.27	0.19	0.24	0.26	0.20	0.23	0.24
1	0.34	0.21	0.23	0.26	0.19	0.21	0.24	0.17	0.19	0.22	0.35	0.38	0.40	0.29	0.36	0.38	0.30	0.34	0.36
3	0.58	0.36	0.40	0.44	0.32	0.37	0.41	0.30	0.33	0.37	0.60	0.64	0.68	0.49	0.61	0.65	0.50	0.58	0.61
5	0.68	0.42	0.47	0.52	0.38	0.43	0.48	0.35	0.39	0.44	0.71	0.75	0.80	0.57	0.71	0.76	0.59	0.68	0.72
10	0.8	0.50	0.55	0.61	0.45	0.50	0.56	0.41	0.46	0.51	0.83	0.89	0.94	0.67	0.84	0.90	0.70	0.80	0.85
20	0.88	0.55	0.61	0.67	0.49	0.55	0.62	0.45	0.50	0.56	0.92	0.98	1.03	0.74	0.92	0.99	0.77	0.88	0.93
ULT	1	0.62	0.69	0.76	0.56	0.63	0.70	0.51	0.57	0.64	1.04	1.11	1.17	0.84	1.05	1.12	0.87	1.00	1.06

Elevation of fill over time

		4	4	4	4	4	4	4	4	4	6	6	6	6	6	6	6	6	6
0		4	4	4	4	4	4	4	4	4	6	6	6	6	6	6	6	6	6
0.5		3.86	3.84	3.83	3.87	3.86	3.84	3.88	3.87	3.85	5.76	5.74	5.73	5.81	5.76	5.74	5.80	5.77	5.76
1		3.79	3.77	3.74	3.81	3.79	3.76	3.83	3.81	3.78	5.65	5.62	5.60	5.71	5.64	5.62	5.70	5.66	5.64
3		3.64	3.60	3.56	3.68	3.63	3.59	3.70	3.67	3.63	5.40	5.36	5.32	5.51	5.39	5.35	5.50	5.42	5.39
5		3.58	3.53	3.48	3.62	3.57	3.52	3.65	3.61	3.56	5.29	5.25	5.20	5.43	5.29	5.24	5.41	5.32	5.28
10		3.50	3.45	3.39	3.55	3.50	3.44	3.59	3.54	3.49	5.17	5.11	5.06	5.33	5.16	5.10	5.30	5.20	5.15
20		3.45	3.39	3.33	3.51	3.45	3.38	3.55	3.50	3.44	5.08	5.02	4.97	5.26	5.08	5.01	5.23	5.12	5.07
ULT		3.38	3.31	3.24	3.44	3.37	3.30	3.49	3.43	3.36	4.96	4.89	4.83	5.16	4.95	4.88	5.13	5.00	4.94





CONSOLIDATION SETTLEMENT OF DIKE CONTAINMENT - CONSIDERING ADJACENT MARSH PLATFORM AND BEACH FILL (PLATFORM AT 3)

	<b>DIKE CREST ELEVATION</b>	4	4	4	4	4	4	4	4	4	6	6	6	6	6	6	6	6	6
	<b>MUDLINE ELEVATION</b>	0	-1	-2	0	-1	-2	0	-1	-2	0	-1	-2	0	-1	-2	0	-1	-2
	<b>WATER SURFACE ELEVATION</b>	0.5	0.5	0.5	1	1	1	1.5	1.5	1.5	0.5	0.5	0.5	1	1	1	1.5	1.5	1.5
	<b>ULTIMATE SETTLEMENT (FT)</b>	0.92	1.03	1.18	0.78	0.94	1.1	0.7	0.8	0.96	1.35	1.5	1.62	1.27	1.37	1.52	1.14	1.29	1.38
		✓	✓								✓	✓							
		Magnitude of settlement in feet over time																	
<b>TIME (YRS)</b>	<b>% ULT</b>																		
0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
0.5	0.23	0.21	0.24	0.27	0.18	0.22	0.25	0.16	0.18	0.22	0.31	0.35	0.37	0.29	0.32	0.35	0.26	0.30	0.32
1	0.34	0.31	0.35	0.40	0.27	0.32	0.37	0.24	0.27	0.33	0.46	0.51	0.55	0.43	0.47	0.52	0.39	0.44	0.47
3	0.58	0.53	0.60	0.68	0.45	0.55	0.64	0.41	0.46	0.56	0.78	0.87	0.94	0.74	0.79	0.88	0.66	0.75	0.80
5	0.68	0.63	0.70	0.80	0.53	0.64	0.75	0.48	0.54	0.65	0.92	1.02	1.10	0.86	0.93	1.03	0.78	0.88	0.94
10	0.8	0.74	0.82	0.94	0.62	0.75	0.88	0.56	0.64	0.77	1.08	1.20	1.30	1.02	1.10	1.22	0.91	1.03	1.10
20	0.88	0.81	0.91	1.04	0.69	0.83	0.97	0.62	0.70	0.84	1.19	1.32	1.43	1.12	1.21	1.34	1.00	1.14	1.21
ULT	1	0.92	1.03	1.18	0.78	0.94	1.10	0.70	0.80	0.96	1.35	1.50	1.62	1.27	1.37	1.52	1.14	1.29	1.38

Elevation of fill over time

		4	4	4	4	4	4	4	4	4	6	6	6	6	6	6	6	6	6
0		4	4	4	4	4	4	4	4	4	6	6	6	6	6	6	6	6	6
0.5		3.79	3.76	3.73	3.82	3.78	3.75	3.84	3.82	3.78	5.69	5.66	5.63	5.71	5.68	5.65	5.74	5.70	5.68
1		3.69	3.65	3.60	3.73	3.68	3.63	3.76	3.73	3.67	5.54	5.49	5.45	5.57	5.53	5.48	5.61	5.56	5.53
3		3.47	3.40	3.32	3.55	3.45	3.36	3.59	3.54	3.44	5.22	5.13	5.06	5.26	5.21	5.12	5.34	5.25	5.20
5		3.37	3.30	3.20	3.47	3.36	3.25	3.52	3.46	3.35	5.08	4.98	4.90	5.14	5.07	4.97	5.22	5.12	5.06
10		3.26	3.18	3.06	3.38	3.25	3.12	3.44	3.36	3.23	4.92	4.80	4.70	4.98	4.90	4.78	5.09	4.97	4.90
20		3.19	3.09	2.96	3.31	3.17	3.03	3.38	3.30	3.16	4.81	4.68	4.57	4.88	4.79	4.66	5.00	4.86	4.79
ULT		3.08	2.97	2.82	3.22	3.06	2.90	3.30	3.20	3.04	4.65	4.50	4.38	4.73	4.63	4.48	4.86	4.71	4.62

SETTLEMENT ANALYSIS - STRESS DISTRIBUTION BY WESTERGAARD THEORY, MAGNITUDE AND RATE OF CONSOLIDATION BY TERZAGHI THEORY  
EE 19292

Table with columns: Load Region Parameter, Region 1-10, and values for Bearing Area Center (Xc, Yc), Bearing Area Width (B), Bearing Area Length (L), Net Applied Pressure, Depth of Application (Da), Depth of Foundation (Df), Contact Pressure Multiplier, and Time of Application.

Table with columns: dX, dY, Region 1-10, and values for Sub 1, Sub 2, Sub 3, and Sub 4.

Table with columns: Parameter and Value, including Abscissa for Analysis (Xa), Ordinate for Analysis (Ya), Poisson's Ratio, Recompression Factor, Total Settlement Multiplier, Stress Bulb Criterion, Current Time, Depth of Wick Drains, Wick Drain Triangular Grid Spacing, Equivalent Drain Diameter, Equivalent Mandrel Diameter, Diameter of Smear Zone, and Drain Capacity Ratio.

Table with columns: Ultmate Settlement and Induced Settlement, and rows for TOTAL and REDUCD, showing values for Recompt Sett, Virgin Sett, Total Sett, and Bulb Sett.

Strain Factor = 0.50

Table with columns: Group, L (ft), and Region 1-10, showing values for Tv and Uv across 10 regions.

Main table with columns: Str No., Depth (ft), g', CR (N/D), RR (N/D), cv (s/ft^2), ch (s/ft^2), Total H (ft), Past Maximum (psf), Zero Burden (psf), Ultmate Settlement (Change, Value), Induced Settlement (Change, Value), Influence Factors (Region 1-10), Rate Parameters (Effective, Position), and Percent Consolidation for Combined Drainage (Region 1-10).

Summary table with columns: Parameter and Value, including TOTAL and REDUCD settlement values and %Ultimate.

SETTLEMENT ANALYSIS - STRESS DISTRIBUTION BY WESTERGAARD THEORY, MAGNITUDE AND RATE OF CONSOLIDATION BY TERZAGHI THEORY

Table with columns: Load Region, Region 1-10, and rows: Bearing Area Center, Xc (ft) =, Bearing Area Center, Yc (ft) =, Bearing Area Width in X-direction, B (ft) =, Bearing Area Length in Y-direction, L (ft) =, Net Applied Pressure (psf) =, Depth of Application, Da (ft) =, Depth of Foundation, Df (ft) =, Contact Pressure Multiplier (N/D) =, Time of Application (yrs) =

water at le 0.5  
1.03 2.47 100.00 247.39  
2.53 37.00 93.46 340.86

Table with columns: Region 1-10 and rows: Sub 1, Sub 2, Sub 3, Sub 4 with dx and dy values.

Strain Factor = 0.50

Table with rows: Abscissa for Analysis, Xa (ft) =, Ordinate for Analysis, Ya (ft) =, Poisson's Ratio, u (N/D) =, Recompression Factor (N/D) =, Total Settlement Multiplier (N/D) =, Stress Bulb Criterion (%) =, Current Time (yrs) =, Depth of Wick Drains (ft) =, Wick Drain Triangular Grid Spacing (ft) =, Equivalent Drain Diameter, dw (in) =, Equivalent Mandrel Diameter, dm (in) =, Diameter of Smear Zone, ds (in) =, Drain Capacity Ratio (scdf) =

Table with columns: Ultmate Settlement, Induced Settlement and rows: TOTAL, REDUCD with (in), (ft), %Ultimate values.

Table with columns: Group, L (ft), Region 1-10 and rows: I, II, III with TV and UV values.

Main data table with columns: Str No., Depth (ft), q' (pcf), CR (N/D), RR (N/D), cv (s/Byr), ch (s/Byr), Total H (ft), Eff Vert Stress at Mid-Stratum (psf), Ultmate Settlement (Recomp, Virgin, Total, Bulb), Induced Settlement (Recomp, Virgin, Total, Bulb), Influence Factors (Region 1-10), Rate Parameters (Effective, Position), Percent Consolidation for Combined Drainage (Region 1-10).

Summary table with columns: (in), (ft), %Ultimate and rows: TOTAL, REDUCD.

SETTLEMENT ANALYSIS - STRESS DISTRIBUTION BY WESTERGAARD THEORY, MAGNITUDE AND RATE OF CONSOLIDATION BY TERZAGHI THEORY

Table with columns: Load Region, Region 1, Region 2, Region 3, Region 4, Region 5, Region 6, Region 7, Region 8, Region 9, Region 10. Rows include parameters like Bearing Area Center, Net Applied Pressure, and Time of Application.

water at le 0.5  
1.35 4.15 100.00 414.62  
1.85 37.00 66.59 483.21

Table with columns: Region 1 to Region 10. Rows include Sub 1, Sub 2, Sub 3, Sub 4 with dx and dy values.

Strain Factor = 0.50

Table with parameters: Abscissa for Analysis, Ordinate for Analysis, Poisson's Ratio, Recompression Factor, etc.

Summary table for Ultimate and Induced Settlement with columns: Recompt Sett, Virgin Sett, Total Sett, Bulb Sett.

Table with columns: Group, L (ft), and Region 1 to Region 10. Rows I, II, III.

Main settlement analysis table with columns: Str No., Depth, q', CR, RR, cv, ch, Total H, Fact Maximum, Zero Burden, Ultimate Settlement, Induced Settlement, Influence Factors, Rate Parameters, Percent Consolidation.

Summary table for Ultimate and Induced Settlement (repeated) with columns: Recompt Sett, Virgin Sett, Total Sett, Bulb Sett.





CONSOLIDATION SETTLEMENT OF DIKE CONTAINMENT - CONSIDERING ADJACENT MARSH PLATFORM AND BEACH FILL (PLATFORM AT 3)  
AND ESTIMATE OF SELF WEIGHT CONSOLIDATION

	<b>INITIAL DIKE CREST ELEVATION</b>	4	4	4	4	4	4	4	4	4	6	6	6	6	6	6	6	6	
	<b>CREST AFTER SELF WEIGHT CONSOL</b>	3.7	3.625	3.55	3.7	3.625	3.55	3.7	3.625	3.55	5.55	5.47	5.4	5.55	5.47	5.4	5.55	5.47	5.4
	<b>MUDLINE ELEVATION</b>	0	-1	-2	0	-1	-2	0	-1	-2	0	-1	-2	0	-1	-2	0	-1	-2
	<b>WATER SURFACE ELEVATION</b>	0.5	0.5	0.5	1	1	1	1.5	1.5	1.5	0.5	0.5	0.5	1	1	1	1.5	1.5	1.5
	<b>ULTIMATE SETTLEMENT (FT)</b>																		
	Magnitude of settlement in feet over time (consolidation settlement)																		
TIME (YRS)	% ULT																		
0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
0.5	0.23	0.21	0.24	0.27	0.18	0.22	0.25	0.16	0.18	0.22	0.31	0.35	0.37	0.29	0.32	0.35	0.26	0.30	0.32
1	0.34	0.31	0.35	0.40	0.27	0.32	0.37	0.24	0.27	0.33	0.46	0.51	0.55	0.43	0.47	0.52	0.39	0.44	0.47
3	0.58	0.53	0.60	0.68	0.45	0.55	0.64	0.41	0.46	0.56	0.78	0.87	0.94	0.74	0.79	0.88	0.66	0.75	0.80
5	0.68	0.63	0.70	0.80	0.53	0.64	0.75	0.48	0.54	0.65	0.92	1.02	1.10	0.86	0.93	1.03	0.78	0.88	0.94
10	0.8	0.74	0.82	0.94	0.62	0.75	0.88	0.56	0.64	0.77	1.08	1.20	1.30	1.02	1.10	1.22	0.91	1.03	1.10
20	0.88	0.81	0.91	1.04	0.69	0.83	0.97	0.62	0.70	0.84	1.19	1.32	1.43	1.12	1.21	1.34	1.00	1.14	1.21
ULT	1	0.92	1.03	1.18	0.78	0.94	1.10	0.70	0.80	0.96	1.35	1.50	1.62	1.27	1.37	1.52	1.14	1.29	1.38

Elevation of fill over time considering self weight consolidation

TIME (YRS)																			
0		3.7	3.63	3.55	3.70	3.63	3.55	3.70	3.63	3.55	5.55	5.47	5.40	5.55	5.47	5.40	5.55	5.47	5.40
0.5		3.488	3.39	3.28	3.52	3.41	3.30	3.54	3.44	3.33	5.24	5.13	5.03	5.26	5.15	5.05	5.29	5.17	5.08
1		3.387	3.27	3.15	3.43	3.31	3.18	3.46	3.35	3.22	5.09	4.96	4.85	5.12	5.00	4.88	5.16	5.03	4.93
3		3.166	3.03	2.87	3.25	3.08	2.91	3.29	3.16	2.99	4.77	4.60	4.46	4.81	4.68	4.52	4.89	4.72	4.60
5		3.074	2.92	2.75	3.17	2.99	2.80	3.22	3.08	2.90	4.63	4.45	4.30	4.69	4.54	4.37	4.77	4.59	4.46
10		2.964	2.80	2.61	3.08	2.87	2.67	3.14	2.99	2.78	4.47	4.27	4.10	4.53	4.37	4.18	4.64	4.44	4.30
20		2.89	2.72	2.51	3.01	2.80	2.58	3.08	2.92	2.71	4.36	4.15	3.97	4.43	4.26	4.06	4.55	4.33	4.19
ULT		2.78	2.60	2.37	2.92	2.69	2.45	3.00	2.83	2.59	4.20	3.97	3.78	4.28	4.10	3.88	4.41	4.18	4.02

CONSOLIDATION SETTLEMENT OF DIKE CONTAINMENT - CONSIDERING ADJACENT MARSH PLATFORM AND BEACH FILL (PLATFORM AT 3)  
AND ESTIMATE OF SELF WEIGHT CONSOLIDATION AND GEOLOGIC SUBSIDENCE

		INITIAL DIKE CREST ELEVATION			4			4			4			6			6			6		
		CREST AFTER SELF WEIGHT CONSOL			3.7			3.625			3.55			5.55			5.47			5.4		
		MUDLINE ELEVATION			0			-1			-2			0			-1			-2		
		WATER SURFACE ELEVATION			0.5			0.5			0.5			1			1			1		
		ULTIMATE SETTLEMENT (FT)																				
Magnitude of settlement in feet over time (consolidation settlement)																						
TIME (YRS)	% ULT																					
0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
0.5	0.23	0.21	0.24	0.27	0.18	0.22	0.25	0.16	0.18	0.22	0.31	0.35	0.37	0.29	0.32	0.35	0.26	0.30	0.32			
1	0.34	0.31	0.35	0.40	0.27	0.32	0.37	0.24	0.27	0.33	0.46	0.51	0.55	0.43	0.47	0.52	0.39	0.44	0.47			
3	0.58	0.53	0.60	0.68	0.45	0.55	0.64	0.41	0.46	0.56	0.78	0.87	0.94	0.74	0.79	0.88	0.66	0.75	0.80			
5	0.68	0.63	0.70	0.80	0.53	0.64	0.75	0.48	0.54	0.65	0.92	1.02	1.10	0.86	0.93	1.03	0.78	0.88	0.94			
10	0.8	0.74	0.82	0.94	0.62	0.75	0.88	0.56	0.64	0.77	1.08	1.20	1.30	1.02	1.10	1.22	0.91	1.03	1.10			
20	0.88	0.81	0.91	1.04	0.69	0.83	0.97	0.62	0.70	0.84	1.19	1.32	1.43	1.12	1.21	1.34	1.00	1.14	1.21			
ULT	1	0.92	1.03	1.18	0.78	0.94	1.10	0.70	0.80	0.96	1.35	1.50	1.62	1.27	1.37	1.52	1.14	1.29	1.38			

Elevation of fill over time considering self weight consolidation and geologic subsidence

TIME (YRS)	sub (ft)																				
0	0	3.7	3.63	3.55	3.70	3.63	3.55	3.70	3.63	3.55	5.55	5.47	5.40	5.55	5.47	5.40	5.55	5.47	5.40		
0.5	0.0125	3.48	3.38	3.27	3.51	3.40	3.28	3.53	3.43	3.32	5.23	5.11	5.01	5.25	5.14	5.04	5.28	5.16	5.07		
1	0.025	3.36	3.25	3.12	3.41	3.28	3.15	3.44	3.33	3.20	5.07	4.94	4.82	5.09	4.98	4.86	5.14	5.01	4.91		
3	0.075	3.09	2.95	2.79	3.17	3.00	2.84	3.22	3.09	2.92	4.69	4.53	4.39	4.74	4.60	4.44	4.81	4.65	4.52		
5	0.125	2.95	2.80	2.62	3.04	2.86	2.68	3.10	2.96	2.77	4.51	4.33	4.17	4.56	4.41	4.24	4.65	4.47	4.34		
10	0.25	2.71	2.55	2.36	2.83	2.62	2.42	2.89	2.74	2.53	4.22	4.02	3.85	4.28	4.12	3.93	4.39	4.19	4.05		
20	0.5	2.39	2.22	2.01	2.51	2.30	2.08	2.58	2.42	2.21	3.86	3.65	3.47	3.93	3.76	3.56	4.05	3.83	3.69		
ULT																					